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Publication Date

2023-10-23

PARTNERS FOR ADVANCED TRANSPORTATION TECHNOLOGY INSTITUTE OF TRANSPORTATION STUDIES UNIVERSITY OF CALIFORNIA, BERKELEY

Reimagining Sensor Deployment

Task ID 3942 (65A0958)

Final Report

October 23, 2023





Partners for Advanced Transportation Technology works with researchers, practitioners, and industry to implement transportation research and innovation, including products and services that improve the efficiency, safety, and security of the transportation system.

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1. INTRODUCTION

The California Department of Transportation (Caltrans) collects megabytes of data every day using a dedicated traffic sensing infrastructure. The collected data provide support for traffic management and system performance monitoring activities that are crucial for supporting the agency's mission, vision, and strategic goals to strengthen stewardship and drive efficiency. Operating this vast detection system requires extensive resources in the form of engineering and maintenance support, along with millions in capital funds to keep the system running.

Within the above context, alternate hybrid data collection models utilizing purchased or third-party data to augment existing data collection system capabilities may enable a reduction in the number of physical detection stations required while maintaining suitable accuracy for Caltrans' purposes. In addition to the potential for cost savings, the reliance on fewer physical sensors also offers the potential to reduce the exposure of Caltrans employees to the occupational hazard of maintaining roadside detection stations, in alignment with the agency's "safety first" strategic goal.

Most third-party data providers can now provide detailed travel time or speed data on any route over which a sufficient number of vehicles are observed traveling. These vendors also generally provide online platforms allowing their customers to visualize graphically collected data and conduct various analyses using the collected trip data samples and estimated traffic performance statistics. While some issues may remain regarding data quality, largely linked to the size of the data samples collected, and the ability to accurately estimate volumes, data quality is expected to improve over time as mobile devices and connected vehicles continue to proliferate and as data analytical processes are improved.

Recent research (Khan, Fournier, Mauch, Patire, & Skabardonis, 2020) has provided practical methods to integrate third-party data into the existing reporting platform and deliverables such as the Mobility Performance Report (MPR). Based on prior analyses, a hybrid approach was shown to provide better estimates of performance measures compared to using only fixed roadway detection stations. These results hold for analyses focusing either on mainline freeway segments or freeway-to-freeway connectors covering different periods during weekday hours. For almost all tested scenarios, the inclusion of third-party travel-time data has been found to reduce estimation errors. These benefits were also found to hold when the number of fixed detectors is reduced.

This project reimagines sensor deployment in the context of a near-term possibility where third-party data is procured to obtain travel time data and sample flow data across a district or the entire state. In this potential future, the role of dedicated roadside detection stations can be expected to change due to the availability of alternate data sources. For instance, it may no longer be necessary to deploy detection stations at regular spacings to build a detailed picture of traffic conditions along a freeway or within an area. Therefore, a new paradigm is required to guide decisions on where detection stations should be placed to provide the most informational value for Caltrans and other traffic data users.

1.1. CONTEXT OF DIGITAL TRANSPORTATION INFRASTRUCTURE

Digital infrastructure, as opposed to physical infrastructure, refers to data, communications, and computational elements needed to manage the transportation system and support its burgeoning transformation to a connected, automated, and electrified system. This transportation is notably

supported by the increasing deployment of the Internet of Things (IoT) and Connected and Automated Vehicle (CAV) technologies, which also have the potential to revolutionize the way we travel. In this context, data should be useful, trustworthy, and reusable. To efficiently support all emerging and envisioned functionalities, it should be possible to measure items of interest once and subsequently use the resulting data many times.

CAVs (including those on the road today) collect a vast amount of data through onboard systems (e.g., RADAR, Light Detection and Ranging (LiDAR), camera). However, this data is not ordinarily shared with other vehicles, roadside infrastructure, or public transportation agencies. This lack of collaboration and continued fragmentation of data may forfeit the opportunity to manage traffic at the systems-level, where significant gains can be made in terms of improving traffic flow and safety, as well as reducing greenhouse gas emissions and vehicle energy use.

Although the scope of this research is limited to freeways and traffic estimation, it is useful to consider a larger context, as illustrated in Figure 1-1.

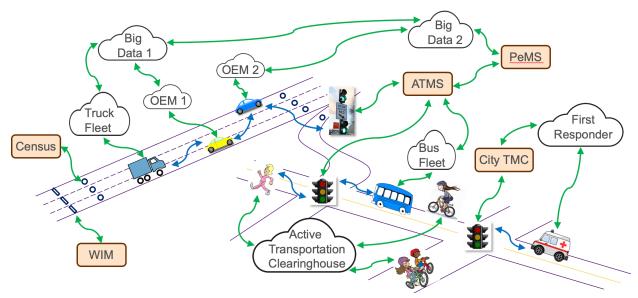


Figure 1-1: Illustration of a Traffic Data Ecosystem

The figure depicts examples of common physical elements found in a transportation network and possible data connections between them (blue arrows) and with data clouds (green arrows) that might, or might not, exist. For instance, the freeway on the upper left side shows loop sensors that may be connected to a Census system or Caltrans Performance Measurement System (PeMS). It also shows Weigh-in-Motion (WIM) sensors that presently archive data in their own system. Passenger cars using the freeway, particularly recent models, likely provide location and status data to original equipment manufacturer (OEM) systems, while commercial trucks may also provide data to logistical fleet systems. Big data aggregators may further purchase data from OEMs and logistics companies, but the exact relationships are fluid and not transparent. The arterial on the lower right side depicts other transportation elements such as pedestrians, bikes, transit, emergency vehicles, and infrastructure elements such as signals and city traffic management centers (TMCs). Although out of scope for this research, these elements present additional opportunities for data gathering and exchange that could improve safety, operations, signal priority, and more.

As noted above, this research is focused on sensors deployed on freeways for better traffic estimation as illustrated in the upper left side of Figure 1-1. This work explicitly considers the need for combining data sets to synthesize better information for better decision-making. Thematically, however, this approach is widely applicable to other parts of the transportation ecosystem.

1.2. BACKGROUND

The problem of determining the best placement of detection stations depends primarily on the goals that one is trying to achieve. This project takes a nuanced view based on existing trends, emerging technologies, recent executive orders and legislation, future applications, and the existing array of sensors that have already been deployed in California.

Key background issues and considerations are described in more detail in the following subsections. This includes:

- Needs for sensors
- Climate change considerations
- Future operational needs
- Emerging third-party data
- Emerging needs for data fusion

1.2.1. NEED FOR SENSORS

Many reasons justify the need to measure the performance of a traffic network. An agency responsible for the operation of such a network needs to build, and subsequently maintain to a satisfactory level, various transportation assets on behalf of the traveling public. To do so requires collecting data supporting the following activities, among potential others:

- Infrastructure inventory status assessment.
- Traffic demand reporting for fund allocations.
- Performance measurements for planning purposes.
- Collection of information to support real-time traffic operations and control activities.

The above requirements generally translate into a need to collect traffic volume and speed data, as well as segment travel time data, from various locations. In turn, this data may be used to assess various metrics of interest, such as delays, vehicle-miles of travel, vehicle-hours of travel, levels of service, average vehicle emissions, and safety metrics. Real-time flow data may further be used to support the operation of dynamic ramp meters, vehicle-actuated traffic signals, and traveler information systems.

Within the above context, sensor placement is largely dependent on the purpose for the data collection, and more specifically on the traveling aspects that are meant to be captured. For instance, regional planning studies may primarily be concerned with the ability to collect flow data from a predefined set of critical locations as well as routing and travel time data between key origins and destinations. However, real-time corridor management applications may on the other hand need to rely on traffic flow and speed data collected near every key interchange, if not all interchanges. Similarly, the operation of ramp meters and actuated traffic signals often requires detailed monitoring of local traffic conditions.

This project recognizes that requirements for real-time operations are more stringent than for traffic studies. For instance, the locations of the ramp meters and their associated sensors are typically determined (as they should be) by operational needs for real-time control. These kinds of decisions are predicated on local traffic needs, not on some global optimization. On the other end, greater flexibility exists regarding sensor placement when considering planning or operational assessment studies. Within this context, this project purposefully does not intend to assess the selection of sensor locations in support of real-time operations but rather chooses to leverage such data for additional purposes.

The key focus of this project is on sensors used to collect data in support of various performance assessments. This includes, among many others, the following reporting needs:

- Mobility performance Report (MPR), which mostly focuses on delays.
- Annual California Public Road Data Report, which focuses on vehicle-miles traveled (VMT) statistics extracted from the Highway Performance Monitoring System,
- California High Occupancy Vehicle (HOV) Facilities Degradation Report, which mostly focused on measuring average vehicle speeds on HOV lanes.

1.2.2. CLIMATE CHANGE CONSIDERATIONS

As a result of climate change and sustainability concerns, Caltrans and the State of California have recently shifted toward a new strategic direction that avoids building new roads that encourage increased vehicular use, and by extension increases in greenhouse gas (GHG) emissions. In the past, building additional lane miles was the standard solution to improve transportation system efficiency and achieve desired levels of service. Moving forward, the construction of new State Highway System (SHS) lane-miles is only to be viewed as a "solution of last resort" when all other options have failed, rather than a standard response to operational and safety problems.

The implementation of senate bill (SB) 743 reflects one aspect of this shift. To better highlight the impacts of a roadway capacity increase on stimulating additional vehicle travel, this bill changed the transportation analysis metric considered in impact assessments following the California Environmental Quality Act (CEQA) from the level of service associated with a given roadway section to the number of VMT. Governor Newsom's executive order (EO) N-19-19 also articulates the Administration's objective of reducing overall vehicle use, while the draft Climate Action Plan for Transportation Infrastructure (CAPTI) further reflects the Executive Order's mandates.

The above changes have created a need to measure VMT and do so regularly and reliably. This need may further become increasingly nuanced in the years to come. For instance, Governor Newsom's executive order EO-N-79-20 mandates that all new sales of light-duty trucks and passenger cars be zero-emission vehicles by the 2035 model year. Based on this mandate, it will eventually become important to differentiate between VMT impacts linked to gasoline-powered vehicles and impacts linked to vehicles using clean energy sources such as electricity of hydrogen. In response to an increased interest in promoting high-occupancy modes of travel as another way to reduce VMT, it will also likely become increasingly important to differentiate between person-miles of and vehicle-miles of travel.

For now, VMT estimation and reporting remain limited. While average daily traffic counts from the Census program are currently used to provide annual assessments of activities within the State Highway System (SHS), most of the stations from which counts are obtained do not operate continuously. Apart from a small fraction of sensors, counts are typically collected from each location once every three years. In

addition, while counts from permanent sensors associated with PeMS could supplement the Census data, data from this system is mainly only collected over freeways.

Implementation of state bill SB-743 implies a requirement for more detailed project-level VMT evaluation, with increased needs for local data collection and before/after studies. In this context, the procurement of rural data is a substantial challenge due to a general sparsity of detailed count data. In addition, changes in travel behavior following the COVID-19 pandemic have created an additional need to reassess daily weekday and weekend traffic demand patterns, as well as patterns over different facility types. This notably implies greater reliance on recent traffic counts.

1.2.3. FUTURE OPERATIONAL NEEDS

Emerging applications such as integrated corridor management (ICM) and proactive traffic management applications require data of an ever-increasing quality to achieve their promised benefits. This is reflected in traffic studies that are increasingly being asked to provide answers to difficult questions. While models could be used to provide some answers, their modeling and analytical capabilities are often limited by the quality of available data. Oftentimes, errors in data are not discovered until the data have already been used to develop and calibrate a model. During this process, it is often discovered that the data that was used is faulty. It is unfortunate when previously accepted performance measures are found to be based on faulty data.

1.2.4. EMERGING THIRD-PARTY DATA

Given the preceding discussion, this project seeks to explore a specific possible future where third-party data are procured and used effectively in decision-making processes.

In recent years, private vendors such as INRIX, Streetlight, HERE, and many others have created online platforms for visualizing data derived from mobile sources such as smartphones, onboard vehicle navigation systems, commercial fleet management systems, etc. The increase in trip data collection capabilities supporting these platforms has been highly correlated with the proliferation of smartphones. The current emergence of connected vehicles further opens the potential for an increased ability to reliably track vehicle movements.

Information collected through these platforms typically consists of the following data:

- Average travel times on roadway segments
- Trip statistics

Travel time over roadway segments is a well-accepted and commonly used metric from third-party data providers as obtaining reasonably reliable travel time stations does not require large data samples. It has been repeatedly established in the literature that a sample penetration rate as low as 2-5% is sufficient for good travel time estimates on freeways. With trip sample penetration rates from mobile devices currently varying between 1% and 35%, with a median of 11.6%, according to a Streetlight market penetration report (StreetLight, 2020), depending on location and time, reasonable travel time estimates can typically be obtained for heavily traveled segments. The reason why accurate travel times can be obtained with limited data is that vehicle speeds are highly correlated. This is because the speed of a following vehicle is often limited by that of its leader and influenced to a certain extent by the speed of vehicles on adjacent lanes. As connected vehicles continue to gain market share, travel time coverage is

notably being extended to arterial streets, thus allowing evaluations to be done on an increasing portion of the roadway network.

The ability to analyze sample trips from mobile data is a newer capability. While it is now possible to track trips from a given starting point to a given destination, this ability is limited in part by privacy agreements that only allow anonymized data to be posted on online analytical platforms. This is typically done by masking the actual start and end points of tracked trips or only providing access to aggregated data. A remaining limitation of this type of data is that there is no guarantee that it can sample all trips conducted along a roadway. Data is typically collected from samples of trips that may vary in size depending on location and time. This prevents directly using the collected data as a direct replacement for count data captured by fixed road sensors. While count estimates could be derived from the sampled trips, the data currently being collected will not allow for matching the accuracy provided by fixed sensors.

Despite the above limitation, third-party data is still useful for analyzing routing patterns. Third-party data has long been used to analyze origin-destination patterns within an area. A recent trend is further seeing the introduction of capabilities for analyzing trip patterns on a much smaller scale, such as turning movement patterns at intersections. While the collected data provides a window into trip patterns, uncertainty remains on whether the collected trip samples are unbiased, and representative of the trips made by all types of individuals. The question remains whether more data is collected from certain population groups than others. This is an area of ongoing research.

While some areas of improvement still exist, previous work has shown that it is possible to improve our understanding of traffic movements made within a road network by fusing vendor-based travel time and routing data collected from probe vehicles with flow data collected from infrastructure-based sensors. This was more particularly assessed to be true for the estimation of commonly used performance metrics such as VMT, vehicle-hours traveled (VHT), and delay (Khan, Fournier, Mauch, Patire, & Skabardonis, 2020).

1.2.5. EMERGING NEEDS FOR DATA FUSION

One crucial use for sensor data is to provide normalization factors, and quality checks on private sources of transportation data, i.e., from so-called third-party sources. Private companies provide analyses of travel times, routing, turning ratios, and other metrics based on data collected from probes, connected vehicles, fleet telematics, and other possible sources. However, these data are usually sampled from a limited and likely biased population of travelers, and this bias is unlikely to change quickly in the coming years.

Infrastructure-based sensors collect a complete cross-section of data across a facility, unlike data from third parties. Instead of relying on data samples, these sensors offer the ability to capture information about every vehicle passing a given location, thus providing highly reliable ground truth representations when sensors operate correctly. In this context, sensor data is therefore special and crucially important.

1.3. PURPOSE

This project aims to provide practical guidance to suggest the best placement of fixed detection stations (mostly counting sensors) in a context in which third-party data can be used to increase data collection over a wide geographic coverage. The analysis focused on mainline detector data and large freeway

interchanges where variations in flow are expected or observed. This document is the final report for Task ID 3942, also known as the Reimagining Sensor Deployment project.

The purpose of this report is to communicate the core results and recommendations of the research. In addition, it describes the key tasks that have been conducted and their outcomes. The most important outcome is to provide practical guidelines for sensor placement. Toward this end, this research proposes to reorganize the primary organizational unit for freeway data around interchanges, not around freeways. The reason for this is that interchanges are major decision points with various rerouting options and places where freeway flows often change, and therefore, places where sensing data are most useful.

The proposed guidelines are based on two methodologies that can be implemented in stages. In stage 1, interchanges along a freeway are listed and categorized. In stage 2, data from existing sensors are analyzed to find locations where the most significant flow changes occur across one or more analysis periods. In stage 3, data from the various analyses conducted in Stage 2 are combined to produce an aggregated evaluation score for each sensor. This is done through a voting mechanism that provides the basis of a user-customizable prioritization. The process should be interpreted as a way to discover a set of interchanges that need complete instrumentation.

1.4. DOCUMENT ORGANIZATION

This document is organized as follows:

- Section 2 presents the results of an American Association of State Highway and Transportation
 Officials (AASHTO) survey deployed to determine the stance of various departments of
 transportation across North America regarding current and future data collection strategies,
 expectations for infrastructure-based sensor deployments, and plans for using purchased data
 from third parties.
- **Section 3** provides a review of prior academic studies on the problem of optimal sensor placement.
- Section 4 describes initial observations on existing deployments of counting sensors in California and provides an overview of existing sensor coverage for PeMS, Census, and Weigh-in-motion stations.
- **Section 5** explains the data quality assessment performed to select the candidate freeways for in-depth analysis.
- **Section 6** describes the considerations that went into the prioritization methodology developed in this research.
- Section 7 describes the three stages of the prioritization methodology that was developed:
 - Stage 1: Topological analysis
 - Stage 2: Empirical data analysis
 - Stage 3: Calculation of aggregated score
- Section 8 concludes with practical guidelines for sensor placement based on the prioritization methodology

2. SURVEY OF DATA TRENDS

This section reports the findings of a survey that was implemented in partnership with Caltrans and the American Association of State Highway and Transportation Officials (AASHTO) Research Advisory Committee (RAC). On November 28, 2022, a survey request was submitted to the AASHTO RAC. The survey itself was administered online using the University of California (UC) at Berkeley Qualtrics survey platform. The last response was received on December 14th, 2022.

2.1. SUMMARY FINDINGS

The survey obtained twenty valid responses from sixteen organizations. This section presents a summary of key findings.

According to the participants, the most important performance measures for decision-making are:

- Vehicle Counts / Annual Average Daily Traffic (AADT); and,
- Vehicle Speeds / Travel Times.

Most participants expect bicycle and pedestrian counts to become more important in the future, while the importance of Vehicle Counts / AADT, Truck Classification / Counts, and VMT/VHT, are expected to stay the same.

Participants unanimously expect more cameras to be deployed in the future, as well as more permanent and temporary sensors. Most expect about the same number of license plate readers, transponder readers, and WIM sensors in the future. However, the stance on inductive loops (whether there will be more or fewer in the future) is mixed. Several participants expect more Al-powered cameras for vehicle classification.

No participant reported that their organization does not purchase data now or does not plan to purchase data in the future. Some of the most popular vendors included INRIX and Streetlight, and some of the most popular information for purchase included:

- Vehicle speeds / Travel times
- Vehicle Flow / Volume
- Origin-Destination Demand Information

Most participants reported that purchased data were moderately to extremely useful. In the future, many participants expect to purchase data including freight movement, bicycle counts, and pedestrian counts.

The most reported intended use for purchased data was to support some type of planning process. Key themes included safety analysis and performance monitoring. Some participants also intend to use purchased data for real-time operations and to reduce dependence on loops.

Participants reported that most organizations use data from multiple sources to calculate performance measures to make business decisions. However, there was a range of opinions about how easy it is to relate multiple data sets for comparison and analysis. Most participants stated that their organization probably has specifications for allowable errors in data.

2.2. DETAILED FINDINGS

This section provides a detailed discussion of the responses to each of the questions of the survey.

From the thirty-nine total responses that were recorded, twenty of them were determined to be valid. The nineteen responses deemed invalid were discarded for the following reasons:

- One response was a test to confirm the correct operation of the survey itself
- Sixteen responses were empty
- One response was left open for three days. It appears that the participant stopped at Question 3
 and never returned to the page. It was unclear whether the response that was recorded for
 Question 3 was finalized as intended.
- One response indicated that their organization purchased data but provided no further details.

2.2.1. SURVEY PARTICIPANTS

The twenty collected valid responses came from sixteen different organizations:

- Two responses were collected from each of the following organizations:
 - Alaska Department of Transportation and Public Facilities
 - Nevada Department of Transportation
 - Virginia Department of Transportation
 - Washington State Department of Transportation
- A single response was collected from each of the following:
 - Arkansas Department of Transportation
 - Delaware Department of Transportation
 - Idaho Transportation Department
 - o Iowa Department of Transportation
 - Kentucky Transportation Cabinet
 - Minnesota Department of Transportation
 - New Hampshire Department of Transportation
 - Ohio Department of Transportation
 - Oklahoma Department of Transportation
 - Pennsylvania Department of Transportation
 - South Carolina Department of Transportation
 - Vermont Agency of Transportation

2.2.2. CURRENT RELATIVE IMPORTANCE OF PERFORMANCE MEASURES

Participants were asked to rank the relative importance of several performance measures used for decisions related to traffic operations for their organization. Fifteen participants chose to answer this question. The ranked list by average score is as follows:

- 1. Vehicle Counts / Annual Average Daily Traffic (AADT)
- 2. Vehicle Speeds / Travel Times
- 3. Truck Classification / Counts

- 4. Vehicle Miles Traveled / Vehicle Hours Traveled (VMT/VHT)
- 5. Pedestrian Counts
- 6. Bicycle Counts

Vehicle Counts and AADT were clearly at the top of the list (receiving nine votes as the most important) and were consistently ranked as number one or number two. Vehicle Speeds / Travel Times received six votes as the most important. Except for Vermont, Truck Classification / Counts were consistently ranked as number two or number three. Except for New Hampshire, all participants ranked Pedestrian and Bicycle Counts as either number five or six in order of importance.

2.2.3. FUTURE RELATIVE IMPORTANCE OF PERFORMANCE MEASURES

Participants were asked to project forward over the next ten years and to predict which performance measures were likely to become more important for their organization. Nineteen participants chose to answer this question.

Overall, the vast majority of participants expected bicycle and pedestrian counts to become more important. In addition, a smaller majority expected that Vehicle Speeds / Travel Times would become more important although a slightly smaller number expected their importance to be about the same. For all the remaining measures (Vehicle Counts / AADT, Truck Classification / Counts, VMT/VHT), participants felt their level of importance would not change.

2.2.4. NEW PERFORMANCE MEASURES UNDER CONSIDERATION

Participants were asked whether their organization was considering the use of new performance measures. Nine participants chose to elaborate. Two participants reported that bicycle and pedestrian-related metrics are under consideration such as the Level of Traffic Stress for Pedestrians, and Route directness.

Four participants replied that to their knowledge, no other performance measures are being considered. One participant reported that "Our area only handles motorized counts within the state and does not handle the bicycle/pedestrian counts that could occur."

Four participants responded that delay and/or reliability-related metrics are under consideration such as:

- Winter speeds as it relates to vehicle speeds and travel times
- Vehicle Travel Time Reliability-Related Performance Measures
 - 80th percentile Travel Time Index,
 - Vehicle Delay during Unreliable Conditions

2.2.5. INFRASTRUCTURE-BASED SENSORS

Participants were asked to provide rough estimates of the number of permanently deployed infrastructure-based sensors maintained by their organizations. Fifteen participants answered this question.

Table 2-1: Deployments of permanent sensors

	Table 2-1. Deployments of permanent sensors						
Organization	Inductive Loops	Radar	License plate readers	Transponder readers (FasTrak/EZ- Pass)	Cameras	Weigh-in- Motion Sensors	Interstate/ expressway lane miles ¹
Alaska Department of							
Transportation and Public							
Facilities	250	10	1	0	0	8	2,377
Arkansas Department of							
Transportation	0	3	0	6	300	50	3,947
Delaware Department of							
Transportation	2,000	259	0	5,000	313	10	549
Idaho Transportation							
Department	151	72	0	0	0	26	2,775
Iowa Department of							
Transportation	150	350	15	0	550	50	3,311
Kentucky Transportation							
Cabinet	11,500	250	17	39	250	13	6,443
Minnesota Department of							
Transportation	3000	450	0	74	1,100	24	5,193
Nevada Department of							
Transportation	123	0	0	0	3	13	3,015
New Hampshire Department of							
Transportation	200	100	0	9	200	10	1,405
Ohio Department of							
Transportation	500	6500	0	0	1,100	0	12,193
Pennsylvania Department of							
Transportation	128	1	0	0	0	21	11,766
South Carolina Department of							
Transportation	166	1				19	4,434
Vermont Agency of							
Transportation	120	50	0	0	90	90	1,320
Virginia Department of							
Transportation	300	200	0		0	3	7,554
Washington State Department							
of Transportation	11,930	40	30	30	930	14	7,203

¹ Information about interstate / expressway lane-miles was not asked in the survey. For comparison purposes, it is displayed in Table 2-1. The values were calculated by adding the lane miles for "interstate" and "other freeways and expressways" as reported on October 26, 2021 in the FHWA statistics according to https://www.fhwa.dot.gov/policyinformation/statistics/2020/

2.2.6. FUTURE EXPECTATIONS OF INFRASTRUCTURE-BASED SENSORS

Participants were asked to project forward over the next ten years and to comment on whether they expected more of the same sensors or alternative types to be used. For each type, participants indicated whether more or fewer were expected to be actively deployed. Sixteen participants answered this question.

Across the board, all sixteen expected more cameras to be deployed. Vast majorities expected more of both permanent and temporary sensors. Regarding license plate readers, transponder readers, and WIM sensors, the majority expect about the same number to be deployed. The stance toward inductive loops was split: five participants reported that they expected fewer, four about the same, and six more.

2.2.7. DEPLOYMENT OF NEW SENSOR TYPES

Participants were asked whether new sensor types were under consideration for deployment. Ten participants chose to elaborate. Four participants mentioned video and/or Al-powered cameras. Two participants mentioned needs for more WIM bending plate sensors for classification and weight enforcement. Three participants mentioned WIM alternatives for classification such as:

- An artificial intelligence camera that collects classification in the FHWA 13 classes
 - o Developed by International Road Dynamics
 - o First full permanent site planned for 2023
- Camera-based vehicle classification -- Leetron Vision
- IRD side-shot classifiers with cameras and IR imaging for temporary vehicle classification

Participants also mentioned the following sensors and applications for consideration:

- Bluetooth and WIFI
- LiDAR Sensors for traffic signal detection
- Connected and Automated Vehicles (CAV) and transit priority detection for signal optimization related to CAV/Transit
- Work zone/school zone automated speed enforcement technology
- Replacing existing radars with better radars having more immediate access and refined data

2.2.8. CURRENT PURCHASE OF DATA FROM PRIVATE VENDORS

All fourteen participants reported that their organization purchased data from private vendors. No one answered in the negative. The two most popular vendors are INRIX and Streetlight.

Table 2-2: Current purchases of data from private vendors

Vendor	Number of participants who purchase data from vendor
HERE	5
INRIX	9
TomTom	1
StreetLight	7
Citilabs	0
Replica	0
Waze	3
Other	5

Other vendors from whom data are purchased include:

- Wejo
- Southern Traffic Services
- The Traffic Group
- Surface Transportation Board
- RITIS

2.2.9. CURRENT DATA TYPES PURCHASED FROM PRIVATE VENDORS

Of the fourteen participants who purchased data, thirteen chose to elaborate on what data types are purchased. The most popular purchases are vehicle speeds and travel times.

Table 2-3: Current types of data purchased

Information type purchased	Number of participants who purchased this type of information
Vehicle Flow / Volume	7
Vehicle speeds / Travel times	12
VMT / VHT	4
Trip and Mode Choice	2
Origin-Destination Demand Information	7
Intersection Turning Counts	3
GPS Traces	2
Freight Movement	1
Bicycle Counts	2
Pedestrian Counts	3
Other	3

In addition, three participants indicated the following types of purchased data:

- Consultants to collect ped/bike/intersection count data per project need
- Incident detection (Waze & Wejo)
- Hourly volume, speed, vehicle classification, and WIM

2.2.10. CURRENT USEFULNESS OF PURCHASED DATA

Thirteen participants rated the usefulness of the purchased data. Most rated the data as moderately or extremely useful.

Table 2-4: Usefulness of purchased data

Usefulness of purchased data	Number of participants who provided this rating
Extremely useless	0
Moderately useless	1
Slightly useless	0
Neither useful nor useless	1
Slightly useful	0

Moderately useful	5
Extremely useful	6
Don't know	0

2.2.11. EXPECTED FUTURE PURCHASE OF DATA FROM PRIVATE VENDORS

All fifteen participants reported their organization intends to purchase data from private vendors in the future. No one answered in the negative. The two most popular vendors are expected to be INRIX and Streetlight. Three participants expect to purchase Wejo or connected vehicle data.

Table 2-5: Expected future purchases of data from private vendors

Vendor	Number of participants who expect to purchase data from this vendor
HERE	2
INRIX	8
TomTom	2
StreetLight	10
Citilabs	0
Replica	0
Waze	3
Other	7

Other vendors from whom data are expected to be purchased include:

- Wejo
- Michelin DDI
- Southern Traffic Services
- Abley Safe Curves

2.2.12. EXPECTED FUTURE DATA TYPES PURCHASED FROM PRIVATE VENDORS

All of the fourteen participants who expect to purchase data in the future elaborated on what data types they expect to purchase. The most popular purchase is expected to be vehicle speeds and travel times.

Table 2-6: Expected types of data for future purchase

Information type purchased	Number of participants who purchased this type of information
Vehicle Flow / Volume	11
Vehicle speeds / Travel times	12
VMT / VHT	4
Trip and Mode Choice	4
Origin-Destination Demand Information	10
Intersection Turning Counts	5
GPS Traces	2
Freight Movement	7

Bicycle Counts	6
Pedestrian Counts	6
Other	4

In addition, four participants indicated the following types of purchased data:

- Safety data
- Conflation services
- Connected vehicle data
- Hourly volume, speed, vehicle classification, and WIM; bicycle and pedestrian data

2.2.13. INTENDED USES OF PURCHASED DATA

Eleven participants reported how their organization intends to use purchased data. The most often cited purpose was to support some type of planning process. Key themes included safety analysis and performance monitoring. Some participants also intend to use purchased data for real-time operations and to reduce dependence on loops. Specific uses included the following:

- Prioritization within the Long-Range Plan process
- General planning and work zone-related data. Detouring optimization and overall TMP planning for construction projects
- We want to use it to support our ramp meeting operation. We and to lessen our dependence on loop detectors
- Planning for mobility and safety
- Network management, work zone safety, construction planning, highway safety, performance measures
- The purchased data will be used for Improvement Alternatives Analytics, Performance Monitoring, Decision Making on Transportation Systems Management and Operations (TSMO)-related projects, etc.
- To meet federal reporting requirements of Model Inventory of Roadway Elements (MIRE) 2.0 and to inform decision-making and planning
- Planning studies (origin-destination); bottleneck identification; corridor operational analysis;
 Planning & Environmental Linkage (PEL) studies
- real-time operations/monitoring (incident detection, travel times, etc.), work zone analysis, vehicle volumes (AADTs) and planning activities
- To calculate performance measures
- Planning studies, operational analysis, safety analysis, resiliency, project identification, design decisions, infrastructure needs, project scheduling, freight bottleneck analysis, etc.

2.2.14. USING DATA FROM MULTIPLE SOURCES

Participants reported that most organizations use data from multiple sources to calculate performance measures to make business decisions. However, there was a range of opinions about how easy it is to relate multiple data sets for comparison and analysis. Most participants stated that their organization probably has specifications for allowable errors in data.

Table 2-7: Data usage and error specifications

Response	Does your organization use data from multiple (internal and external) sources to calculate performance measures to make business decisions?	For the data your organization collects, is it easy to relate multiple datasets from multiple (internal and external) sources for comparison and analysis?	Does your organization have specifications for allowable errors in purchased data, infrastructure-based sensor data, and in performance measures?
Definitely not	1	3	0
Probably not	0	3	2
Might or might not	3	3	3
Probably yes	3	4	7
Definitely yes	9	2	2
Don't' know	0	0	0
All answers	16	15	14

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3. REVIEW OF SENSOR PLACEMENT LITERATURE

The purpose of this literature review is to take an inventory of the types of sensor location problems that have been investigated in academic publications. Sensors are deployed for multiple purposes and researchers have attempted to optimize their placement to collect travel time information, locate bottlenecks, or estimate Origin Destination (OD) demands. Some of the literature considers the impacts of measurement errors and sensor failures. In addition, some optimization formulations incorporate a budget intended to limit the maximum number of sensors or their total cost.

3.1. OVERVIEW

One general category of the literature focuses on vehicular flows. For example, (Gentili & Mirchandani, 2012) provides an extensive literature review of this field and synthesizes a framework to organize previous work in this area. In this framework, there are two main categories of inquiry: (1) the Sensor Location Flow-Observability problem; and (2) the Sensor Location Flow-Estimation problem. The former observability problem attempts to find a unique solution to determine all the flows in the system while also optimizing (in some sense) the allocation of sensors. The latter estimation problem attempts not to find a unique solution, but rather to improve the quality of some flow-related estimates such as OD trips, link flows, or route flows.

The observability problem receives a good deal of attention. Subsequent works such as (Xu, Lo, Chen, & Castillo, 2016) and (Shao, Xie, & Sun, 2021) propose approaches to minimize the effect of measurement error that could propagate into the inference of flows on unmeasured links.

For the estimation problem, a great deal of work focuses on determining the best link-counting locations for OD estimation. For example, (Zhou & List, 2010) proposes a Kalman filtering framework to combine both automatic vehicle identification (AVI) and counting sensor data with prior OD information. It considers the structure of uncertainty in historical demand as well as sensor measurement errors. The objective function is to minimize expected uncertainty in the OD demand estimate subject to a budget constraint. The solution method involves stochastic optimization and beam search. One limitation is that the method assumes that the market penetration of "tagged" AVI vehicles is representative of the overall population. In practice, the algorithm prioritizes the largest OD pairs.

The OD demand estimation problem is also considered in (Fei & Mahmassani, 2011). This article considers the information available from counting sensors. The objective function is to maximize the link informational gains from each sensor that is placed. As formulated, it is a binary integer nonlinear programming (BINLP) problem. The proposed heuristic greedy solution method provides near-optimal results and recognizes budget constraints. The article further defines the notion of eigenvolumes and eigenlinks. An eigenlink is a virtual link that captures an element of demand variance in the OD matrix, while an eigenvolume is the volume on the associated eigenlink. The intuition offered by the authors is that sensors should be placed by finding and reducing demand variances. Sensors should be placed on the most "unstable" links that exhibit high variability in demands and flows. The first sensor should be placed on a link to capture the largest demand variance. The second sensor should be placed to capture the remaining variance not accounted for by the first sensor, and so on. The authors compare the process to a kind of dimensionality reduction problem similar to Principle Component Analysis (PCA).

Instead of OD demand estimation, (Liu & Danczyk, 2009) considers the challenge of placing counting sensors to find freeway bottlenecks. The chief consideration is to find places where the velocity changes. The authors define the notion of a benefit factor used to determine the relative value of a pair of sensors for recognizing a bottleneck. The objective function maximizes this benefit factor, defined as a nonnegative speed difference between adjacent sensors and divided by the distance between cells. The analytical model includes constraints for the total cost and the number of sensors. The formulation is limited to a unidirectional freeway. It requires modeling the freeway using the cell transmission model (CTM) or some method to calculate link velocities. The velocities are then used to calculate the benefit factor. A genetic algorithm is used to solve the nonlinear mixed-integer optimization problem.

The travel time estimation problem is investigated in (Zhan & Ran, 2022). However, the article considers both fixed and mobile data sources. Its objective is to minimize the sum of two things: the average travel time mean absolute percentage error (MAPE) of all segments and the variance of the segment length. The solution method has two parts. For fixed sensors, a genetic algorithm is employed. For mobile sensors, a simulation method is used to determine a point of diminishing returns from probe data. Regarding the role of mobile sensors for travel time estimation, this article finds diminishing returns at about a 6% penetration rate. One limitation of this work is that it only considers a unidirectional freeway with no network effects.

The application of travel time estimation using infrastructure-based AVI or speed sensors is explored in (Dehestani Bafghi & Ahmadi, 2022). The article considers the possibility of one or multiple sensor failures. Its objective is to minimize a combination of travel time error and OD coverage. The authors chose a floating search method that uses a feature reduction technique to reduce the number of combinations of sensor failures that need to be considered in the search space. One limitation is that the method assumes only one path for each OD pair.

3.2. SENSOR PLACEMENT GUIDELINES

This section describes guidelines for sensor placement found in the literature, mostly having to do with the set of literature concerned with origin-destination matrix estimation (ODME).

3.2.1. INTRODUCTION OF GUIDELINES FOR ODME

The first formulation of maximal possible relative error (MPRE) is credited to (Yang & Zhou, 1998). This MPRE concept is then employed to establish four location guidelines (or "rules") to determine the optimal number and locations of traffic counting sensors. This list is expanded to a total of eight guidelines in (Gentili & Mirchandani, 2012). Although these guidelines are often referred to as "rules", one should be mindful of the underlying assumptions that went into their development.

- Guideline 1 (OD Covering Rule): Traffic counting sensors should be allocated on a road network
 so that for every OD pair, at least some proportion of trips is observed. A criterion to maximize
 could be the number of OD pairs covered by a set of sensors.
- **Guideline 2 (maximal flow fraction rule)**: Traffic counting sensors should be allocated so that the flow fraction observed for any OD pair is maximized. A criterion to maximize could be the sum of the maximum flow fractions (on a link) over all the OD pairs.

- Guideline 3 (route covering rule or OD separation rule): Traffic counting sensors should be
 allocated on a road network so that for every OD pair, all the routes connecting them are
 covered. This guideline is more demanding than Guideline 1. A criterion to be maximized could
 be the total number of routes covered by a set of sensors.
- **Guideline 4 (maximal observed flow rule)**: For a fixed number of links to be observed, the chosen links should intercept as much flow as possible. This simple rule would prioritize the links with the highest flows, regardless of the additional information that might be obtained from them.
- **Guideline 5 (maximal OD demand fraction rule)**: Traffic counting sensors should be allocated so that the demand fraction observed for any OD pair is maximized. This is similar to Guideline 2, except that instead of using flow fractions calculated as ratios using the flows on each link, this criterion uses demand fractions calculated using the number of trips for each OD pair itself.
- Guideline 6 (maximal net route flow captured rule): For a fixed number of links to be observed,
 the chosen links should intercept as much net route flow as possible. What this means is that
 multiple counting of the same vehicle should be avoided. The net flow intercepted by the
 observed links should be maximized. Formulations involve maximizing the sum of route flows
 observed on each route.
- **Guideline 7 (maximal net OD trips captured rule)**: For a fixed number of links to be observed, the chosen links should intercept as many net OD trips as possible. This formulation also avoids multiple counting's of the same vehicle. Formulations involve maximizing the sum of trips observed on each link.
- Guideline 8 (link independence): Traffic counting sensors should be allocated such that traffic
 counts on observed links are linearly independent. In other words, avoid sensing links that do
 not provide additional information. What this means is that one should avoid a fully accounted
 traffic volume (FATV) as described in (Khan, Fournier, Mauch, Patire, & Skabardonis, 2020).

3.2.2. DISCUSSION OF GUIDELINES

Many of the above guidelines assume some prior knowledge. One type of knowledge is that of an OD matrix of all trips over some period. The period could be an entire day or a peak period of interest. In general OD matrices may be time-dependent and an optimal allocation of sensors for a morning peak might not be optimal for an afternoon peak. Another type of knowledge is that of a prior assignment of demand onto routes. This traffic assignment in general is also time dependent. In congested networks, a given OD trip during an off-peak period is likely to use a different route than during a peak period.

Computational experiments to compare the performance of alternate formulations have been noted in the literature (Gentili & Mirchandani, 2012). One interesting finding is that a simple model that maximizes Guideline 4 often provides "surprisingly good results," even without regard to link flow dependencies or double counting. It is possible to obtain superior performance using more complex models and more complex solution methods. One numerical improvement is noted for strategies that aim to reduce the uncertainties in route flow estimates.

In an ideal world, Guideline 8 seems sensible. In practice, however, measurement errors, sensor failures, and misconfiguration problems make Guideline 8 useless and unproductive. It is far better to have available a method to check that data are correct and usable (Khan, Fournier, Mauch, Patire, &

Skabardonis, 2020), and so a certain level of redundancy is of critical importance for any real decision-making.

For freeway traffic, one simple and implementable strategy could be to focus not on links but on major freeway interchanges. Rank all the interchanges that serve the greatest volume of traffic, and then instrument each interchange to comply with Guideline 3, thus implementing a set of graph cuts to cover the possible routes, while ignoring Guideline 8. The benefit would be to get the maximum information about route flows at places where route flows can change due to congestion, incidents, and planned events.

4. OBSERVATIONS ON EXISTING DEPLOYMENT OF COUNTING SENSORS

This section provides a review of the existing sensor deployment within California. It begins with a high-level description of PeMS, Census, and Weigh-in-Motion (WIM) sensors. It then synthesizes some basic characteristics of typical sensor configurations. Finally, some challenges involving sensor coverage and redundancy are discussed. These include:

- A common lack of sensing on freeway auxiliary lanes, thus preventing a proper accounting of the complete cross-section of vehicular flow
- Inconsistent coverage of major freeway-freeway interchanges
- Possible redundancies in mainline coverage where the information gain from the installed sensors is minimal given the presence of other nearby sensors and the possibility of data fusion with third-party vendor data

4.1. EXISTING SENSOR COVERAGE

This section presents a summary of the existing sensor deployment within California. This includes reviews of the following data sources:

- Traffic sensors connected to the Performance Measuring System (PeMS)
- Sensors used by the Traffic Census Program
- Weigh-In-Motion (WIM) stations

4.1.1. PEMS STATIONS

Sensors connected to the PeMS network are used to monitor traffic continuously. These sensors primarily consist of inductive loop detectors embedded in the pavement but also include other types of detection technologies such as magnetometers and radars, where the use of inductive loops proved to be difficult. Figure 4-1 to Figure 4-4 show the extent of detection coverage within the various regions of California.

Key observations from the sensor deployment are as follows:

- Detection stations are mainly deployed along freeways, with some additional stations along key urban and rural state routes.
- There are no detection stations in the northern part of California. The northernmost sensors are near Orland along Interstate 5 (I-5) and Chico along State Route 99 (SR-99).
- Several Caltrans districts have no PeMS sensors. These are typically rural districts. They include District 1 (Eureka), District 2 (Redding), and District 9 (Bishop).
- Freeway stations generally include sensors located on the mainline
- Sensors are often installed on freeway on-ramps and off-ramps in urban areas. However, not all interchanges will typically have ramp sensors. Some interchanges may have on-ramp sensors only, off-ramp sensors only, or no sensors at all.

It is currently estimated that PeMS is connected to over 40,000 individual detectors. However, due to various reasons, data is not typically collected from all sensors. As a result of a range of potential technical issues, sensors may stop operating. While there is a goal to try to repair non-operating sensors, months or years can elapse before problematic sensors are repaired.

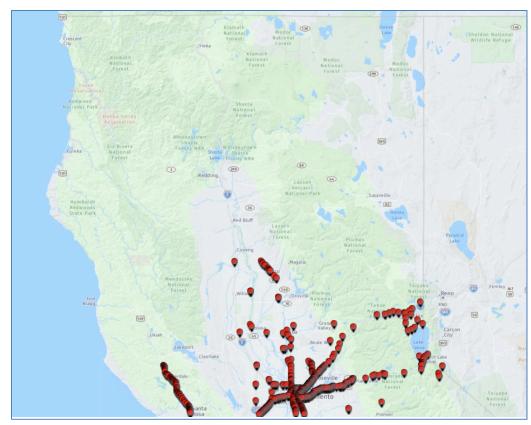


Figure 4-1: PeMS Sensors – Sacramento and Northern California Area

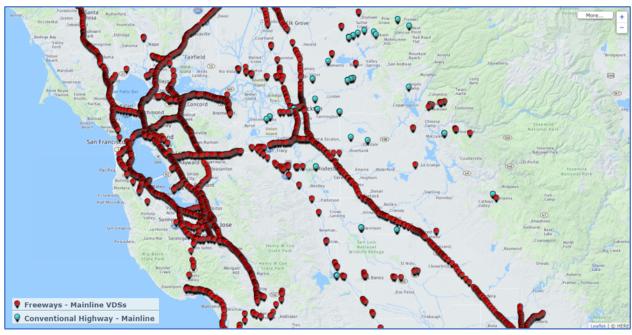


Figure 4-2: PeMS Sensors – San Francisco Bay and Northern Central Valley Areas

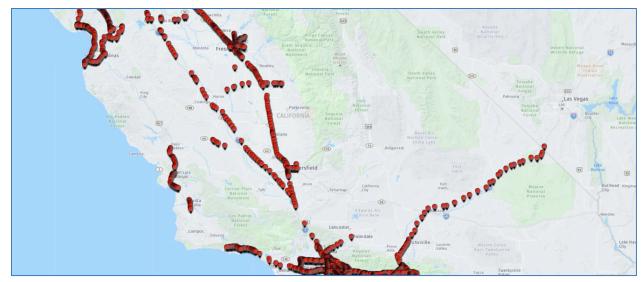


Figure 4-3: PeMS Sensors – Central Coast and Southern Central Valley Areas

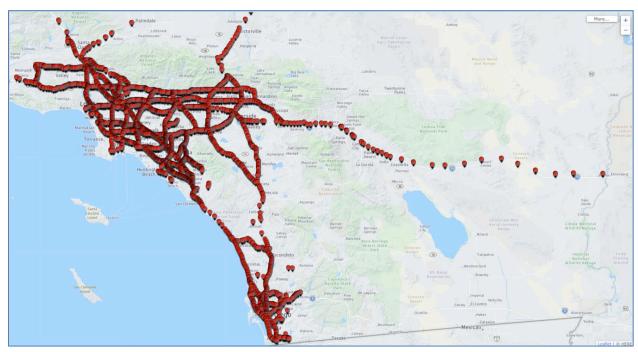


Figure 4-4: PeMS Sensors – Los Angeles, San Diego, and Southland Areas

4.1.2. TRAFFIC CENSUS STATIONS

To support various federal and state reporting programs, Caltrans conducts traffic counts at various locations within the state in addition to using data from PeMS stations. Figure 4-5 illustrates the locations where Census counts are typically made. These include approximately 7,100 count locations spread along freeways and state routes in urban, suburban, and rural areas.



Figure 4-5: Traffic Census Locations within California

Depending on the location, observed data may be available every day or for only specific days during the year. This is due to how the Traffic Census program operates. Due to limited resources, traffic counting is typically performed using electronic instruments that are moved from one location to another in a program of continuous traffic count sampling that aims to revisit each important location every three years. Only stations with strategic importance will have permanent counting instrumentations. On the other end of the spectrum, remote count locations may only be revisited when a notable change in traffic is assumed to have occurred. This results in some remote stations not to have been revisited for over 10 years. For the years when a traffic count is not performed at a given location, either the previous year's data is used and reported, or growth factors calculated using control station data are applied and an estimated volume is generated and reported.

4.1.3. WEIGH-IN-MOTION STATIONS

To monitor truck traffic, Caltrans maintains a network of weigh-in-motion stations spread across the state. Figure 4-6 presents a photo of a typical station. These usually consist of bending plates on frames embedded in concrete. As a vehicle travels over the plate, the weight associated with each axle is determined based on the degree of bending in the plate. This can be done while the vehicle is traveling at normal traffic speed. Inductive loops are further installed before and after the WIM sensor array to measure vehicle speed and overall vehicle length.



Figure 4-6: Weigh-in-Motion Station at Loleta on US-101

Caltrans currently maintains a network of 114 weigh-in-motion stations spread across all districts. Stations are typically installed along well-known heavily truck-traveled routes, typically in rural areas or at the edge of urban areas.

4.2. TYPICAL FREEWAY SENSOR PLACEMENTS

This section provides a brief assessment of potential issues associated with existing sensor placement along freeways. This review focuses on identifying locations with the following conditions:

- Freeway mainline
- HOV/ High-Occupancy Toll (HOT) lanes
- Ramps
- Connectors

The review is based more specifically on a visual assessment of detector placements along the following freeways:

- District 3: I-5, I-80, I-205, US-50, and SR-99
- District 4: I-580, I-80, US-101
- District 6: I-5, SR-180, SR-199
- District 7: I-210, SR-91
- District 8: I-10, I-15, I-40
- District 11: I-5, I-805, SR-94

4.2.1. FREEWAY MAINLINE COVERAGE

Most urban freeways have a relatively high density of mainline sensors. The most common setup in a given direction is to have sensors between the off-ramp and on-ramp of an interchange, typically at some

short distance upstream of the on-ramp, as shown in Figure 4-7. The high frequency of this setup comes from the use of the mainline sensors to support dynamic on-ramp metering, where the rate at which vehicles are allowed on the freeway is adjusted based on observed traffic conditions on the freeway. Since most urban freeways have ramp meters, the presence of mainline sensors near each interchange is therefore generally expected in such an environment.

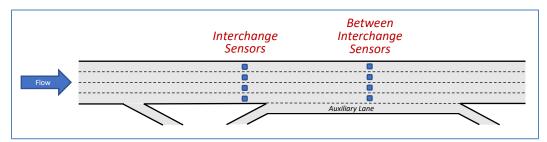


Figure 4-7: Typical Freeway Mainline Sensor Placements

In addition to sensors at a freeway interchange, mainline sensors may also be located between ramps. The primary purpose of these sensors is to collect information on traffic between interchanges.

In general, as shown in Figure 4-7, auxiliary lanes are not typically covered by sensors, regardless of their length. This is problematic for use cases relying on the ability to obtain complete traffic flows across freeway cross-sections as the current setup will not necessarily capture all the freeway through traffic, particularly where the auxiliary lane is used as a passing lane by the through traffic. In addition, it causes discrepancies between the number of lanes in PeMS meta-data and the number of lanes in Google Streetview, or traffic models used to simulate the freeway segments in question.

4.2.2. HOV/HOT LANE COVERAGE

Sensor placement along HOV lanes typically follows the placement of mainline sensors. As illustrated in Figure 4-8, HOV lane sensors are usually placed as an extension of the mainline setup.

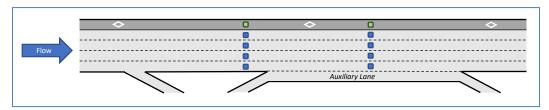


Figure 4-8: Typical HOV Sensor Placement

4.2.3. RAMP COVERAGE

Typically, on-ramps with active ramp metering have installed traffic sensors. For the other on-ramps and off-ramps, the presence of sensors depends on the freeway section:

- Some sections will have sensors installed on virtually all on-ramps and off-ramps. These are typically urban freeways with significant congestion.
- Some sections may have sensors installed only on on-ramps or off-ramps.
- Some sections may have irregular partial deployment at selected on-ramps and off-ramps.

• Some sections may have no ramp sensors at all. These tend to be rural sections but can also be urban sections.

4.2.4. CONNECTORS

Key urban freeway-to-freeway interchanges tend to have sensors installed on their various connectors, while rural interchanges tend not to have sensors. However, this rule is not uniform, as some urban interchanges have partial coverage and others have no coverage at all.

4.3. CHALLENGES WITH DETECTOR PLACEMENT

This section provides a brief assessment of potential issues associated with existing freeway sensor placement along freeways. This review focuses on identifying locations with the following conditions:

- Mainline sensor redundancy
- Partial ramp coverage
- Interchanges with inadequate coverage

4.3.1. MAINLINE COUNT REDUNDANCY

Sensor redundancy defines situations where multiple freeway sensors are expected to give identical counts. A typical situation would be the one illustrated in Figure 4-9, where two or more sensors exist between two freeway interchanges. In this case, because traffic cannot enter or exit the freeway between the sensors, the same volume would be expected to be produced by each sensor. If the main purpose behind the sensor installation is to provide volume counts between the two ramps, then a single location would suffice. A redundancy can therefore be assumed to exist in geometric configurations shown in Figure 4-9.

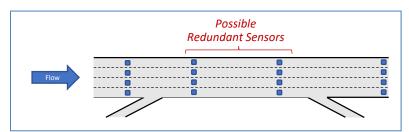


Figure 4-9: Examples of Redundant Sensors

A review of the freeways identified in the introduction to this section indicates that while sensor redundancy is not a widespread issue, it is nonetheless an issue that can affect a sizeable number of locations. Figure 4-10 and Figure 4-11 illustrate a few examples of potential redundancies across various segments. The diagrams are taken from the detector strip maps produced by PeMS. Since the diagrams do not represent ramps without sensors, these were manually added and shown in light grey. In each diagram, the redundant sensors from a count standpoint are marked by a red circle. For each segment, redundancy was determined by verifying sensor locations using aerial photos from Google Maps or snapshots from Google Streetview.

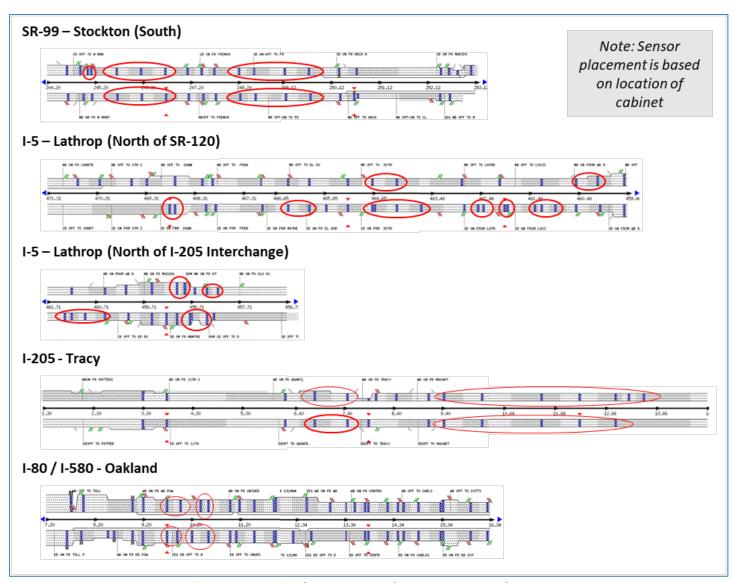


Figure 4-10: Typical HOV Sensor Placement – Example Set 1

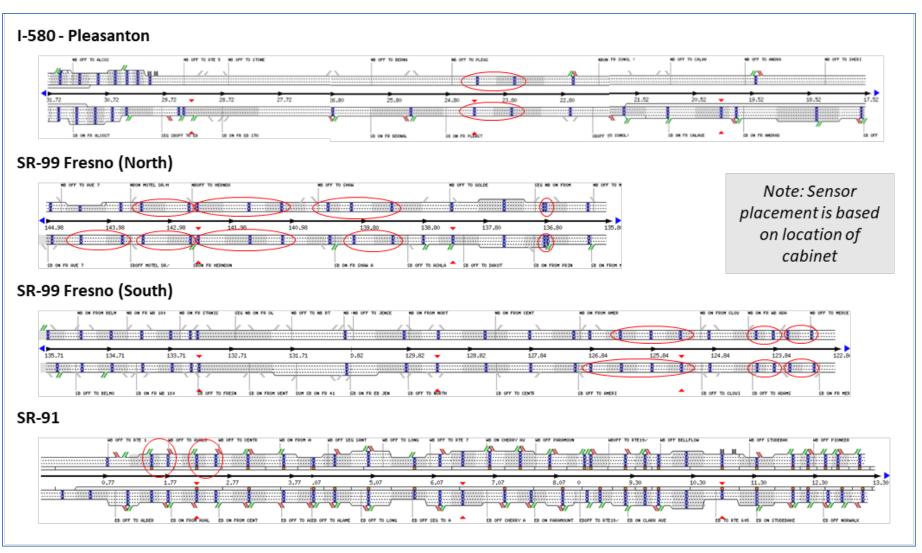


Figure 4-11: Typical HOV Sensor Placement – Example Set 2

As can be observed, sensor redundancy varies significantly across the various sections reviewed. Some sections, like I-80/I-580 in the middle of the figure and SR-91 at the bottom, have very few potentially redundant sensors. Other sections, like various SR-99 sections, have a significant number of potential redundancies.

While all the illustrated freeway sections in Figure 4-10 and Figure 4-11 are in urban or suburban areas, the issue of sensor redundancy is not limited to these areas. Figure 4-12 illustrates four rural freeway sections with potentially redundant sensors. These include I-5 near Buttonwillow, SR-99 north of Bakersfield, I-10 east of Palm Spring, and I-15 through the Mojave Desert. All the locations indicated by red circles are freeway segments that have multiple sensors between successive on-ramps and off-ramps.

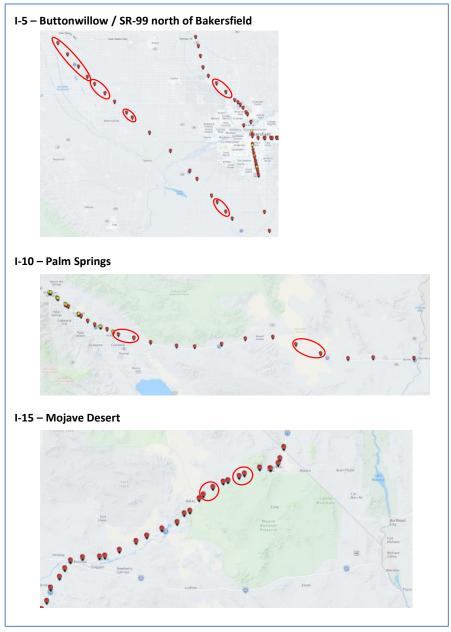


Figure 4-12: Rural Freeway Sections with Potentially Redundant Sensors

In some cases, the apparent redundancy may be explained by one of the following reasons:

- Desire to monitor in some detail the location of congestion between interchanges or ramps. In this case, while the multiple sensors may provide similar counts, the interest may be in monitoring the speeds returned by each sensor to assess the location of the back of a queue or congested area.
- Installation of a new set of sensors next to an old set of sensors without deactivation of the old set within the data collection system.

4.3.2. PARTIAL/NO RAMP COVERAGE

Many freeway sections have partial to no ramp coverage. One example is provided in Figure 4-13. The figure illustrates sensor placement along the section of I-580 leading to the Richmond-San Rafael Bridge in the San Francisco Bay Area. On the western portion of the section, sensors in the eastbound direction only cover on-ramps while sensors only cover off-ramps in the westbound direction. On the eastern section, several ramps are not covered at all. While the numerous mainline sensors enable an analyst to build a picture of traffic along the freeway itself, the partial ramp setup makes it difficult to fully evaluate traffic entering and exiting the freeway.

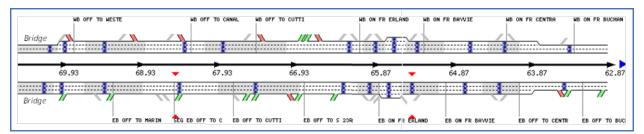


Figure 4-13: Ramp sensors along the Richmond section of I-580

One potential justification for the lack of sensors at some ramps may be associated with the low volumes expected for those ramps. If a ramp is to serve only a small number of vehicles daily, not knowing how many vehicles use the ramp will then have a relatively limited impact on traffic analyses. However, a lack of sensors at interchanges carrying significant traffic volumes is more problematic if there is an interest in analyzing traffic movements on a larger scope than the freeway mainline.

Within the example of Figure 4-13, the lack of sensors on some of the corridor's on-ramps and off-ramps mainly limits the ability to understand where traffic is entering and exiting the freeway where there are multiple ramps between two adjacent mainline sensors. Where a single ramp exists, it is sometimes possible to determine the ramp volume by comparing the volumes recorded at the two nearby mainline sensors, as long as the data are accurate.

Figure 4-14 illustrates three additional examples of key interchanges without sensors. In the three illustrated cases, the lack of sensors on the interchanges with regional state routes limits the ability to assess how traffic from the various state routes interacts with the freeway traffic.

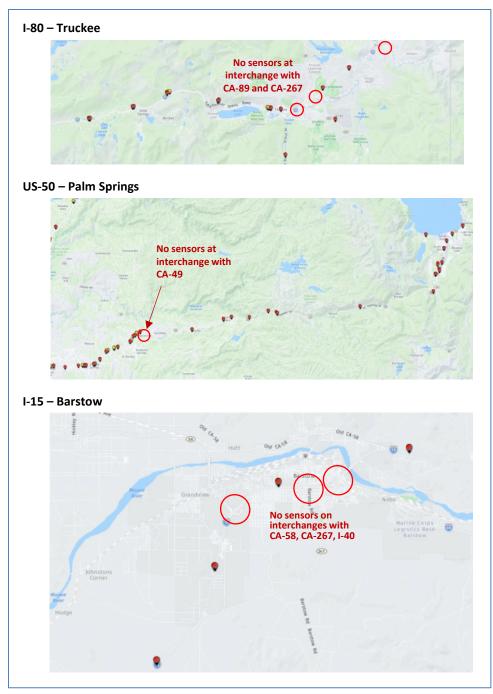


Figure 4-14: Rural State Route Interchanges without Sensors

4.3.3. PARTIAL/NO FREEWAY-FREEWAY INTERCHANGE COVERAGE

Similar to ramp coverage, the deployment of sensors within freeway-to-freeway interchanges can vary significantly. Some interchanges have sensors on all connectors, while others have partial or no coverage. An example is provided in Figure 4-15, which illustrates sensor placement along the section of SR-91 in Los Angeles extending from the I-710 to the I-5 freeways. As indicated, this section features a freeway-to-freeway interchange with no detector coverage (the interchange with I-710), an interchange with

missing coverage on some of the connectors (the interchange with I-605), and a fully covered interchange (the one with I-5).

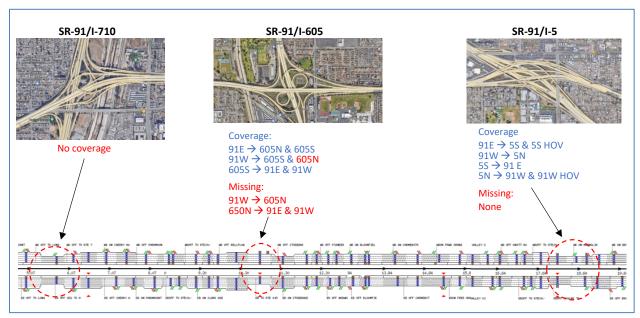


Figure 4-15: Sensor Coverage at Three Freeway Interchanges along SR-91

5. SELECTION OF FREEWAYS FOR ANALYSIS

This section summarizes the investigations that were made to identify representative corridors for detailed analysis. The objective was to select a set of corridors with adequate availability of quality data covering typical urban, suburban, and rural environments. Particular attention was also paid to including facilities with a high-occupancy vehicle (HOV)/ high-occupancy toll (HOT) lane, major freeway-to-freeway connectors, and other significant on-ramps and off-ramps. The overall goal was to select a useful set of facility types with significant interest to Caltrans.

Based on the previous analyses, and discussions with Caltrans staff, the following corridors were selected for project analyses:

- I-880, I-680, and I-280 corridors in the San Francisco Bay Area.
- I-5, SR-99, and I-205 in the area around Modesto, Tracy, and Stockton.
- I-5, SR-91, SR-55, and SR-57 around the Anaheim triangle.

Details of the analyses that were conducted to arrive at the above selection and provided below. This is split into an initial analysis focusing on the quality of the returned data by stations across a corridor, followed by a more detailed assessment of traffic statistics across successive stations.

5.1. INITIAL ANALYSIS

The initial analysis for corridor selection focused on a simple review of PeMS data quality. This was done by outputting the detector health summary report produced by PeMS for the various LDS stations along each corridor of interest. An example of data query setup is shown in Figure 7-7. Since PeMS can only do this analysis for a single day, reports were typically extracted for a given weekday in October or November 2022. Some variations occurred in the selected dates across corridors due to sensors coming online or offline. The goal was to try to select a day with a maximum number of sensors operating correctly.

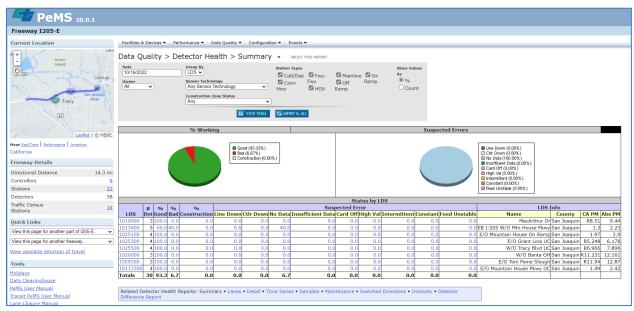


Figure 5-1: PeMS Detector Health Summary Report Example

Data were extracted for most freeways in the San Francisco, Sacramento, Central Valley, and Los Angeles areas. For each corridor, a station was assumed to produce good data only if it returned at least 80% of observed data. This means that only stations where at least 4 out of 5 sensors working correctly were assumed to produce good data. Conversely, any station where one sensor is not working out of two, three, or four was considered as returning bad data.

In the analyses, an observation threshold lower than 80% was not considered due to potential issues associated with PeMS data imputation method. While the data imputation method coded with PeMS often produces reasonable estimates if only one sensor is not working, it also often produces unrealistically high or low estimates when several sensors in succession are not working. The focus was on finding sections with a high number of stations showing good data. It was not to find segments where all stations work perfectly, but segments where a majority of sensors return good data.

Figure 5-2 to Figure 5-5 illustrate the result of the analyses. Each figure only illustrates the corridors for which segments with good data were identified. Segments with the highest proportions of good sensors are shown in dark green. Segments in lighter green represent sections with a fair ratio of good sensors but with some problematic sections. Sections shown in dark orange are those that were deemed to return insufficient data. Within each corridor, the yellow circles represent the freeway-to-freeway interchanges, while the blue circles represent interchanges with non-freeway state routes or key major arterials.

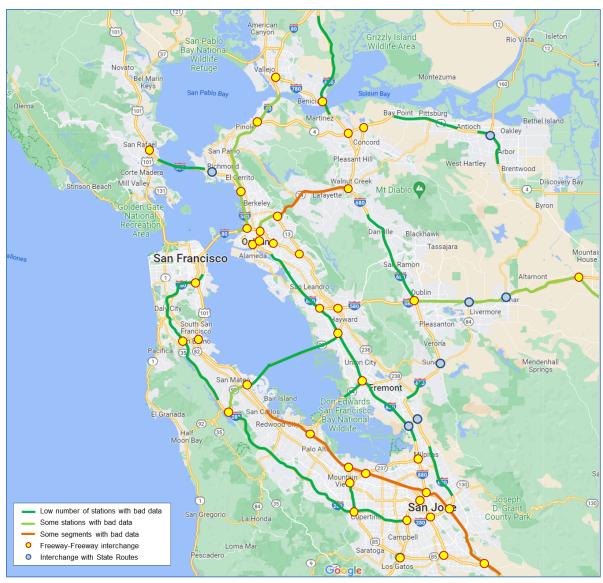


Figure 5-2: Corridor Data Quality Analysis – San Francisco Bay Area

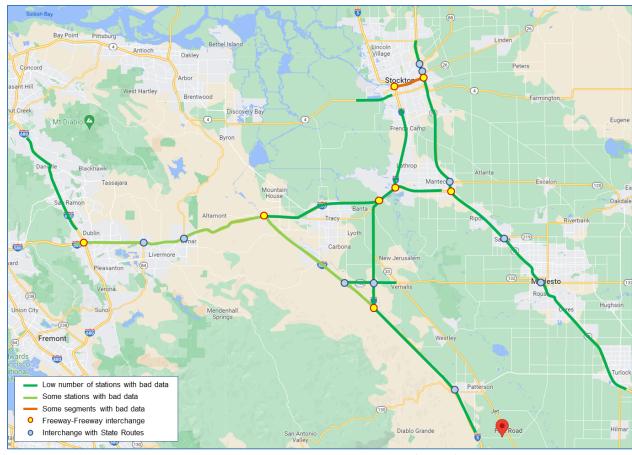


Figure 5-3: Corridor Data Quality Analysis – Stockton/Tracy/Modesto Area

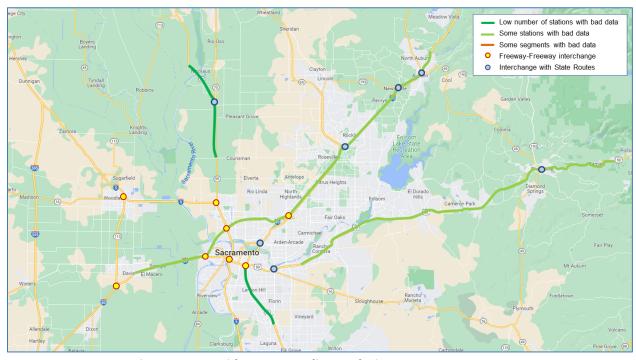


Figure 5-4: Corridor Data Quality Analysis – Sacramento Area

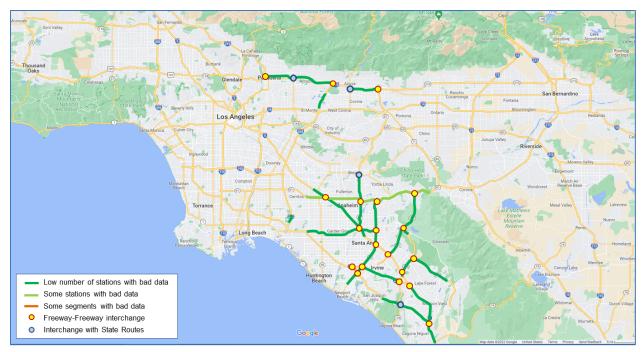


Figure 5-5: Corridor Data Quality Analysis - Los Angeles Area

5.2. DETAILED CORRIDOR DATA QUALITY ASSESSMENT

The second step of the analysis focused on a review of the spatial flow variability across each of the corridors that have been flagged as potentially having good data. As illustrated in Figure 5-6, this was done by mapping on a graph the volumes returned by each station along a freeway, while distinguishing sensors producing good and bad data, still using the 80% observed data threshold to categorize good and bad stations.

Figure 5-6 to Figure 5-10 show the diagrams that were produced for I-5 around Anaheim, SR-99 around Modesto and Stockton, I-880 and I-650 in the San Francisco Bay Area, and I-205 through Tracy in the northern Central Valley. Each graph shows the observed/estimated flows over a particular hour, in this case, either 12:00 Noon or 5:00 PM.

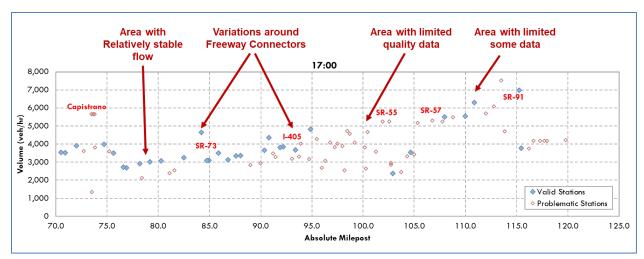


Figure 5-6: Spatial Corridor Data Analysis – I-5 North, Anaheim Area

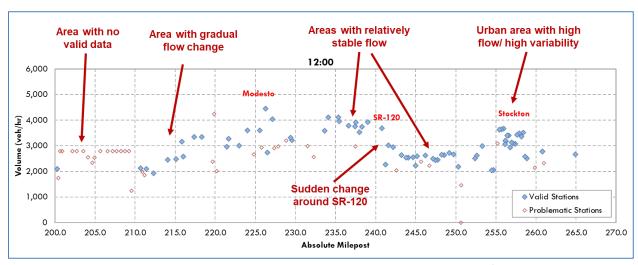


Figure 5-7: Spatial Corridor Data Analysis – SR-99 North, Modesto/Stockton

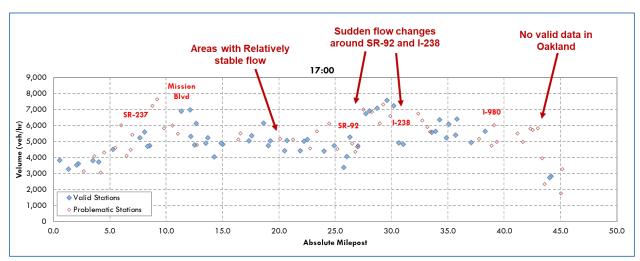


Figure 5-8: Spatial Corridor Data Analysis – I-880 North, San Francisco Bay Area

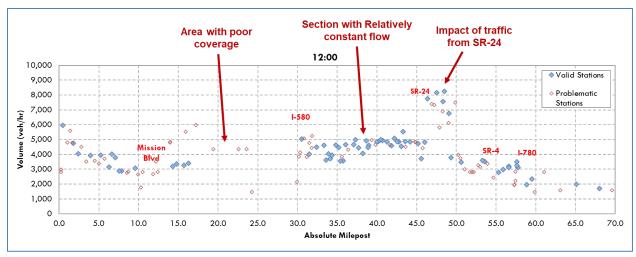


Figure 5-9: Spatial Corridor Data Analysis – I-680 North, San Francisco Bay Area

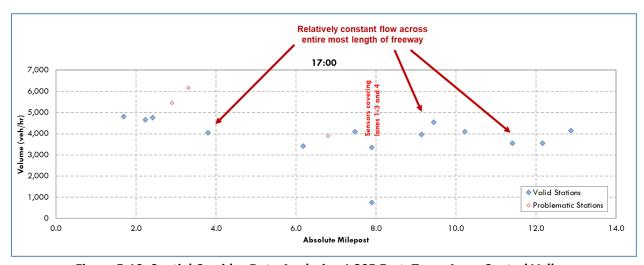


Figure 5-10: Spatial Corridor Data Analysis – I-205 East, Tracy Area, Central Valley

The focus here was to use the illustrated data to identify sections of potential interest for further analyses. A particular interest was to map sections with relatively constant flow across stations, which can be expected in rural areas (such as along some sections of SR-99 in rural areas) or on urban/suburban freeways used mainly as pass-through facilities (such as the I-205 in Tracy), as those sections have a high probability of having redundant sensors. Identifying sections with highly variable flows was also important, as these might be sections for which a higher density of sensors may need to be kept.

6. CONSIDERATIONS FOR SENSOR PLACEMENT

This section provides considerations for sensor placement along freeways. Sensors under consideration are fixed infrastructure-based sensors such as loops, or radar devices meant to measure vehicle flows and that may also measure or estimate speeds and road occupancy. Use cases under consideration may include real-time operations, performance monitoring, modeling, and emerging applications.

Although not discussed in detail, this research did touch lightly on vehicle classification and the specialized weigh-in-motion (WIM) sensors that perform that data collection. Classification data and other flow data can naturally be used together to draw inferences about the movement of trucks that might not be measured directly. This kind of use case is just one example that illustrates the utility of combining multiple data sets.

The work documented in this report took a unique big-picture approach, taking a holistic view of multiple applications while considering the broad context of Digital Transportation Infrastructure (DTI). Digital infrastructure, as opposed to physical infrastructure, refers to data, communications, and computational elements needed to manage the transportation system and support its burgeoning transformation to a connected, automated, and electrified system. In this context, data should be useful, trustworthy, and reusable. It should also be possible to measure elements of interest once and subsequently use the resulting data many times. This work explicitly considers the need for combining data sets to synthesize better information for better decision-making.

6.1. SUMMARY OF USE CASES

This section describes the set of use cases that were considered during the project to prioritize the importance of infrastructure-based sensors. Two operational use cases include ramp metering and integrated corridor management. Many other cases focus on performance monitoring, such as HOV degradation reporting, Census reporting, Mobility Performance reporting (MPR), and truck movement monitoring. Additional uses include traffic modeling for either planning or operations, and emerging applications that employ data fusion.

6.1.1. RAMP METERING

For ramp metering, the goal of sensing is to collect enough information about prevailing conditions around each ramp to set or select an appropriate local metering rate to achieve a given desired operational goal for the facility. Sensing may support various metering algorithms ranging in complexity. Depending on the algorithm, system-wide and local traffic data may be needed in addition to time-of-day information to set the appropriate metering rate.

Typical deployment patterns for sensors in California employ mainline sensors, on-ramp sensors at the metering stop lane to monitor arriving and departing vehicles, and queue sensors near the upstream beginning of the ramp to relax ramp metering if queues threaten to impede traffic on local arterials. Many ramps will also have additional sensors near their downstream end to count traffic, although these are typically not used for metering purposes. All the sensors used on a ramp must provide accurate, real-time data for the ramp metering algorithms to work as intended.

6.1.2. INTEGRATED CORRIDOR MANAGEMENT

Integrated Corridor Management (ICM) refers to a system for proactive traffic management that may be deployed to achieve a wide range of operational goals. Possible goals for sensing in this context include congestion monitoring, incident detection, incident verification, incident impacts analysis, post-incident traffic incident management (TIM) performance analysis, support for decision-making, and response plan deployment. Response plans may include temporary operational modifications to ramp metering or nearby traffic signals to mitigate congestion during planned or unplanned events.

In this context, typical deployment patterns for sensors will include what is already needed for ramp metering but may also include additional sensors on the freeway mainline, off-ramps, HOV lanes, freeway collector/distributors, and local arterials used as alternate routes.

6.1.3. HOV DEGRADATION

The goal of HOV lanes is to encourage carpooling. States also use them to promote the adoption of cleanair vehicles, as exemplified by the carpool sticker program in California that allows vehicles that do not meet the standard HOV lane requirements to use them for some years. However, an HOV lane is only successful if it actually yields time savings for travelers. To ensure that HOV lanes maintain a certain level of performance, State Departments of Transportation are required to monitor their operational performance and submit yearly operational reports about performance degradation to the Federal Highway Administration (FHWA). According to subsection (d) of 23 U.S.C. § 166, an HOV facility is considered 'degraded' if the average traffic speed during the morning or evening weekday peak hour period is less than 45 mph for more than 10 percent of the time over a consecutive 180-day period.

To perform the required monitoring, sensors are typically deployed along HOV facilities. Deployment of HOV sensors may depend on the Caltrans district and the configuration of the HOV lanes themselves. For example, in Caltrans District 7, HOV lanes are designed as limited access facilities with traffic only allowed to enter or exit at certain gates. This theoretically limits the number of sensors required for performance assessment to key locations between gates. However, HOV sensors are often placed in the middle of the gates. Such sensors do not measure HOV inflow or outflow at the gate, but rather produce a count value that is confounded by lane-changing of vehicles into and out of the HOV lane or lanes. On the other hand, HOV lanes in Caltrans District 4 mainly have continuous access and are active only during specific portions of the day, reverting to general use outside of restricted hours. In this case, a higher density of detectors is usually used as traffic conditions can change anywhere. This has resulted in the deployment of HOV detectors that are typically configured in line with the detectors of the general-purpose lanes.

6.1.4. CENSUS

Census monitoring is another federal requirement that supports the allocation of federal transportation funding. In this case, the goal of sensing is to take a statistical sample of traffic volumes across the state over a range of facilities to estimate average annual daily traffic (AADT) statistics and quantify total yearly vehicle miles traveled (VMT) across the State Highway System (SHS).

The deployment of sensors for this purpose need not be continuous, such as 24 hours per day across 365 days per year. Within California, data is typically collected over a three-year cycle, with data collected at each site of interest once every three years across a few days. While data at some locations may be collected continuously, these sites are a slim minority. Sites with low traffic may further only be sampled

when noticeable changes in traffic are observed, resulting in long intervals between updates. This kind of information, having broad coverage, may have other useful applications when combined with other data.

6.1.5. MOBILITY PERFORMANCE REPORTING

In prior years, the Mobility Performance Report (MPR) was generated quarterly using outputs automatically generated from the Performance Measurement System (PeMS). The report summarizes key performance measures such as VMT (vehicle miles traveled), VHT (vehicle hours traveled), and VHD (vehicle hours of delay). The goal of sensing in this context is to obtain an overall picture of system performance over each district.

Interestingly, the MPR only uses PeMS data from mainline and HOV sensors and ignores data on adjacent freeway facilities such as on-ramps, off-ramps, and connectors. One previous study (Khan, Fournier, Mauch, Patire, & Skabardonis, 2020) estimated that this methodology neglects 15-20% of Caltrans right-of-way (ROW) lane-miles in urban freeways.

6.1.6. TRUCK MONITORING AND PROJECTED MAINTENANCE

There are multiple goals for truck monitoring including the enforcement of weight limits, projection of overall wear and tear on the road to project future requirements for maintenance and pavement rehabilitation, and design of road upgrades or new roads for accommodating changing truck traffic.

One goal of sensing in this context is to perform vehicle classification. Another is to provide some underlying data to quantify truck movements that can be used to predict future travel needs and maintenance costs.

Within California, vehicle classification is typically obtained from the two following sources:

- Weigh-in-motion (WIM) stations installed across the state
- Census traffic sensing stations with classification capabilities

All mainline WIM sensors used by Caltrans are bending plates on frames embedded in concrete. As a vehicle travels over the plate, the weight associated with each axle is determined based on the degree of bending in the plate. This can be done while the vehicle is traveling at normal traffic speed. Inductive loops are further installed before and after the WIM sensor array to measure vehicle speed and overall vehicle length.

WIM stations typically gather and store data on a 24/7 basis. This is accomplished in the roadside cabinet. Information captured at the WIM stations is then automatically sent to a data management system hosted on a Caltrans server that allows data to be queried based on location or date. Information typically collected from each passing truck by WIM stations includes:

- Axle spacing
- Axle weights
- Gross vehicle weight
- Caltrans vehicle classification
- Vehicle speed
- Vehicle overall length
- Weight violation flag

- Day/time of observation
- Direction of travel
- Lane of travel

Fairly comprehensive data is also obtained from Census stations with vehicle classification capabilities. This includes vehicle classification counts by hour, day of the week, day of the month, direction of travel, and lane of travel. Weights are not obtained due to the lack of capability for measuring this characteristic.

6.1.7. MODELING

There are many different types of traffic models built for different purposes. Planning models (typically static, macro-level models) benefit from the types of data discussed here but are not the focus of the present work. Dynamic traffic models are increasingly built at larger scales and are needed to create so-called "digital twins" for emerging transportation applications. Dynamic traffic models may be used to evaluate the benefits of proposed transportation projects, select desired alternative scenarios, or be used as a permanent component of an ICM project.

The goal of sensing in this context is to provide underlying data to build and calibrate models so that they can recreate realistic congestion and travel patterns. Typical data will include flows, speeds, travel times, and turning ratios. As models get geographically larger, there are specific challenges around routing, especially when multiple routing options are feasible and driver choices are influenced by localized congestion, especially at major freeway-freeway interchanges and decision points.

6.1.8. EMERGING USES

Emerging applications for sensor data relate to the burgeoning digitization of transportation, and new possibilities to manage the transportation system at an unprecedented level to achieve strategic goals such as improved safety as well as targets for equity and environmental sustainability.

One crucial use for sensor data is to provide normalization factors, and quality checks on private sources of transportation data—from so-called third-party sources. Private companies provide analyses of travel times, routing, turning ratios, and other metrics based on data collected from probes, connected vehicles, fleet telematics, and other potential sources. However, these data are usually sampled from a limited and likely biased population of travelers, with a bias that is unlikely to change quickly in the coming years.

Unlike tracking data from third-party vendors, infrastructure-based sensors collect a complete cross-section of data across a facility. Instead of relying on data samples, these sensors offer the ability to capture information about every vehicle passing a given location and thus reliable ground truth estimates when they are operating adequately. Sensor data is therefore special and crucially important for the reliable monitoring of transportation systems.

When data from fixed sensors and third-party sources are fused, the potential exists to improve the quality of information available and decision-making outcomes for all the use cases listed above. This is incredibly important. However, this is only possible if enough dedicated sensor data exist, and if the sensor data are made to be trustworthy and reusable.

6.2. DATA CHALLENGES

Referring back to Figure 1-1, the main idea is to be able to benefit from the promise of DTI—to have the ability to gather and exchange data that can be used to manage traffic at the systems level. This is where significant gains can be made in terms of improving traffic flow and safety, reducing vehicle energy use and greenhouse gas emissions, and more. The very first step towards this goal is to generate and have data that are suitable for machine-to-machine interfaces.

Physical sensor placement is inextricable from knowing the sensor placement. In other words, the actual location of the sensor must be known in both the physical and digital worlds. Not only is it crucial for sensors to be positioned in the correct location, but it is crucial to be able to know with high confidence and detail exactly where a sensor is and what it measures: This is the key to reusability and data fusion.

Existing challenges impeding data usage and data fusion include the following limitations on sensor location meta-data:

- Meta-data are insufficient for machine-to-machine interfaces
- Meta-data contain only minimal detail—enough for a human to find and service
- Meta-data are insufficient at major freeway-freeway interchanges
- Meta-data could be wrong
- Few automated checks are implemented to maintain the quality of meta-data
- Data availability can be low

These challenges and their effect on several use cases are described below.

6.2.1. DATA CHALLENGES RELATED TO REAL-TIME OPERATIONS

Typical sensor installations for ramp metering are adequate for local ramp metering control but are often not sufficient for applications requiring more details, or an integration with multiple systems or multiple jurisdictions such as in ICM systems. For instance, data collection from queue sensors located near the upstream beginning of a ramp is typically not available in PeMS or in conjunction with mainline or onramp data.

It is also quite common to have a lack of sensing on auxiliary freeway lanes. This defeats the purpose of mainline freeway monitoring as vehicles can use facilities as a passing lane instead of their intended purpose as entry or exit lanes. The main value of mainline traffic monitoring is being able to measure the entire cross-section of traffic to generate an unbiased estimate of flow at a particular point on the freeway network. This is notably crucial for cross-checking or calibrating third-party data.

Another challenge is that data availability and quality can be highly variable. A particularly useful time to collect data is during construction projects so that traffic can be monitored, and congestion mitigated. However, local electrical power is typically shut off as a safety measure during construction, and without local power, sensors are rendered inoperable. Often, temporary lane shifts also result in traffic passing between sensors instead of on top of them. To compensate for this, Caltrans may install temporary sensing where necessary. Having the possibility to use multiple data sources during construction projects would greatly improve monitoring capabilities when and where they are most needed. Data availability and quality can also be impacted by copper theft, vandalism, or signal loss due to communications, modem, network, server, or other failure. Redundant systems would add a layer of robustness.

Another challenge is the highly variable accuracy of sensor location meta-data over multiple-year timespans. Primarily, this is because there is a lack of automated means for meta-data accuracy checking. Meta-data errors can be introduced during maintenance or after new construction is completed. A typical error is that sensors in the same controller can have their labels swapped. When this happens, a sensor may be assumed to be localized on the other side of the freeway, in the wrong lane, or on an adjacent HOV facility.

6.2.2. PERFORMANCE MONITORING

When it comes to performance monitoring, the main challenges are that the data sources are fragmented and difficult to relate and use together. Depending on whether the data are housed in PeMS, WIM systems, or separate Census systems, access may be limited, and the meta-data may be insufficient to connect the measurement data with their location on a traffic network or a shared data layer where the relationships between the data can be automatically determined. Better information and more accurate reports can be generated when data are integrated with other sources.

Below are some examples of elements that may affect performance monitoring:

- Inaccuracies in meta-data. As mentioned above, monitoring and yearly reporting of HOV lane degradation is a federal requirement. However, meta-data inaccuracies confound the collection of this data. One example of a common mislabeling problem is when the HOV lane sensor is mislabeled as a general-purpose lane sensor. A previous study (Fournier, Farid, & Patire, 2023) estimated that 5% to 8% of HOV sensors in Caltrans District 7 are mislabeled in this way. One potential fix would be to have automated systems to check meta-data accuracy such as those implemented in the above study.
- Unequal coverage of off-ramps, on-ramps, and freeway collectors. Knowing the level of traffic
 coming onto or leaving a freeway, particularly at busy interchanges, helps understand how a
 freeway functions in context. Unfortunately, not all freeways are instrumented the same way.
 While many urban freeways have sensors on all on-ramps and off-ramps, such as the I-210
 freeway in Pasadena, others, such as the I-580 freeway in Richmond, may only have sensors on
 off-ramps or on-ramps. Other freeways may have no ramp sensors.
- Limited number of WIM stations. Only 140 WIM stations have been installed across California, mainly along key transportation corridors used by trucking companies. Most of the stations are located along freeways and major highways in rural areas and at the edge of urban areas. In addition to the limited number of stations, various technical issues may prevent data collection from existing stations. As an example, only 54 of the 140 WIM stations installed across California were successfully transmitting data in September 2022.
- Limited data availability from truck weigh stations. While many weigh stations exist across the state, very limited data is typically obtained from these facilities. Weigh stations are normally operated by the California Highway Patrol (CHP), not Caltrans. While all stations have scales, some facilities may not be operating all the time. Their operational hours are based on need and are typically determined based on average daily truck traffic, peak truck traffic hours, and seasonal needs. In addition, based on discussions with commercial vehicle enforcement facility (CVEF) operators, information about passing trucks is typically only recorded for vehicles flagged with a violation.

6.2.3. MODELING

Traffic modeling, especially mesoscopic or microscopic modeling requires high-quality data for calibration. For large models of extended geographical scope, there are specific challenges related to routing and the selection of routes for origin-destination (OD) pairs where multiple routing options are feasible and affected by localized traffic congestion.

These routing challenges are particularly difficult at major freeway-freeway interchanges with multiple flyovers and idiosyncratic connectivity to local streets. One typically requires having reasonable knowledge of split ratios (the percentage of vehicles that turn or go straight) at network decision points. For this reason, model calibration stands to benefit from having both fixed sensor data as well as third-party data to cross-check and validate the routing choices of modeled vehicles.

7. SENSOR PRIORITIZATION METHODOLOGY

7.1. STAGE 1: PRIORITIZATION BASED ON ROAD TOPOLOGY

This section describes a prioritization framework for sensors based on road topology, and specifically considering freeway-freeway interchanges. It also discusses some user inputs that are required to support the voting process described in the subsequent stage.

To help determine the relative importance of the various interchanges found along a given freeway, the following simple categorization based solely on the number of connectors was developed:

- Type 1: 1-2 connectors
- Type 2: 3-4 connectors
- Type 3: 5-7 connectors
- Type 4: 8 connectors
- Type 5: 9+ connectors

Examples of interchanges belonging to Types 1, 2, 3, and 4 are shown in Figure 7-1. These categorizations do not include at-grade intersections controlled by traffic lights, stop signs, or yield signs.

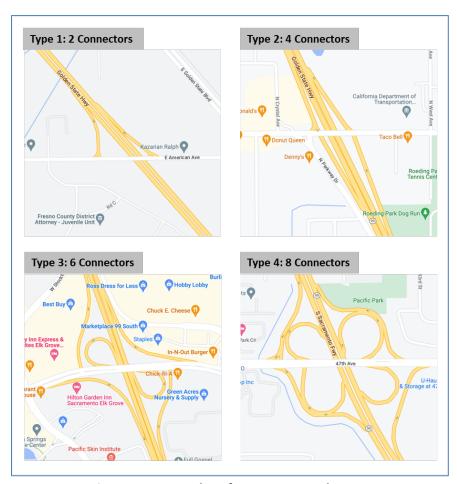


Figure 7-1: Examples of Freeway Interchanges

The overall goal is to prioritize candidate locations for sensors in a simple and straightforward manner whilst considering multiple purposes for the data. When considering interchanges, one useful metric is the number of connectors or ramps that provide connectivity from the subject freeway to the other freeway or local arterials. One challenge with this metric is that it is subject to some level of interpretation. While complex interchanges are often associated with high-traffic locations, some interchanges may exhibit a simple layout due to geometrical constraints, such as diamond interchanges connecting an elevated freeway in a dense urban setting.

In some examples, it might be difficult to decide whether a particular set of on- and off-ramps constitute a specific interchange or are instead part of a nearby interchange. In such a case, some engineering judgment may be required to determine the specific case at hand. Consider the example of Figure 7-2, illustrating the ramps between I-280, I-880, SR-17, and Stevens Creek Boulevard in San Jose. Of particular interest is the illustrated blue path from Stevens Creek Boulevard to Moorpark Avenue. This path utilizes a connector that could be used to take SR-17 south but instead merges onto I-280 before taking the next exit. In this case, the combination of ramps around Stevens Creek Boulevard and the I-280 / I-880 / SR-17 interchange can be considered to belong to a single interchange. In the methodology presented in this document, the general guidance to address such an issue is that if it is difficult to decide whether a particular collection of connectors constitutes one or two interchanges the collection should then be counted as a single interchange.

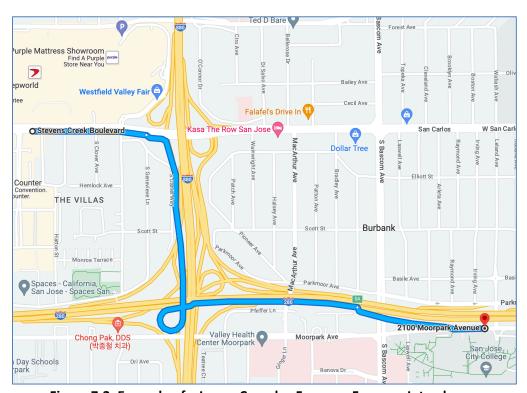


Figure 7-2: Example of a Large, Complex Freeway-Freeway Interchange

Using the above framework, the proposed methodology for categorizing interchanges proceeds as follows:

 Obtain a facilities spreadsheet from PeMS. Be certain it is bidirectional and includes all PeMS sensor types such as HV, OR, FR, ML, etc. This spreadsheet will be used to keep track of the topology features.

- 2. Start at the beginning of the spreadsheet. Using an online map, identify the first interchange and count the number of connectors.
- 3. Associate the PeMS sensors with the interchange; label them with a number starting with one and incrementing upward.
- 4. Add columns for the number of connectors and categorization of interchange type to the spreadsheet.
- 5. Continue this process for each successive interchange, labeling each one.

Note that in general, not all interchanges will be included in the PeMS facilities spreadsheet. However, it provides a good first start. Additional interchanges should be added to the spreadsheet with postmile locations so they can be considered.

A facilities spreadsheet for all sensors in a district can be obtained from the PeMS website (https://pems.dot.ca.gov/) by using the Data Clearinghouse accessed through the "Clearinghouse" link in the lower left of the front page. The Clearinghouse interface is displayed in Figure 7-3. From here one can select the district, by using "district" dropdown list, specify the file type, by selecting "Station Metadata" from the "Type" dropdown list, and click the "submit" button.

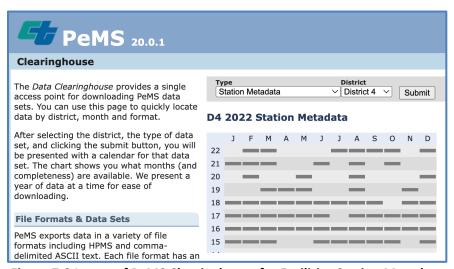


Figure 7-3 Image of PeMS Clearinghouse for Facilities Station Metadata

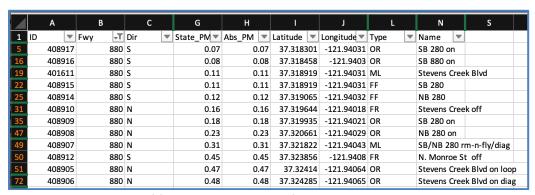


Figure 7-4 Example of facilities spreadsheet for I-880 sorted by absolute postmile

The station metadata is downloaded as a text file. It can be opened in a spreadsheet program such as Excel. At this point one can filter on the freeway (column B), and sort by absolute postmile (column H) as displayed in Figure 7-4.

As described above, additional columns for interchange category, number of connections, and intersection number can be added by the analyst. These additional data are obtained by inspecting an online map. They can be entered into the appropriate rows of the spreadsheet. When this process is completed, the spreadsheet will have multiple rows per interchange.

For visualization purposes, multiple entries corresponding to the same interchange can be dropped and the result can be plotted on a map. Figure 7-5 is the result of this process for I-880 and a portion of SR-99. Large red dots correspond to the largest (Type 5) interchanges and the small green dots correspond to the smallest (Type 1) interchanges. A color gradient is further used to illustrate interchanges associated with intermediate types.

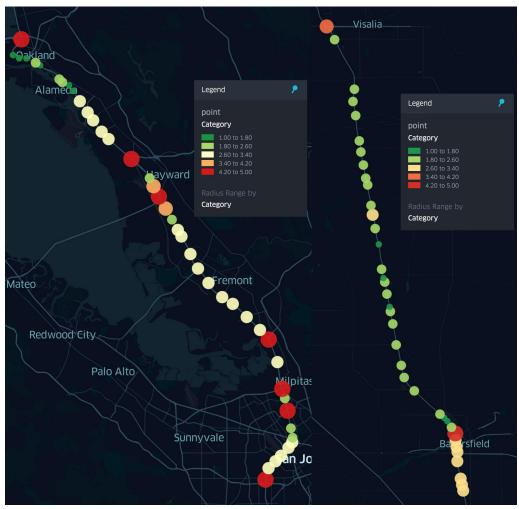


Figure 7-5: Categorization of Interchanges on I-880 (left) and SR-99 (right)

Having performed this analysis, the large red dots correspond to major interchanges where sensor data is valuable and important. However, the string of green dots in Oakland toward the northern end of I-880 is deceptive. In this dense downtown area, there is no space for large interchanges. The freeway is elevated,

and the Posey Tube provides underwater access to Alameda. The connectors and ramps are attached to arterial streets, but this complexity is not captured in this methodology. For this reason, it is important to consider other user input as well as existing sensor data to add more context.

Considering the discussion in the previous section, several use cases may specifically demand supporting instrumentation. Below are a few examples, among potential others:

- If there is a local need for ramp metering, then these interchanges will need to be flagged.
- HOV lanes still require their own dedicated sensors because third-party data does not yet distinguish reliably traffic on separate, but adjacent, lanes.
- If there is a plan for ICM, then additional sensing locations will further need to be flagged in alignment with the goals of the ICM.

All of the above factors must be considered in their local context and flagged. This provides a topological priority for sensing that is considered in the next stage.

7.2. STAGE 2: PRIORITIZATION OF MAINLINE SENSORS BASED ON EMPIRICAL ANALYSIS

This section describes a methodology to select key interchanges for instrumentation. It does this by combining the topological features, above, with an empirical analysis of mainline sensor data. The process involves calculating a set of metrics and then using a weighted sum to vote for the most important sensors. Key steps in the process can be summarized as follows:

- 1. Gather mainline data over desired weekday and weekend periods
 - a. Select one week over each of the spring, summer, and fall seasons
 - b. Select time-of-day periods corresponding to AM peak, midday, and PM peak
- 2. Filter out data where less than 80% of values are actually measured
- 3. Assess hourly flow variations across stations
- 4. Calculate the following metrics for successive sensors along the freeway and organize them in a spreadsheet
 - a. flow changes
 - b. distance to the last station with valid data
 - c. cumulative flow change
 - d. absolute cumulative flow change
- 5. Collect topological priorities from stage one and insert them into the spreadsheet
- 6. For each metric assign a voting weight and calculate the final score

The list of final scores is the prioritization for the mainline sensors on the freeway. By proxy, this corresponds to key interchanges. All of this is described in detail below.

7.2.1. FREEWAY DATA COLLECTION

To evaluate current monitoring capabilities along each reviewed freeway, hourly traffic flows reported by PeMS stations along each freeway are collected using a Time of Day Contours analysis. This data collection

is intended to obtain average daily traffic flow profiles for each station reporting valid data along a freeway. Comparison of the resulting profiles allows assessment of reported traffic flow variations on an hourly basis across successive stations.

Figure 7-6 and Figure 7-7 illustrate the typical analysis setup that was used to obtain average weekday and average weekend profiles. Due to limitations in PeMS analysis capabilities, which restrict time-of-day contour analyses to cover a minimum of two days and a maximum of 7 days, average weekday profiles were obtained by considering data from all weekdays in a single week while average weekend profiles were obtained by considering data from both a Saturday and Sunday in a single weekend.

To assess potential seasonal effects, data were further collected to cover the following periods:

- Spring (typically mid-March)
- Summer (typically mid-July)
- Fall (typically mid-September)

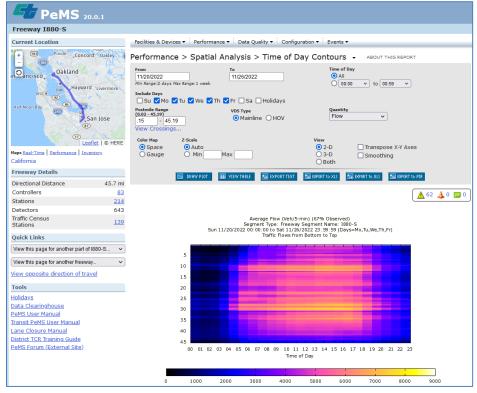


Figure 7-6: PeMS Data Collection Setup Example – Weekday Analysis

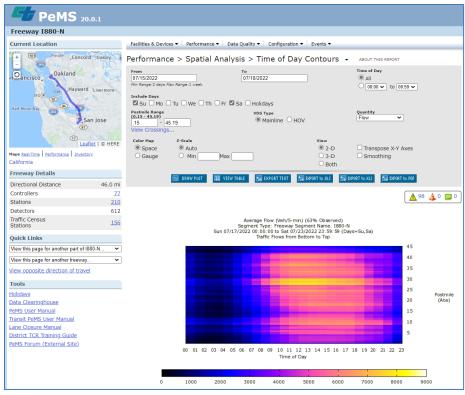


Figure 7-7: PeMS Data Collection Setup Example – Weekend Analysis

Because of detection issues, such as stations suddenly experiencing data reporting issues, data from all freeways were not necessarily collected on the same week or weekend. Efforts were made to select weeks or weekends with good data coverage falling within a relatively close range. Table 7-1 indicates the specific dates from which data were collected for the various analyses conducted as part of this project.

Table 7-1: PeMS Data Collection Periods

Freeway	Weekday Profiles			Weekend Profiles		
	Spring	Summer	Fall	Fall	Spring	Summer
I-880	3/14/2022	7/18/2022	9/24/2022	3/12/2022	7/16/2022	9/24/2022
I-680	3/14/2022	7/18/2022	9/19/2022	3/12/2022	7/16/2022	9/17/2022
I-280	3/14/2022	7/18/2022	9/26/2022	3/12/2022	7/16/2022	9/24/2022
I-205	3/21/2022	7/18/2022	9/19/2022	3/12/2022	7/16/2022	9/24/2022
I-5 District 10	3/14/2022	7/25/2022	9/19/2022	3/12/2022	7/23/2022	9/24/2022
I-5 Central Valley	3/13/2023	07/17/2023	09/19/2022	3/18/2023	07/22/2023	09/17/2022
I-5 District 12	3/7/2022	7/25/2022	9/19/2022	3/5/2022	7/23/2022	9/24/2022
SR-120	3/14/2022	7/18/2022	9/19/2022	3/12/2022	7/16/2022	9/24/2022
SR-91	3/14/2022	7/25/2022	9/19/2022	3/12/2022	7/23/2022	9/24/2022
SR-99	3/14/2022	7/18/2022	9/19/2022	3/12/2022	7/16/2022	9/24/2022
SR-57	3/14/2022	7/18/2022	9/26/2022	3/12/2022	7/16/2022	10/01/2022
SR-55	3/14/2022	7/18/2022	9/18/2022	3/12/2022	7/16/2022	9/24/2022

An example of collected data is shown in Figure 7-8. The figure shows the hourly flows that were returned by PeMS for the fall 2022 analysis period for each station along I-880 North from the absolute milepost 0.31 to the absolute milepost 24.48. The corresponding VDS number is shown on the left in Column C, and the percent of observed data associated with the station over the analysis period is shown in Column D.

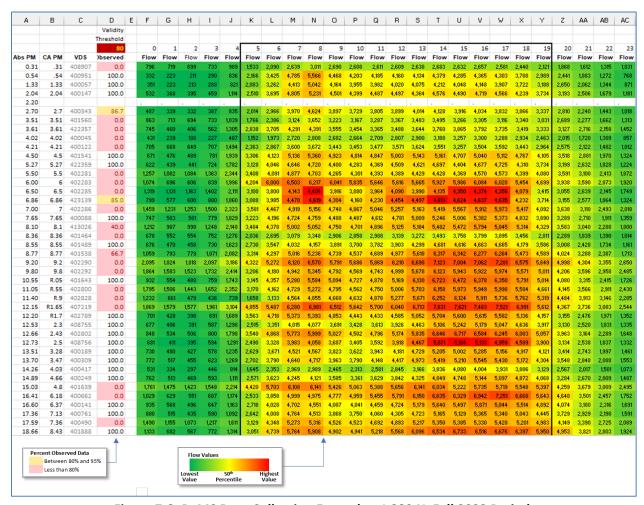


Figure 7-8: PeMS Data Collection Example – I-880 N, Fall 2022 Period

7.2.2. DATA VALIDATION CHECK

The percent of observed data is used in the analyses as a filter to remove stations for which too many measurements may be missing. While PeMS has an internal process to input missing data, this process often results in a significant underestimation or overestimation of measured flows. For the analyses, an 80% threshold was used as a validity threshold, as indicated at the top of Column C in Figure 7-8. This means that data from any station with less than 80% observed values were removed from consideration.

Higher or lower validity thresholds can be used if necessary. Using a threshold higher than 80% could focus the analysis on sensors operating correctly but could also cause large gaps to be produced between valid stations. On the other hand, using a lower threshold could result in considering more stations, but

could also result in inaccurate data at some stations leading to false conclusions. In this case, 80% was deemed as a good compromise between the two approaches.

7.2.3. ASSESSING HOURLY FLOW VARIATIONS ACROSS STATIONS

The first stage of the analysis is a comparison of observed flows across successive detection stations. This analysis is based on the hypothesis that successive stations covering significant variations in flows may provide more valuable information than stations returning similar flow levels as their immediate neighbors.

Figure 7-9 illustrates the result of the data processing for the sample of Figure 7-8. The numbers in each row represent the ratio of flow at the current station compared to the nearest upstream station with valid data. For instance, a value of 1.28 means that the measured flows at a given station correspond to 128% of the observed flows at the previous valid stations. Similarly, a value of 0.84 indicates that flows at a station correspond to 84% of flows at the previous valid station. Ratios shown in Columns AG to BD are for each hour of the day, while the ratio in Column BF represents the average ratio across the daytime period considered, in this case, 5 AM to 8 PM.



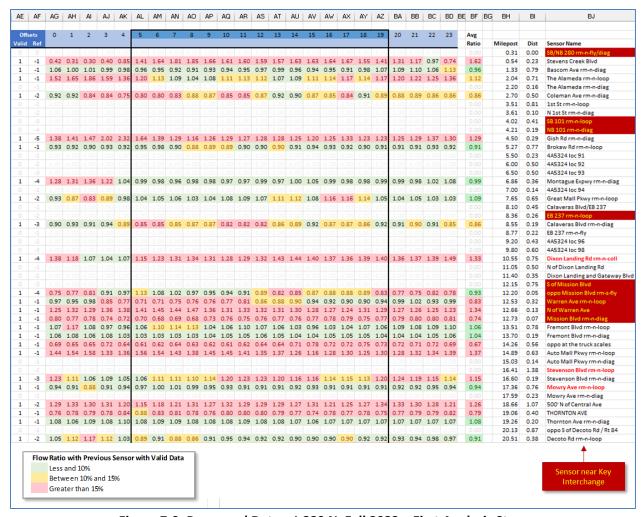


Figure 7-9: Processed Data – I-880 N, Fall 2022 – First Analysis Step

For each row, the reference station used in the calculations may be the one immediately upstream or could be several physical stations away. Data from Column AF identifies the reference station used in the calculations. For instance, a value of "-1" indicates that the station immediately upstream is used while a value of "-2" indicates that the second upstream station is used. Jumps across existing stations happen due to stations considered as returning no valid data based on the selected PeMS data validity threshold, in this 80% of observed values.

On each row for which flow differentials are calculated, the following color scheme is further used to help visualize flow changes across stations:

- Green: Change in flow less than 10% (ratios between 0.90 and 1.10)
- Yellow: Change in flow between 10% and 15% (ratios between 0.85 and 0.90, or 1.10 and 1.15)
- Red: Change in flow greater than 15%

7.2.4. ASSESSING STATION IMPORTANCE

The second step of the analysis focuses on identifying stations meeting one or more of the following relevancy criteria:

- Network Topology: Intersections or sensors considered necessary to monitor traffic regardless
 of the flow level being measured. This includes, among potential others, sensors identified as
 important based on road topology analysis in the methodology of stage one, sensors that
 support ramp metering operations, sensors attached to weigh-in-motion stations, or sensors
 marking the start or end of a corridor.
- Major Flow Change: Sensor capturing a major shift in observed traffic volume with the previous
 upstream sensor with valid data. In the example, this is taken to be a flow differential of 15% or
 more.
- Medium Flow Change: Sensor capturing a moderate shift in observed traffic volume with the
 previous upstream sensor with valid data. In the example, this is taken to be a flow differential
 of 10 to 15%.
- **Distance with Last Station with Valid Data:** Consideration of distance between the current sensor and the closest upstream station with valid data. This is to ensure that a certain maximum distance is not exceeded between successive stations with valid data, i.e., that a certain minimum density of sensors remains considered. The distance threshold can be set based on the environment. In the example, a uniform 15-mile threshold is used for illustrative purposes. However, a shorter 5-mile threshold could be used instead in urban environments, while a longer threshold could be used in rural environments. In practice, this criterion could be set to consider stations without valid data (those with less than 80% observed data) to ensure that a minimum coverage is maintained.
- Cumulative Flow Change: Summation of observed flow changes since the last upstream station marked as important, based on the previously listed criteria. As illustrated in Figure 7-10 this criterion helps capture gradual increases or decreases in traffic flows occurring across several stations, such as what might occur when approaching an urban area. The figure shows flow differentials for 13 successive stations. Station #1 is flagged as important since it is the first in the sequence. Station #7 is then flagged based on the fact that its cumulative flow difference with Station #1 exceeds 15%. This causes a reset of the cumulative differential calculations, illustrated by the down arrow. Station #12 is then similarly flagged based on the fact that its cumulative flow differential with station #7 again exceeds 15%.

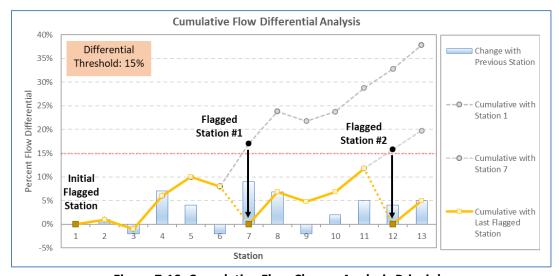


Figure 7-10: Cumulative Flow Change Analysis Principle

• Absolute Cumulative Flow Change: Summation of absolute observed flow change since the last upstream station marked as important. This criterion is similar to the Cumulative Flow Change, except that helps capture spatial flow variations that may not result in gradual flow increases or decreases. An example is shown in Figure 7-11. This figure shows flow differentials for 13 successive stations, with Station #1 flagged based on the fact it is the first in the sequence. In this case, Station #6 is flagged based on the fact that the summation of absolute flow differences across successive stations between Stations #1 and #6 exceeds 15% even though the actual flow differential between Stations #1 and #6 is almost nil. This causes a reset of the absolute cumulative differential calculations, as illustrated by the down arrow. Station #12 is then similarly flagged based on the fact that the summation of the absolute flow differentials between stations #6 and #12 again exceeds 15%.

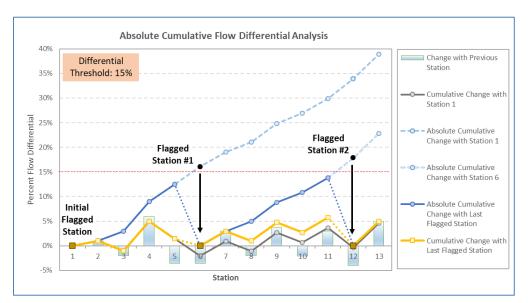


Figure 7-11: Absolute Cumulative Flow Change Analysis Principle

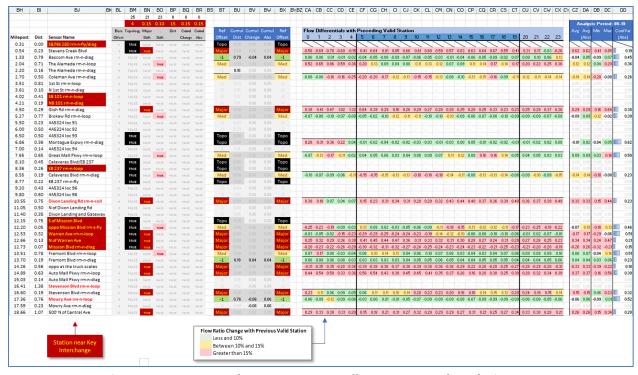


Figure 7-12: Processed Data – I-880 N, Fall 2022 – Second Analysis Step

The result of this analysis for the sample data of Figure 7-8 and Figure 7-9 is shown in Figure 7-12, for an analysis over the 5 AM to 8 PM period. Source spreadsheets are available separately. The descriptions here provide an overview of the spreadsheet layout. Key results of the analysis are shown in the following columns:

• Evaluation Criteria Application Results

- Column BL: Reference station offset, i.e., location of last upstream station flagged.
- **Column BM:** Indication whether a network topology criterion is applied to the station.
- Column BN: Indication of major change in traffic flow compared to the upstream reference station, considered here to be a 15% difference.
- Column BO: Indication of a medium change in traffic flow compared to the upstream reference station, taken here to be a 10-15% difference.
- Column BP: Indication whether the station exceeds the set 15-mile distance threshold.
- Column BQ: Indication whether the cumulative flow differential since the last flagged station exceeds 15%.
- Column BR: Indication whether the summation of absolute flow differentials between successive stations since the last flagged station exceeds 15%.

Summary Results

Column BT: Preliminary identification of stations flagged based on the Network Topology, Major Flow Change, and Medium Flow Change criteria. Stations marked in red are those with a major change in flow, while those marked in yellow as associated with a medium change, and those marked in green with a minor change. The number shown in the cell represents the reference station against which flow comparison is made. Any station associated with a major or medium flow change results in a reset of

the reference offset, i.e., in being used as a reference for the next comparison. This can be observed by the reference number dropping back to -1 on the next row.

- Columns BU-BW: Results of calculations of cumulative distance, cumulative flow change, and summation of absolute flow changes.
- Column BX: Final determination of flagged stations based on all evaluation criteria.

Station Statistics

- Columns CA to CX: Flow differential with previous reference station. This is the same data as in Figure 7-9 but reconfigured to show positive or negative differences.
- Columns CZ to DD: Statistics regarding the data contained in the analysis interval are considered to help understand variability within the data. This includes:
 - Average flow change with the previous station with valid data.
 - Absolute value of flow change with the previous station with valid data.
 - Observed minimum and maximum flow changes.
 - Coefficient of variation of observed changes based on the analysis of absolute values.

7.2.5. CONSIDERATION OF ALTERNATE ANALYSIS PERIODS

The methodology outlined in the previous section indicates what to do to analyze station importance based on observed traffic flow data associated with a given period. In practice, there might be an interest in assessing station importance associated with various periods of the day, weekdays, or seasons to see if the consideration of alternate periods may affect the identification of stations representing a major or medium shift in flow.

For the analyses conducted as part of this project, corridor analyses were conducted over each of the following periods:

- Average daytime performance analyses (5 AM 8 PM)
 - o Weekdays
 - Spring
 - Summer
 - Fall
 - Weekend
 - Spring
 - Summer
 - Fall
- Time-of-day performance analyses
 - Weekday AM peak, 6 AM 8 AM
 - Fall
 - Weekday Midday, 11 AM 1 PM
 - Fall
 - Weekday PM Peak, 3 PM 5 PM
 - Fall

An example is shown in Figure 7-13, using the same data from I-880 North as in previous figures. The data shown in columns F to P, are simply a copy of the data from column BX in Figure 7-12 for each analysis period considered. The data from columns T and U further capture the average observed absolute change in flow at the station compared to the flow at the previous valid station. This is essentially an average of the data of Column DA in Figure 7-12 across all analysis periods.

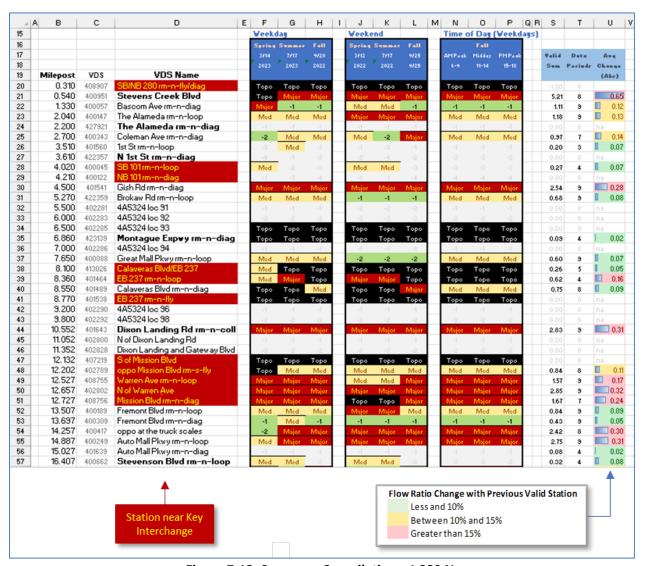


Figure 7-13: Summary Compilation - I-880 N

7.3. STAGE 3: AGGREGATE EVALUATION SCORES

The final part of the analysis consists of assigning an average evaluation score to each station along a corridor based on the results of the empirical evaluations described in the previous sections. This is done to help summarize the potentially different importance status assigned to each station across various analysis periods. As an example, several stations in the example of Figure 7-13 were on occasion flagged

as capturing major shifts in flow during some analysis periods and capturing a medium shift during other periods. It is also done to help account for the fact that analyses may not always be possible across all evaluation periods due to fluctuations in data availability and validity. The goal here is to combine all these individual evaluations into an average score that could help determine the general importance of each station based on available evaluation results.

To perform the evaluation, a customizable voting mechanism assigning weights to each station based on the following two dimensions was developed:

- Weights according to reason a particular station was flagged as important during the empirical evaluation:
 - Network topology
 - Major flow changes
 - Medium flow changes
 - Distance to the previous station with valid data
 - Cumulative flow change
 - Absolute Cumulative Flow Change
- Weights according to analysis period:
 - o Weekdays
 - Average daytime analyses
 - Time-of-day analyses (AM Peak, Midday, PM Peak)
 - Weekends
 - Average daytime analyses

Figure 7-14 illustrates an application of the voting mechanism to the data of Figure 7-13 considering sensor prioritization along I-880 North across various analysis periods. The voting mechanism used in this case uses the following weights:

- Weights varying between 1 and 10 for the assigned importance within each evaluation period.
 In this case, the highest weights are assigned to stations flagged as capturing a major or medium change in traffic flows. Lower weights are assigned to stations flagged based on distance or cumulative flow changes.
- Weights varying between 0.25 and 0.50 for the specific evaluation period considered. In this
 case, it is assumed that analysts would normally put more emphasis on peak weekday traffic
 periods and other periods.

The resulting evaluation scores are shown in Column Z, with a color gradient highlighting the stations with high (red), medium (yellow), and low (green) scores. Stations for which there were no evaluation periods with valid data, and thus for which it was not possible to conduct empirical evaluations, are shown with a "na" indication.

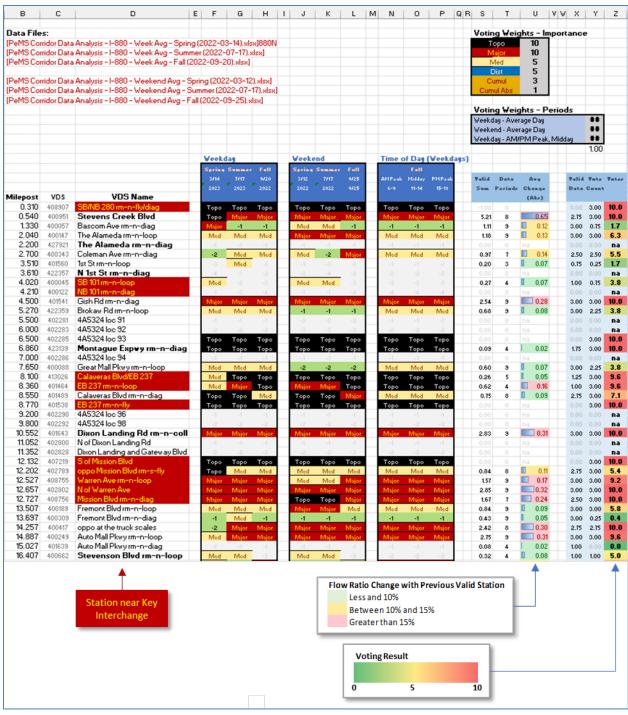


Figure 7-14: Evaluation Results – I-880 N

8. PRACTICAL GUIDELINES FOR SENSOR PLACEMENT

In the context of DTI and emerging needs for greater connectivity and more diverse sources of data, new strategies are needed to benefit from the data made possible by fixed infrastructure sensors. This research proposes to leverage the strengths of different data types to "get the most bang for the buck."

Fixed sensors are not generally needed where the intent is primarily to monitor traffic congestion. Private sources of data do have limitations regarding latency but do provide broad coverage and useful samples of traffic speeds and travel times. One notable exception is when the intent is to monitor speed on a specific lane, for example in the case of HOV performance reporting. Private third-party data may be used to cover specific situations such as:

- Monitoring locations of congestion hotspots, and extent of queues, through monitoring of speeds
- Obtaining routing patterns from the location
- Checking relative intensity of traffic across locations to validate other observed data

8.1. BASIC CONSIDERATIONS

8.1.1. INTERCHANGES AS AN ORGANIZATIONAL UNIT FOR FREEWAY DATA

One crucial outcome of this research is the proposal to reorganize the primary organizational unit for freeway data around major interchanges, not around freeways. The reason is that interchanges are places of major decision points, rerouting options, and places where flows change, and therefore where sensing data are most useful. A secondary organizational unit could be so-called FATVs (fully accounted traffic volumes)--a concept explored in a separate study (Khan, Fournier, Mauch, Patire, & Skabardonis, 2020). A tertiary organizational unit for convenience could be freeways. In this structure, groups of interchanges would constitute the main groupings of freeway data and multiple freeways could be associated with the same interchange. This would be an improvement on the current existing meta-data system.

8.1.2. KEY TACTICAL CONSIDERATIONS

Fixed-sensor data can provide complete traffic cross-sections of counts. This is only possible if all lanes, including auxiliary lanes, are instrumented. This is a crucial advantage of infrastructure-based sensing. In order to benefit, auxiliary lanes must be instrumented together with mainline lanes.

HOV facilities need their own instrumentation, but it is better not to instrument at the center of an HOV access gate. A preferred method would be to instrument in the actual HOV lane before or after the gate.

At any interchange of significance, all connectors, on-ramps, off-ramps, etc. should be instrumented.

It is especially valuable if traffic signal data at the entrance/exits of ramps are also available and integrated into a system for freeway traffic situational awareness and monitoring.

8.1.3. LOCAL FACTORS AS KEY DETERMINANTS OF SENSOR PRIORITY

Sensors required to support freeway operations (ramp metering, HOV monitoring, WIM support) and for which data cannot be reliably obtained through other means (e.g., t party) constitute a minimum set needed.

8.1.4. MAXIMUM DISTANCE BETWEEN SENSORS

The allowed distance between sensors depends on location and is only necessary to maintain a minimum density of sensors. In urban areas, it is likely that the presence of major interchanges, and data variations, will obviate the need for a maximum distance metric in this methodology. However, for rural areas with little data, it might be useful to set a target. Rough target distances might be on the order of 5 miles for an urban area, but more like 15-20 miles in a rural setting dominated by through traffic.

8.2. HOW TO USE THE PROPOSED METHODOLOGY

The suggested practice is to locate the most important freeway-to-freeway interchanges and to completely instrument them. This allows for better monitoring of area/regional trip patterns. Busy local interchanges are important in the sense that on-ramp/off-ramp data collected there facilitate a better understanding of local traffic patterns and can help explain why congestion or other problems exist at a given location. Complex interchanges often will be associated with busy interchanges.

8.2.1. SUMMARY OF METHODOLOGY

The proposed methodology can be summarized by the application of the following three analytical stages:

- Stage 1
 - o For a freeway, generate a list of interchanges and categorize them
 - Collect local factors, such as the need for ramp metering
 - Incorporate these needs as topological features of importance in stage 2
- Stage 2
 - Gather data as described and follow the process to prioritize sensors based on quantitative analysis of each period considered
- Stage 3
 - Combine results from all evaluated periods to develop an aggregate prioritization

At the end of this, what is achieved should be interpreted not as a list of mainline positions, but as a list of interchanges of importance. Each interchange of high priority should be completely instrumented. Sensor location meta-data should organized so that it is possible to associate sensors with interchanges, FATVs, and freeways. This will enable the possibility to implement automatic error checking, and also make it easier to integrate the data with other sources.

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APPENDIX A. CORRIDOR ANALYSIS RESULTS

This appendix presents the results of the prioritization analyses that have been conducted for the following corridors:

- I-5 Central Valley
- I-5 District 10
- I-5 District 12
- I-205
- I-280
- I-680
- I-880
- SR-55
- SR-57
- SR-91
- SR-99
- SR-120

			Weekday			Weeken	Weekend		Time of D	days)		
			Spring	Summer	Fall	Spring	Summer	Fall		Spring		
			3/13/2023		9/19/2022	3/18/2022		9/17/2022	AM Peak	Midday	PM Peak	١
Milepost	VDS								6-9		15-18	
221.29		JNO RTE 99 JCT - 15 NB	Торо	Торо	Торо	Торо	Торо	Торо	Торо	Торо	Торо	
225.09		JNO RTE 166 - 15 NB	Med	-1	-1	-1	-1	-1	Med	Med	Med	
227.98	601282	3.21 MI N-O RTE 166 - I5 NB	-1	-2	Med	-2	-2	-2	-1	-1	-1	
233.36		HERRING RD NB	-2	-3	Med	-3	-3	-3	-2	-2	-2	
234.98		3.62 MI S-O RTE 223 - 15 NB	-3 -4	-4	-1	-4	-4	-4 Dist	-3 -4	-3 -4	-3	
237.19 238.64		1.34 MI S OF RTE 223 NB (BEAR BL JCT RTE 223 BEAR MTN BLVD 5 NB	-4	Dist	-2	Dist	Dist	Dist	-4	-4 -5	-4	
242.43		S OF RTE 119 NB	Dist	-2	Med	-2	Med	Med	Dist	Dist	Dist	
244.29		RTE 119 SEP 5 NB	Med	Med	Med	-3	Med	-1	Med	Med	Med	
246.42	601179	I-5 AT SR 43 - NB	-1	-1	-1	-4		-2	-1	-1	-1	
249.04		N OF RTE 43 - 15 NB	Med	-2	Med	-5	-2	-3	Med	Med	Med	
253.76		N OF STOCKDALE HWY - 15 NB	Major	Major	Major	Med	Med	Major	Major	Major	Major	
257.69 258.97		JNO RTE 58 - 15 NB	Med	Med	-1	Med	Med	-1	Med	Med	Med	
258.97		N OF SR 58 (REST AREA) - 15 NB 7TH STANDARD RD 5 NB	-1 -2	-1 -2	Med	-1 -2	-1 -2	-2 Med	-1 -2	-1 -2	-1 -2	
263.92		0.5 MI NO BUTON WILLOW DR NB	Med	Med	Med	-3	Med	-1	Med	Med	Med	
266.30		S-O LERDO HWY - I5 NB	-1	-1	-1	-4	-1	-2	-1	-1	-1	
268.04		N-O LERDO HWY - I5 NB	Med	Med	Med	-5	-2	Med	Med	Med	Med	
270.34		AT MERCED - I5 NB	-1	-1	-1	-6	-3	-1	-1	-1	-1	
273.12		5.4 MI NO LERDO HWY (AT CANAL) N	-2	-2	-2	Dist	-4	-2	-2	-2	-2	
275.67 282.26		S-O RTE 46 - I5 NB LOST HILLS RD 5 NB	-3 Med	-3 Maior	-3 Med	-1 Med	-5 Med	-3 Med	-3 Med	-3 Med	-3 Med	
287.52		TWISSELMAN RD - NB	-1	-1	-1	-1	-1	-1	-1	-1	-1	
289.62		2 MILE NO TWISSELMAN RD - NB	-2		-2	-2		-2	-2		-2	
293.32		11 MI SO UTICA NB	-3	-3	-3	-3	-3	-3	-3	-3	-3	
296.14	601734	IS 8 MILES SO UTICA - NB	Med	-4	Med	Major	-4	Med	Med	Med	Med	
299.26		WATERLOO AVE OC - IB NB	Med	Med	Major	Major	Med	Major	Med	Med	Med	
301.59		SOUTH OF UTICA NB	Med	Med	Med	Major	Med	Med	Med	Med	Med	
304.50		I-5 JSO UTICA AVE NB	-1	-1	Med	-1	-1	Med	-1	-1	-1	
305.66 315.19		N OF UTICA AT QUINCY AVE NB	-2	-2 -2	Med	Med	-2	Med	-2	-2	-2	
317.24		I-5 1.9 MI S-O RTE 269 NB	Dist	Dist	-2	Med	Dist	Med	Dist	Dist	Dist	
319.11		I-5 AT RTE 269 NB	-1	Med	-3	-1	Med	-1	-1	-1	-1	
327.32		N OF JAYNE AVE NB	-2	-1	Dist	-2	Med	-2	-2	-2	-2	
330.26	601591	I-5 3.5 MI S-O 198 NB	Med	-2	-1	-3	-1	-3	Med	Med	Med	
333.62		I-5 JSO RTE 198 NB	Med	Med	Med	Med	-2	Med	Med	Med	Med	
342.49		IS NB SOUTH OF COALINGA-MENDOTA	-1	Major	Major	-1	Dist	Major	-1	-1	-1	
343.39 344.50		0.11 MI N-O JEFREY OC NB IS NB NORTH OF COALINGA-MENDOTA	Major -1	-1 -2	-1 -2	Major -1	-1 -2	-1 -2	Major -1	Major -1	Major -1	
345.18		3.17 MI S-O JCT 33 NB	-2	-2 -3	-3	-2	-2 -3	-3	-2	-1 -2	-1 -2	
349.53		JNO RTE 33 (NEXT TO CMS 26) NB	-3	-4	-4	-3	-4	Med	-3	-3	-3	
368.23		N-O PANOCHE RD NB	-4	-5	-5	-4	-5	-1	-4	-4	-4	
371.01		I-5 S-O RUSSELL AVE NB	Dist	Dist	Dist	Med	Med	Dist	Dist	Dist	Dist	
373.30		AT JENSEN AVE NB	-1	-1	-1	-1	Med	-1	-1	-1	-1	
375.43		N-O CALIFORNIA NB	Med	-2	-2	-2	-1	Med -1	Med	Med	Med	
378.99 384.91		SHIELDS AVE OC 5 NB NEES AVE OC 5 NB	Med -1	Med	-3 -4	Med	Med	-1	Med -1	Med -1	Med -1	
385.55		S-O Ericca Rest Area	-2	-1	-5	-2	-1	-3	-2		-2	
390.09		S-O Off Ramp to Vista Point Rd	-3		-6	-3		-4	-3		-3	
390.42		N-O On Ramp from Vista Point Rd	-4		-7	-4	-3	-5	-4		-4	
391.20	1031810	S-O SR 165-Mercy Springs Rd	-5		-8	Med	Med	-6	-5		-5	
391.54		N-O SR 165	-6		-9	-1		-7	-6		-6	
402.50		S-O SR 152	-7		-10	-2		-8	-7		-7	
406.99		N-O SR 33 N-O Truck Scales	-8		-11	-3		-9 -10	-8		-8	
408.57 417.13		N-O Truck Scales S-O SR 140	-9 -10	Med	-12 -13	-4 -5	Med	Med	-9 -10		-9 -10	
428.09		S-O Fink Rd	-11	-1	-14	-6	-1	Med	-11		-10	
433.70		Sperry Ave	Major	Major	Major	Major	Major	-1	Major	Major	Major	
440.64		N-O Howard Rd	-1	Med	-1	Med	Med	Med	-1	-1	-1	
445.04		Westley Rest Area	-2	-1	Major	-1	Major	-1	-2	-2	-2	
445.40		Westley Rest Area	-3	-2	-1	-2	-1	-2	-3	-3	-3	
445.84		N-O I-580	Major	Major	Med	Major	Major	Major	Major	Major	Major	
449.59		N-O SR 132	Med	-1	Major	Med	-1	-1	Med	Med	Med	
452.13			-1	Med	-1	-1	Med	-2	-1		-1	
458.14 458.37		S-O Jct SR EB I-205 S-O Jct SR EB I-205	-2	Major	Major	-2	Major	Major	-2	-	-2 Major	
458.62		S-O Jct SR EB 1-205 S-O Jct SR EB 1-205	Major -1	Major Med	Major -1	Major Med	Major Major	Major Med	Major -1	Major -1	Major -1	
458.99		S-O Manthey Rd	Major	Major	Major	Major	Major	Major	Major	Major	Major	
459.52		N-O Mossdale Rd	Major	-1	-1	Major	-1	-1	Major	Major	Major	
460.06		N-O Jct EB SR 120	Major	Major	Major	Major	Major	Major	Major	Major	Major	
460.50		N-O Jct SR 120	Med	Med	Med	Med	Med	Major	Med	Med	Med	

Figure A-1: Evaluation Results – I-5 N Central Valley

			Weekday			Weekend			Time of D	ay (Week	days)	
			Spring	Summer	Fall	Spring	Summer	Fall		Spring		
			3/13/2023		9/19/2022	3/18/2022		9/17/2022	AM Peak		PMPeak	
									6-9		15-18	
ilepost												
460.92		N-O Jct SR 120	Торо	Торо	Торо	Торо	Торо	Торо	Торо	Торо	Торо	
460.50		N-O Jct SR 120	-1	-1	-1	-1	-1	-1	-1	-1	-1	
460.01 459.52		N-O Jct WB SR 120 N-O Mossdale Rd	Major Major	Major Major	Major	Major Major	Major Major	Major	Major Major	Major Major	Major Major	
459.32		S-O Mossdale Rd	Major	Major	Maior	Major	Maior	Maior	Major	Major	Major	
458.99		S-O Manthey Rd	-1	-1	-1	-1	Med	-1	-1	-1	-1	
458.72		S-O Manthey Rd	-2	-2	-2	-2	-1	-2	-2	-2	-2	
458.69			Major	Major	Major	Major	Major	Major	Major	Major	Major	
458.37		S-O Jct SR EB I-205	Major	Maior	Major	Major	Maior	Major	Major	Maior	Major	
458.22		S-O Jet SR WB I-205	-1	-1	-1	-1	-1	Med	-1	-1	-1	
458.22		S-O Paradise Cut	Major	Major	Major	Major	Major	Major	Major	Major	Major	
452.13		Jct SR 33	-1	Major	-1	-1	Major	-1	-1		-1	
449.51	10119510	N-O SR 132	Major	-1	-2	Major	-1	-2	Major	Major	Major	
445.80	1091610	N-O I-580	Med	Med	Major	-1	-2	Major	Med	Med	Med	
445.34		Westley Rest Area	-1		-1	-2		-1	-1		-1	
444.98		Westley Rest Area	-2	-2	-2	-3	-4	-2	-2	-2	-2	
440.57	10112510	N-O Howard Rd	Major	Major	Major	Major	Major	Major	Major	Major	Major	
433.64		Sperry Ave	Med	Major	Med	Med	Med	Med	Med	Med	Med	
428.17	10120010		Major	Med	Major	Med	Med	Med	Major	Major	Major	
417.16		S-O SR 140	-1		-1	-1	Med	-1	-1		-1	
408.32		N-O Truck Scales	-2		-2	-2		-2	-2		-2	
407.00	1033810		-3		-3	-3		-3	-3		-3	
402.25		S-O SR 152	-4		-4	-4		-4	-4		-4	
391.48			-5	-5	-5	-5	-4	-5	-5	-5	-5	-
391.10		S-O SR 165-Mercy Springs Rd	Med	Med	-6	Med	Med	-6	Med	Med	Med	
385.49		S-O Ericca Rest Area	-1	-1	-7	-1	-1	-7	-1	-1	-1	
384.85		NEES AVE OC 5 SB	-2	-2	Med	-2	Med	Med	-2	-2	-2	
378.92		SHIELDS AVE OC 5 SB	-3 Dist	-3	-1	-3	-1	-1 84-4	-3 D ist	-5 D ist	-3 D ist	
375.37 373.24		N-O CALIFORNIA SB	Dist Med	-4 Med	Med	-4 Med	-2 Med	Med	Dist Med	Dist	Dist Med	
373.24		AT JENSEN AVE SB	-1	-1	-1 -2	-1	Med	-1 -2	-1	Med -1	-1	
368.16		N-O PANOCHE RD SB	-2	-1 -2	-2 -3	-1	-1	-2 -3	-1	-1 -2	-1 -2	
349.46		JNO RTE 33 (NEXT TO CMS 26) SB	Med	Med	Med	Med	Med	-3 -4	Med	Med	Med	
345.11		3.17 MIS-OJCT33 SB	-1	Med	Med	-1	-1	Med	-1	-1	-1	
344.43		IS NB NORTH OF COALINGA-MENDOTA	-2	-1	-1	-2	-2	-1	-2	-2	-2	
343.31		0.11 MI N-O JEFREY OC SB	-3	-2	-2	-3	-3	-2	-3	-3	-3	
342.42		IS NB SOUTH OF COALINGA-MENDOTA	-4	-3	-3	-4	-4	-3	-4	-4	-4	
333.55		I-5 JSO RTE 198 SB	Major	Maior	Major	Major	Major	Major	Major	Maior	Maior	
330.19		I-5 3.5 MI S-O 198 SB	Major	Major	Med	Major	Major	Major	Major	Maior	Major	
327.25		N OF JAYNE AVE SB	-1	-1	-1	-1	-1	-1	-1	-1	-1	
319.04		I-5 AT RTE 269 SB	-2	Med	Med	-2	Med	-2	-2	-2	-2	
317.18		I-5 1.9 MI S-O RTE 269 SB	Med	Med	Med	Med	Med	Med	Med	Med	Med	
315.13		AT QUINCY AVE SB	-1		-1	-1		-1	-1		-1	
305.59	602506	N OF UTICA	-2	-2	-2	-2	-2	-2	-2	-2	-2	
304.44	602505	I-5 JSO UTICA AVE SB	-3		-3	-3		-3	-3		-3	
301.52	602390	SOUTH OF UTICA SB	Med	Med	Med	Med	Dist	Med	Med	Med	Med	
299.20	602326	WATERLOO AVE OC - 15 SB	-1	-1	-1	-1	-1	-1	-1	-1	-1	
296.08	602734	IS 8 MILES SO UTICA - SB	Med	-2	Med	Major	-2	Med	Med	Med	Med	
293.25	602391	11 MI SO UTICA SB	-1	-3	Med	-1	-3	Med	-1	-1	-1	
289.55		2 MILE NO TWISSELMAN RD - SB	-2		-1	-2	-4	-1	-2		-2	
287.46		TWISSELMAN RD - SB	-3	-5	-2	-3	Med	-2	-3	-3	-3	
282.12	602318	LOST HILLS RD SB	Med	Dist	-3	Major	Med	-3	Med	Med	Med	
275.60		S-O RTE 46 I5 SB	Med	Major	Med	Major	Major	Med	Med	Med	Med	
273.06		5.4 MI NO LERDO HWY (AT CANAL) S	-1	-1	-1	-1	-1	-1	-1	-1	-1	
270.27		AT MERCED - I5 SB	-2	-2	-2	-2	-2	-2	-2	-2	-2	
267.97		N-O LERDO HWY - I5 SB	-3	-3	-3	-3	-3	-3	-3	-3	-3	
266.23		S-O LERDO HWY - I5 SB	-4	-4	Med	-4	-4	Med	-4		Cumul Abs	
263.87		0.5 MI NO BUTON WILLOW DR SB	-5	-5	-1	-5	-5	-1	-5	-5	-1	
261.64		7TH STANDARD RD 5 SB	Med	Cumul Ab		Med	Med	-2	Med	Med	Med	
258.89		N OF SR 58 (REST AREA) - I5 SB	-1	-1	Med	-1	-1	-3	-1	-1	-1	
257.62		JNO RTE 58 - IS SB	-2	Dist	-1	-2	-2	-4	-2		-2	
253.67		N OF STOCKDALE HWY - 15 SB	-3	Med	-2	Med	Med	-5	-3	-3	-3	
248.97		N OF RTE 43 - I5 SB	Major	Med	Major	Med	Med	Med	Major	Major	Major	
246.38		I-5 AT SR 43 - SB	-1	Med	-1	-1	Major	-1	-1	-1	-1	
244.21		RTE 119 SEP 5 SB	Med	Med	Med	-2	Major	-2	Med	Med	Med	
242.36		S OF RTE 119 SB	-1	-1	Med	Med	Med	Med	-1	-1	-1	
238.57		CT RTE 223 BEAR MTN BLVD 5 SB	-2	-2	-1	-1	-1	-1	-2	-2	-2	
237.12		1.34 MI S OF RTE 223 SB (BEAR BL	-3	-3	Med	-2	Med	Med	-3	-3	-3	
234.93		3.62 MI S-O RTE 223 - I5 SB	-4	-4	-1	-3	-1	-1	-4	-4	-4	
233.28		HERRING RD SB	Med	-5	Med	-4	-2	-2	Med	Med	Med	
227.89 225.01		3.21 MI N-O RTE 166 - I5 SB	-1	Med	Med	-5 Dist	-3 -4	-3	-1	-1 -2	-1 -2	
	603383	JNO RTE 166 - I5 SB	-2	-1				-4	-2			

Figure A-2: Evaluation Results – I-5 S Central Valley

		Weekda	У		Weekend			Time of D		lays)	
		Spring	Summer		Spring	Summer			Fall		
		3/14/2023		9/19/2022	3/12/2022		9/24/2022	AM Peak	Midday	PM Peak	V
								6-9		15-18	
lilepost											
385.55		Торо	Торо	Торо	Торо	Торо	Торо	Торо	Topo	Торо	1
390.09	1031610 S-O Off Ramp to Vista Point Rd	-1		-1	-1		-1	-1		-1	
390.42	1031710 N-O On Ramp from Vista Point Rd	-2		-2	-2		-2	-2		-2	
391.10	1031810 S-O SR 165-Mercy Springs Rd	-3		-3	-3		-3	-3		-3	
391.54	The second secon	-4	-4	-4	-4	-4	-4	-4	-4	-4	
402.41	1032010 S-O SR 152	Торо	Торо	Торо	Торо	Торо	Торо	Торо	Торо	Торо	1
406.75	1032410 N-O SR 33	-1		-1	-1		-1	-1		-1	
407.17	1032510	-2 -3		-2	-2		-2 -3	-2		-2 -3	
408.45	1032610 N-O Truck Scales	-3		-3	-3		-3	-3		-5	
408.72	1032710	-4	-4	-4	-4	-4	-4	-4	-4	-4	
417.13		Major	Major	Major	Major	Major	Major	Major	Major	Major	1
417.23	1032810	-1	-1	-1	-1	-1	-1	-1	-1	-1	
428.09		Major	Med	Major	Med	-2	Med	Major	Major	Major	
433.70 440.64	1007810 Sperry Ave	-1	Med	D. A. o. d	-1	Iviajor	NA or d	-1	NAI	NAc el	
440.64	10112310 N-O Howard Rd	Med	Med	Med	Med	Med	Med	Med	Med	Med	1
445.84	1091810 Westley Rest Area 10119610 N-O I-580	Major	Topo	Topo	Med	Topo	Topo	Topo	Topo	Topo	
458.14	1015810 N-O I-580 1015810 N-O SR 132	iviajor	торо	1000	ivied	Τορο	Τορο	ТОРО	Τορο	Τορο	_
458.14	1015810 N-0 SR 132 1015910 S-0 Jct SR EB I-205	-2	Major	Major	Adolos	Major	Major	-1	Major	Majer	1
458.62	1015910 S-0 Jct SR EB I-205 1016010 S-0 Jct SR EB I-205	Naior	-1	Major -1	Major Med	Major Med	Major Med	Major -1	Major -1	Major -1	
458.83	1016110 S-O Jet SR EB I-205	Iviajoi	-1	-1	ivieu	ivieu	ivieu	-1	-1	-1	
458.99		Major	Major	Major	Major	Major	Major	Major	Major	Major	1
459.52	1048010 N-O Mossdale Rd	IVIAJOI	IVIAJUI	IVIAJOI	IVIAJOI	IVIAJUI	Iviajui	IVIAJOI	IVIAJUI	IVIAJUI	
459.52	1048010 N-O Mossdale Rd 1048610 N-O Jct EB SR 120		Major	Maior	Major	Maior	Major	Majar	Maior	Major	1
460.50	1048810 N-O JCE EB SK 120 1048210 N-O JCE SR 120	Major	Med	Med	Med	Major	Major	Med	Med	Med	
461.20	1048210 N-0 Jet SR 120 1048310 S-O Louise Ave	Major	Major	ivied	Major	Major	Iviajor	ivied	ivied	ivied	1
462.06	10121610 Louise Ave	Med	Med	Med	Med	Med	Med	Med	Med	Med	
462.83		Med	Med	Med	Med	-1	Med	Med	Med	Med	
464.15	10121210 N-O Lathrop Rd	Med	-1	-1	Med	-2	Med	-1	-1	-1	
464.65	10121210 N-O Lathrop Rd 10121410 S-O Roth Rd	-1	-1 -2	-1	-1	-2 -3	-1	-2	-1 -2	-1 -2	
465.15	10121110 N-O Roth Rd	-2	-2 -3	-3	-2	-3 -4	-2	-3	-2 -3	-2 -3	
465.94	10121110 N-O Roth Rd 10121310 N-O Eldorado St	-3	Med	Cumul Abs		Cumul Ab		_	Cumul Ab	-	
466.37	10121310 N-O Eldorado St 10121010 N-O Eldorado St	Med	-1	Med	-3 -4	-1	-3 -4	Med	Med	Med	
466.93	1012010 N-O Erdorado St 10120910 Mathews Rd	-1	-1 -2	-1	-5	-2	-5	-1	-1	-1	
467.93	1096710 S-O French Camp Rd	Med	-2 -3	-2	Med	Med	Cumul Abs	-2	-1 -2	-2	
468.14	1096310 N-O French Camp Rd	-1	Med	Med	-1	Maior	-1	Med	Med	Med	
468.70	1007410 French Camp Turnpike	Major	-1	Major	Med	-1	Med	Major	Major	Major	
469.27	1021310 N-O French Camp Slough	Iviajoi	-1 -2	-1	ivieu	-1 -2	ivieu	IVIAJOI	-1	IVIAJUI	
469.59	1021410 N-O Downing Ave UC	2		-2	2		2	2		2	
470.24	1021410 N-O Downing Ave UC 1021510 N-O Eighth St UC	-2		-3	-2		-3	-2		-2	
470.24	1021510 N-O Eighth St OC 1016610 S-O Charter Way	Med	Major	Med	Med	Major	Med	Med	Med	Med	
470.69	1027410 Off to Fresno Ave	Topo	Topo	Торо	Topo	Topo	Topo	Topo	Topo	Торо	
472.96	10115310 N-O Fremont St- Pershing Ave	Major	Major	Major	Major	Major	Major	Major	Major	Major	
473.42	1022010 N-O Monte Diablo Ave UC	iviajor	-1	-1	iviajor	iviajoi	-1	iviajor	-1	-1	
475.39	1022010 N-O Monte Diablo Ave UC 1022210	-1		-1 -2	-1		-1 -2	-1		-1 -2	
476.85	1022210 1022410 N-O Benjamin Holt Drive	-2		-3	-2		-3	-2		-2 -3	
477.20		-3	Med	-4	-3		-5 -A	-3		-5 -4	
477.20	1022610 Hammer Lane UC	-4	ivied	-4 -5	-4		-5	-4		-4 -5	
478.52	1073410 N-O Hammer Lane	-5		-6	-5 -6		-5 -6	-6		-5 -6	
479.52	1073410 N-O Hammer Lane 1077110 S-O Eight Mile Rd	-7		-7	-7		-7	-7		-0 -7	
480.01	1077110 S-O Eight Mile Rd 1077210 N-O McAuliffe Rd	-/		-8	-8		-/	-7		-/ -8	
480.48	1077310 S-O Eight Mile Rd	-8		-9	-0		-9	-0		_0	
483.57	1022910 N-O Van Ruiten UC	Major	Maior	Major	Maior	Maior	Major	Maior	Major	Maior	
486.59	1022910 N-O Van Ruiten UC 1077910 S-O Turner Rd	iviajor	iviajor	iviajoi	Iviajor	IVIAJUI	IVIAJOI	Iviajor	IVIAJUI	ividjul	
		-1 140i-	Major	Major	- L	Mod	Major	Major	Maior	Major	
486.81 487.20	1073210 S-O Turner Rd 1078010 N-O Turner Rd	iviajor	iviajor	iviajor	Med	Med	iviajor	iviajor	iviajor	ividjor	
487.72		-1		-1	-1		-1	-1		-1	
	1078110 S-O Woodbridge Rd	-2		-2	-2		-2	-2		-2	
488.19	1078210 N-O Woodbridge Rd	-3	-3	-5	-3	-3	Med	-3	-3	-3	
491.38	10125010 Beaver Slough	Med	Med	Med	Med	Med		Med	Med	Med	

Figure A-3: Evaluation Results – I-5 N District 10

		Weekda	У		Weekend			Time of Day (Weekdays)				
		Spring	Summer	Fall	Spring	Summer	Fall		Fall			
		3/14/2023		9/19/2022	3/12/2022		9/24/2022	AM Peak	Midday	PM Peak		
	VDQ.							6-9		15-18	_	
lepost			_		_	_	_	_	_			
496.79	10146710 Sac-SJ County Line	Торо	Торо	Торо	Торо	Торо	Торо	Торо	Торо	Торо		
491.38 488.19	10125110 Beaver Slough	-1	-1 -2	-1 -2	-1	-1 -2	-1	-1	-1 -2	-1 -2	-	
487.69	1078310 N-O Woodbridge Rd 1078410 S-O Woodbridge Rd	-3		-3	-2 -3		-2 -3	-2		-3		
487.17	1078510 Turner Rd	-A		-4	-4		-4			-4		
486.81	1073310 S-O Turner Rd	-5	-5	Med	-5	-5	-5	Med	Med	Med		
486.58	1078610 S-O Turner Rd	-6	-6	-1	-6	-6	-6	-1	-1	-1		
483.57	1024810 N-O Van Ruiten UC	-7	-7	Major	-7	-7	-7	Major	Maior	Major		
480.46	1078710	-8	-8	-1	-8	-8	-8	-1	-1	-1		
479.47	1077410 S-O Eight Mile Rd	-9		-2	-9		-9	-2		-2		
478.52	1073510 N-O Hammer Lane	Major	Major	-3	Major	Major	Major	-3		-3		
478.15	1024510 S-O Hammer Lane UC	-1	Major	-4	-1	Med	Major	-4		-4		
477.20	10115110 N-O Benjamin Holt Drive	Med	Med	-5	Med	Med	Med	-5		-5		
476.60	1024210 Swain Rd UC	Major	Major	-6	Major	Major	Major	-6		-6		
475.48	1024110 S-O March Lane UC	Major	-1	-7	Med	-1	-1	-7		-7		
473.42	1023910 Monte Diablo Ave	Major	Major	-8	Major	Major	Major	-8	-8	-8		
472.96	10115210 N-O Fremont St- Pershing Ave	Major	Major	Major	Major	Major	Major	Major	Major	Major		
471.97	1023710 Off Ramp to SR 4	Major	Major	Торо	Major	Major	Major	Торо	Торо	Торо		
468.81	1017010 S-O Downing Ave	Major	Major	Major	Major	Major	Major	Major	Major	Major		
468.70	1007510 French Camp Turnpike	Major	Major	-1	Major	Major	Major	-1	-1	-1		
468.06	1096010 N-O French Camp Rd OC	-1	-1	Med	-1	-1	-1	Med	Med	Med		
467.87	1096510 S-O French Camp Rd	-2	-2	Med	-2	-2	-2	Med	Med	Med		
466.90	1000110 N-O Mathews Rd	-3	Major	-1	-3	Major	-3	-1	-1	-1		
466.37	1000210 N-O Eldorado St	Major	Major	-2	Major	Major	Major	-2	-2	-2		
465.94	1000310 N-O Eldorado St	Major	Major	-3	Major	Major	Major	-3	-3	-3		
465.59	1000410 N-O Roth Rd	Med	-1	Med	-1	-1	-1	Med	Med	Med		
465.15	1000510 N-O Roth Rd	-1	-2	Med	-2	-2	-2	Med	Med	Med		
464.65	1000610 S-O Roth Rd	-2	-3	-1	-3	-3	-3	-1	-1	-1		
464.60	1000710 N-O Lathrop Rd	-3	Med	-2	-4	-4	Med	-2	-2	-2	_	
464.15	1000810 N-O Lathrop Rd	Med	-1	-3	-5	Med	Med	-3	-3 -4	-3		
463.66	1000910 N-O Lathrop Rd	Med	Med	-4	Cumul Ab	Med	Med	-4		-4		
462.78	1001010 S-O Lathrop Rd	-1	Med	Med	-1	Med	Med	Med	Med	Med		
462.50	1001210 S-O Lathrop Rd	-2	-1	-1	-2	-1	-1	-1	-1	-1		
462.39 461.96	1001110 S-O Lathrop Rd	-3	-2	Med	-3	-2	-2	Med	Med	Med		
	1001310 S-O Louise Ave	Med	Med	-1	-4	Med	Med -1	-1	-1	-1		
461.90 461.34	10121510 Louise Ave	-1 -2	-1 -2	Med Med	-5 -6	-1 -2	-1	Med	Med	Med Med	-	
461.20	1001410 S-O Louise Ave 1001610 S-O Louise Ave	-2	-Z	ivied	-6	-2	-2	ivied	ivied	ivied		
460.92	1001510 N-O Jct SR 120	Med	Med	-2	Cumul Ab	-4	-4	-2	-2	-2		
460.50	1001710 N-O Jet SR 120	Med	Med	-3	-1	-4 -5	Med	-3	-2 -3	-3		
460.01	1001710 N-0 Jct SK 120 1001810 N-0 Jct WB SR 120	-1	-1	Major	-2	-5 -6	-1	Major	Maior	Maior		
459.52	1001910 N-O Mossdale Rd	-2	-2	-1	Med	Med	-2	-1viajO1	-1	-1		
459.52	1003410 S-O Mossdale Rd	-3	-3	Major	Med	Med	-3	Major	Maior	Major		
458.99	10127310 S-O Manthey Rd	-4	Cumul Ab	major	-1		Cumul Abs	-1	-1	-1		
458.72	1003510 S-O Manthey Rd	Med	-1	-2	Med	-2	Med	-2	-2	-2		
458.69	10119710 Jct SR 205	-1	Med	Major	-1	Med	-1	Maior	Major	Major		
458.37	10120810 S-O Jct SR EB I-205	-2	-1	Major	-2	-1	-2	Major	Major	Major		
458.22	1003610 S-O Paradise Cut	Major	Major	-1	Major	Major	Major	-1	-1	-1		
458.22	10121710 S-O Jct SR WB I-205	-1	-1	Major	-1	-1	-1	Major	Major	Major		
449.51	10119510 N-O SR 132	Торо	Торо	Торо	Торо	Торо	Торо	Торо	Торо	Торо		
445.80	1091610 N-O I-580	-1	-1	Major	-1	-1	-1	Major	Major	Major		
440.57	10112510 N-O Howard Rd	-2	Major	Major	-2	-2	-2	Major	Major	Major		
433.64		-3	-1	Med	-3	-3	-3	Med	Med	Med		
428.17		-4	-2	Major	-4	-4	-4	Major	Major	Major		
417.16	1034110 S-O SR 140	-5		-1	-5		-5	-1	-1	-1		
417.07		-6		-2	-6		-6	-2		-2		
408.32	1034010 N-O Truck Scales	Major	Major	-3	Major	Major	Major	-3		-3		
408.28	1033910	-1	-1	-4	-1	-1	-1	-4		-4		
407.00	1033810	Major	Major	-5	Med	Med	Med	-5		-5		
406.67	1033710 N-O SR 33	-1	-1	-6	-1	-1	-1	-6	-6	-6		
402.25	1033310 S-O SR 152	Торо	Торо	Торо	Торо	Торо	Торо	Торо	Торо	Торо		
391.48	10133110 N-O SR 165	-1	-1	-1	-1	-1	-1	-1	-1	-1		
391.10	1033110 S-O SR 165-Mercy Springs Rd	Med	Med	-2	Med	Med	Med	-2		-2		
385.49		-1		-3	-1	Med	-1	-3		-3		

Figure A-4: Evaluation Results – I-5 S District 10

				Summer			Summer	Fall		Time of D	Fall			
			3/7/2023		9/19/2022						Midday 11-14			Vo
Vilepost V		S. LUIS REY	Торо	Торо	Tono	Торо	Торо	Торо		Tono	Tono	Tono		10
73.52	1204211	MAGDALENA	Med	Med	Topo Med	Med	Med	Med		Med	Topo Med	Topo Med		5.
74.09 74.73	1204230 1204244	EL CAMINO REAL	Med	-1 Med	Med	-1 Med	-1 Med	-1 -2		Med	Med	Med		4.
75.06	1204244		-1	-1	-1	-1	-1	Cumul		-1	-1	-1		0
75.49 75.82	1210908 1204268		Major	Major	Major	Major Med	Major Med	Major Med		Major	Major	Major		8
76.28		HERMOSA 1	Med	Med	Med	-1	-1	-1		Med	Med	Med		3
76.36		HERMOSA 2	-1	-1	-1	-2	-2	-2		-1	-1	-1		0
76.96 77.66		S OF VAQUERO N OF VAQUERO	Med -1	Med -1	Med -1	Med -1	-3 -4	-3 -4		Med -1	Med -1	Med -1		0
77.99	1204316	ESTRELLA1	-2	-2	-2	-2	Cumul Abs	Cumul Abs		-2	-2	-2		0
78.15 78.73	1204328	ESTRELLA2 SACRAMENTO	Med	Cumul Ab	s Med	Med	Med	-1 -2		Med	Med	Med		4
79.17		LAS RAMBLES	Med	Med	Med	Med	Med	Med		Med	Med	Med		5
79.72 80.25		CAPISTRANO AEROPUERTO	Med	Med	Med Med	Med	Med	Med Med		Med Med	Med Med	Med Med		5
80.90	1204409	S JUAN CREEK	Med	Med	Med	Med	Med	Med		Med	Med	Med		5
81.41 81.93	1210926 1220030		-1	-1 -2	-1	-1 -2	-1 -2	-1		-1	-1	-1		0
81.95		ORTEGA 2	-2 Med	-2 -3	-2 Med	Major	-2 -3	-2 Med		-2 Med	-2 Med	-2 Med		4
82.36	1210974	EL HORNO	Major	Major	Major	Major	Major	Major		Major	Major	Major		10
82.66 83.31		SERRA PARK JUNIPERO SERRA	Med -1	Med	Med	Med	Med	Med		Med	Med	Med		5 n
83.63	1204447	TRABUCO	Med	-2	Med	-2	-2	Med		Med	Med	Med		5
84.17 84.52	1204453 1211107	WILDWOOD	-1 Med	Med	-1 Med	-3 -4	Med Med	-1 Med		-1 Med	-1 Med	-1 Med		5
84.81		MVDS at Avery 1	-1	-1	-1	-5	-1	-1		-1	-1	-1		п
85.31		MVDS at AVERY	-2		-2	-6				-2				п
85.31 85.96		AVERY PARK MVDS S-O Crown Va	-3 -4		-3 -4	-7 -8				-3 -4				n
85.96	1222018	CROWN VA1	Major	Major	Major	Major	Major	Major		Major	Major	Major		10
86.13 86.14		CROWN VA2 MVDS Crown Valley 2	-1 Med	Maior	-1 Med	-1 -2	Major	Major		Med	Med	-1 Med		6
86.72	1213570	FAIRCOURT	Major	-1	Major	-3	-1	Major		Major	Major	Major		10
87.39 87.42	1204515 1221811	OSO PARK1	Med	-2 Med	Med	-4 Med	-2 Med	-1 Major		Med	Med	Med		8
87.42	1204532	OSO PARK2	-1	-1	-1	-1	-1	-1		-1viajor -1	-1	-1 -1		7
87.65	1221803	OSO 2	Major	Major	Major	Major	Major	Major		Major	Major	Major		10
88.06 88.73	1213700 1221835	CERVANTES LA PAZ 1	-1 -2		-1 -2	-1 -2				-1 -2				0
88.77	1204546	LA PAZ 1	-3		-3	-3				-3				r
88.95 89.03	1204559 1221843		-4	-4 Major	-4 Major	-4 -5	-4 Major	-4 Major		-4 Major	-4 Major	-4 Major		10
89.69	1204571		-1	-1	-1	- -5	-1	-1		-1	-1	-1		n
89.89	1204586	ALICIA 2	-2		-2	-7				-2				п
90.43		ANKERTON FL TORO 1	-3 -4		-3 -4	-8				-3 -4				п
91.08		EL TORO 2	-5		-5	-10				-5				n
91.59		RED ROBIN	-6	-6	-6	-11	-6	-6 Mains		-6 Malaa	-6 Mains	-6		10
92.26 92.50	1204672	LAKE FOR2 OLDFIELD	-1	-7 Major	-1	-12 Major	-7 Major	-1		-1	-1	-1		2
92.85	1204697		Торо	Торо	Торо	Торо	Торо	Торо		Торо	Торо	Торо		1
93.20 93.51	1204699 1204703		Major Topo	Topo Topo	Major Topo	Major Topo	Topo Topo	Торо Торо		Major Topo	Major Topo	Major Topo		10
94.36	1204731	ALTON 2	Major	-1	Major	-1	-1	Major		Major	Major	Major		10
94.46 95.01	1204750	ALTON 3 BARRANCA	-1 Malas	Major Med	-1 Mai-e	Major Med	Major Med	Торо		-1	-1	-1		8
95.01	1204787	S OF 133	Med	Topo	Med	Med	Topo	Торо		Med	Med	Med		6
95.46	1204808		Торо	Topo	Торо	Med	Торо	Торо		Торо	Торо	Торо		9
95.76 95.95		S OF SAND CNYN SAND CANYON 1	Major	-1 -2	Major	Major Med	Med	Med		Major	Major	Major		3
96.31	1204861	SAND CANYON 2	Med	-3	Med	-1	-1	-1		Med	Med	Med		3
96.76 97.34	1204878 1204924	N OF SAND CNYN	Med	Med	Med	Med -1	Med -1	Med		Med	Med	Med		5
97.41	1204924		-1	-1	-1	-2	-2	-1		-1	-1	-1		0
98.06	1204950		-2	-2	-2	-3	-3	-2		-2	-2	-2		0
98.82 99.07	1204982 1205012		-4	-4	-4	Med	-5	Med		-4	-4	-4		1
99.80		JAMBOREE 1	-5		-5	-1	-6			-5				r
99.81 100.35		JAMBOREE 2 TUSTIN RANCH	-6 Med	-6 Med	-6 Med	-2 -3	-7 Med	-2 Mod		-6 Med	-6 Med	-6 Med		5
101.49	1205135		-1	-1	-1	-4	-1	Med		-1	-1	-1		0
102.04 102.25	1205152 1205157	NEWPORT*	Торо Торо	Topo Topo	Торо Торо	Med	Med Topo	Med Med		Торо Торо	Topo Topo	Торо Торо		9
102.25	1205157	S OF 55	Med	Med	Med	Major	Med	Med		Med	Med	Med		5
102.65	1205168 1205175	N OF 55	Торо	Major	Торо	Торо	Major	Торо		Торо	Торо	Торо		10
103.05 103.48	1205175 1205193		-1 -2	Major	-1 -2	Med	Major			-1 -2				1
103.65	1205204	CONCORD	-3	Med	-3	-1	Med			-3				5
103.85 103.98	1205215 1205225		-4		-4	Major				-4			H	1
104.75	1205262	17TH 2	Major	-3	Major	Major	-3	Med		Major	Major	Major		7
104.85 105.45	1205269 1205290		-1		-1	-1				-1				r
106.05	1205303	S OF 22	Med	Med	Med	Med	Med	Med		Med	Med	Med		5
106.45	1205320	LA VETA	Major	Major	Major	Major	Major	Major		Major	Major	Major		1
106.55 107.25	1205330 1205375	N OF 57 CHAPMAN 2	Med Major	Med Major	Med Major	Med Major	Med Major	Med Major		Med Major	Med Major	Med Major		1
107.35	1205341	STATE COLLEGE	Med	Med	Med	Med	Med	Med		Med	Med	Med		5
107.85 108.65		GENE AUTRY ORANGEWOOD 2	Med	Topo	Med	Topo	Med	Med		Med	Med	Med		5
108.73	1205409	KATELLA	Topo	Topo	Topo	Topo	Topo	Topo		Topo	Торо	Topo		10
108.85		ANAHEIM 2	Med	Med	Med	Med	Med	Med		Med	Med	Med		5
109.25 109.63	1212216 1205432	S OF HARBOR HARBOR	Med	Med	Med Med	Med	-1 Med	-1 Med		Med	Med	Med		5
109.95	1205452	BALL	-1	-1	-1	-1	Med	-1		-1	-1	-1		0
110.35	1205473		-2 Cumul Abs	Med -1	-2 Cumul Abs	Med -1	-1 -2	-2 Cumul Abs		-2 Cumul Abs	-2 Cumul Abi	Cumul -1	Н	1
111.15 111.55	1205493 1212649		Cumul Abs -1	-1 -2	Cumul Abs	-1 -2	-2 -3	Cumul Abs -1		Cumul Abs	Cumul Ab: -1	-1 -2	Н	0
111.85	1205517	EUCLID 2	Med	Med	Med	Major	Med	Med		Med	Med	Med		5
112.29 112.75	1205528 1212588	CRESCENT BROOKHURST 1	Med	Med	Med Major	Med	Med	Med	-	Med	Med	Med		10
112.95	1205553	BROOKHURST 2	-1	-1	-1	-1	-1	-1		-1	-1	-1		0
113.23	1205562	LA PALMA	-2	Med	-2	-2	-2	-2		-2	-2	-2		0
113.59	1205567 1205590	GILBERT MAGNOLIA	Med	Med	Med Med	Major	Med	Major Major		Med	Med	Med		6
114 05		N OF MAGNOLIA	Major	Major	Major	Major	Major	Major		Major	Major	Major		10
114.05 114.15				Major	Major	Major				Major	Major			10
114.15 114.77	1205607	ORANGETHORPE 2	Major	IVIAJUI	iviajor	TVIJOT					iviajui	Major		
114.15 114.77 114.95	1205607 1216505	N OF ORANGETHORPE	Med Med	Topo Med	Med	Med Major	Med	Med Med		Med Med	Med Med	Med Med		5
114.15 114.77	1205607	N OF ORANGETHORPE STANTON BEACH 1	Med Med Med	Topo Med Med		Med Major Major					Med Med Med			5 5

Figure A-5: Evaluation Results – I-5 N District 12

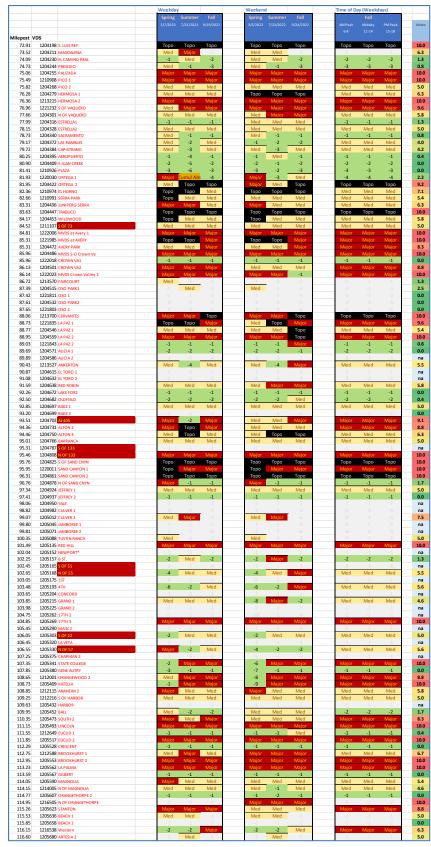


Figure A-6: Evaluation Results – I-5 S District 12



Figure A-7: Evaluation Results – I-205 E



Figure A-8: Evaluation Results - I-205 W

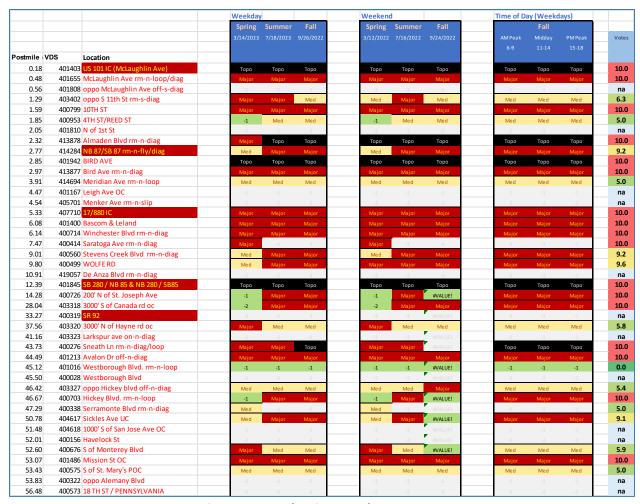


Figure A-9: Evaluation Results - I-280 N

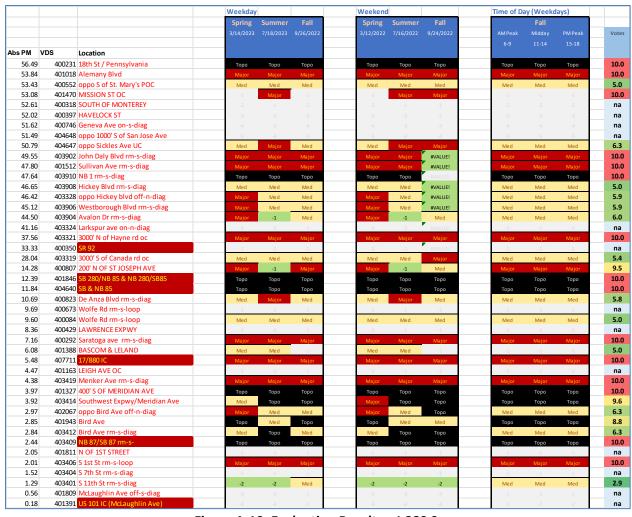


Figure A-10: Evaluation Results - I-280 S

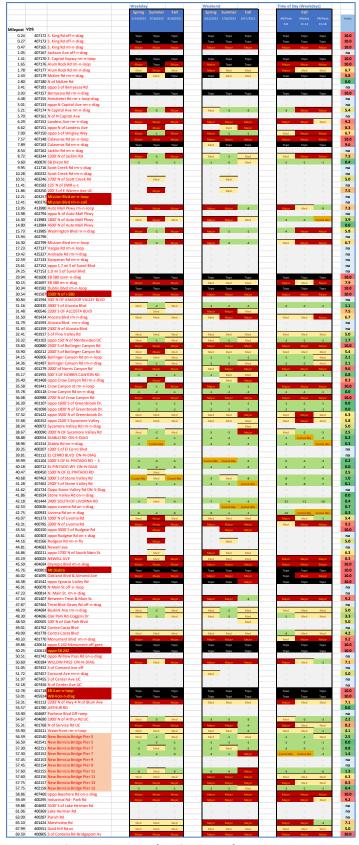


Figure A-11: Evaluation Results - I-680 N

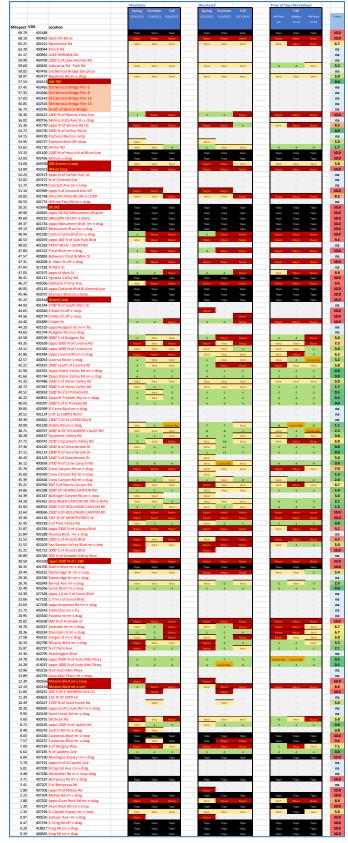


Figure A-12: Evaluation Results - I-680 S

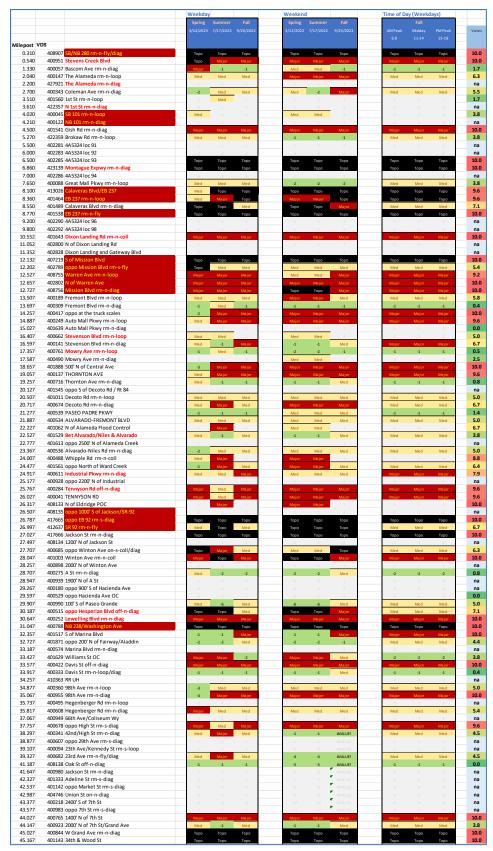


Figure A-13: Evaluation Results - I-880 N

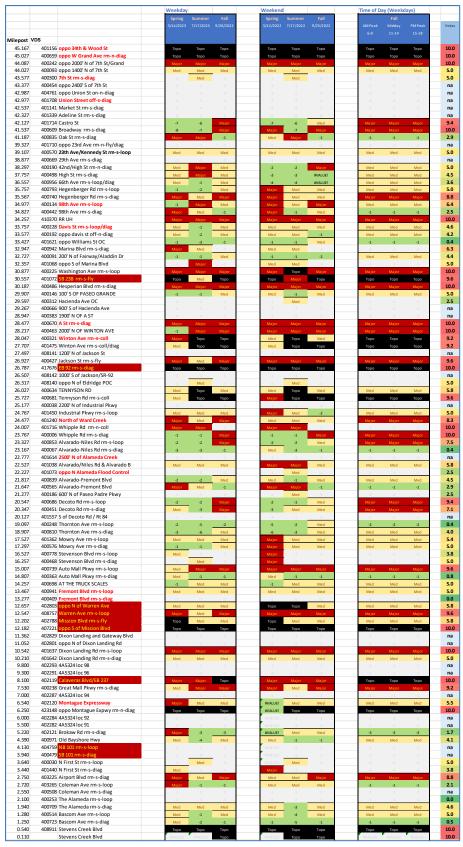


Figure A-14: Evaluation Results - I-880 S



Figure A-15: Evaluation Results - SR-55 N



Figure A-16: Evaluation Results – SR-55 S

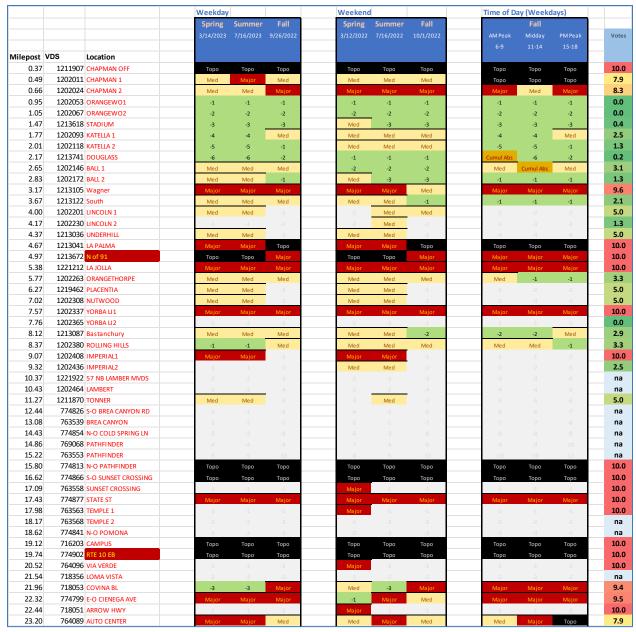


Figure A-17: Evaluation Results - SR-57 N

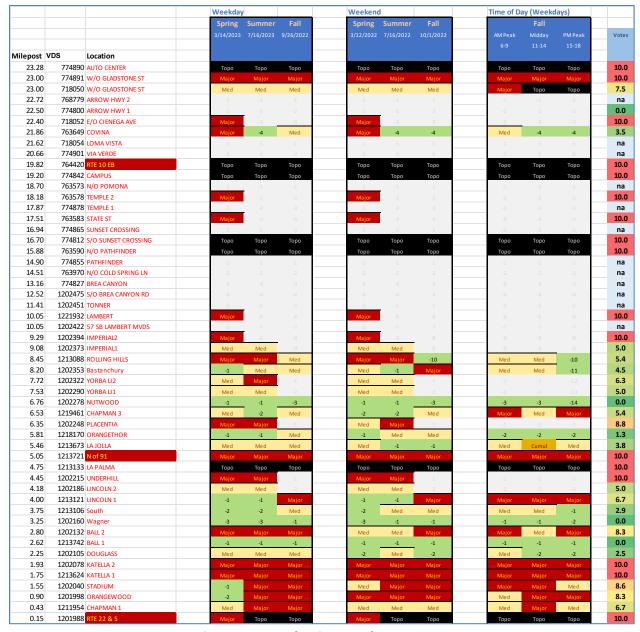


Figure A-18: Evaluation Results - SR-57 S

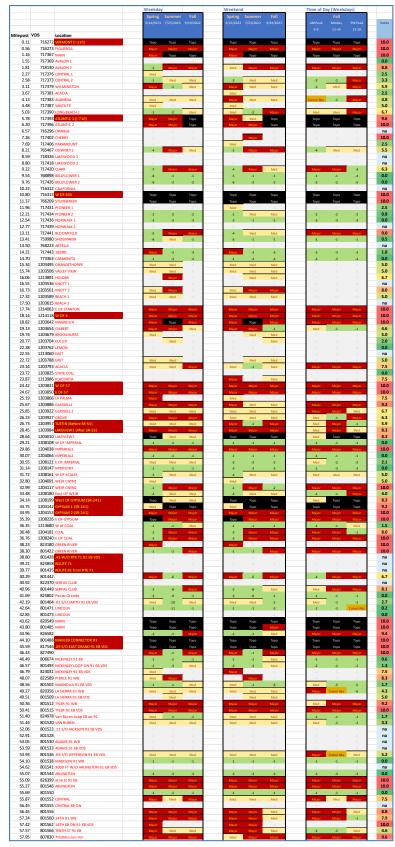


Figure A-19: Evaluation Results - SR-91 E

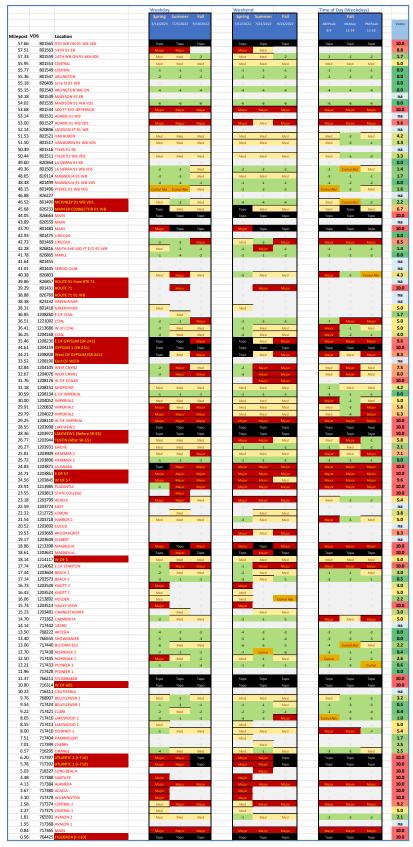


Figure A-20: Evaluation Results - SR-91 W

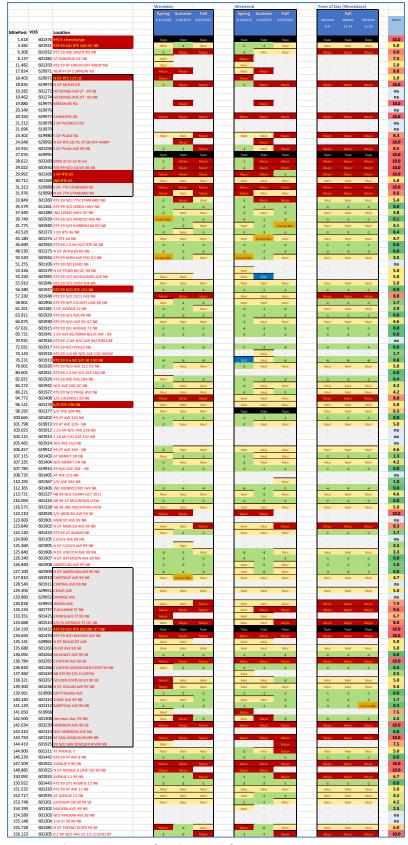


Figure A-21: Evaluation Results - SR-99 N - Part 1

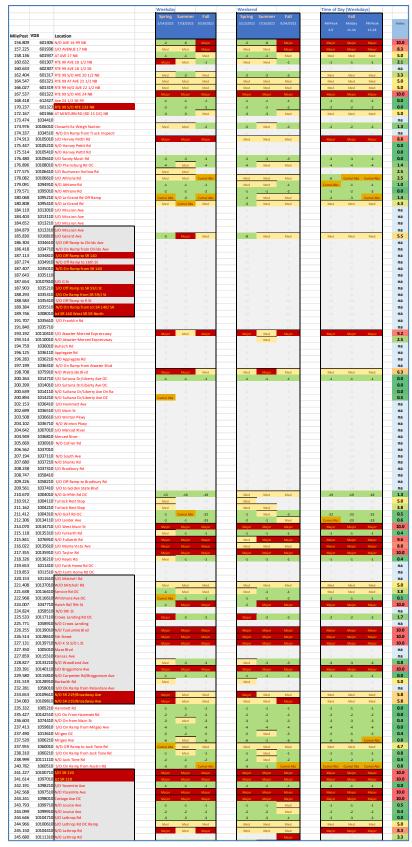


Figure A-22: Evaluation Results – SR-99 N – Part 2

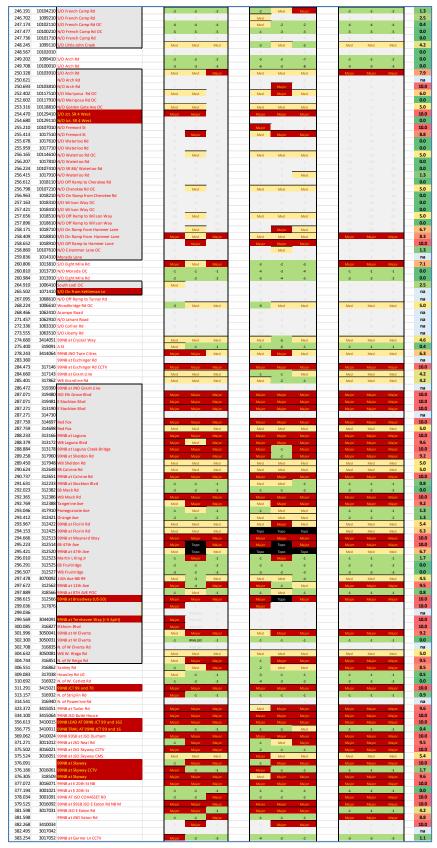


Figure A-23: Evaluation Results - SR-99 N - Part 3

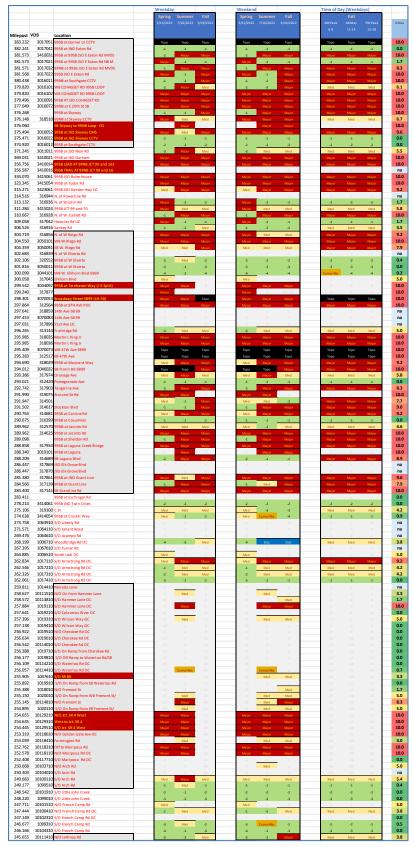


Figure A-24: Evaluation Results - SR-99 S - Part 1

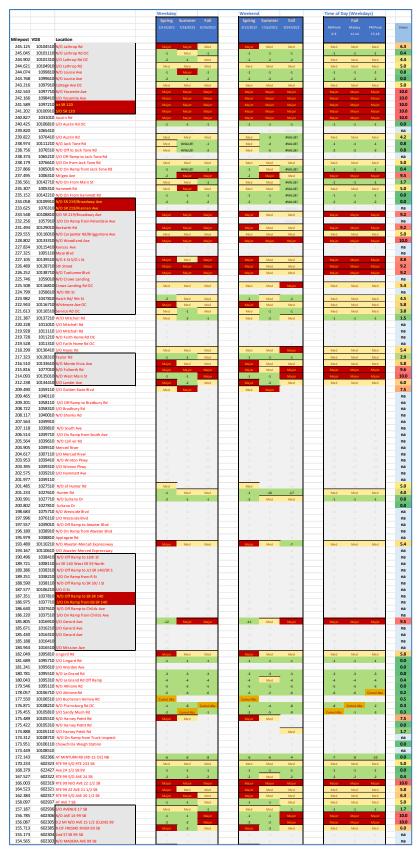


Figure A-25: Evaluation Results – SR-99 S - Part 2

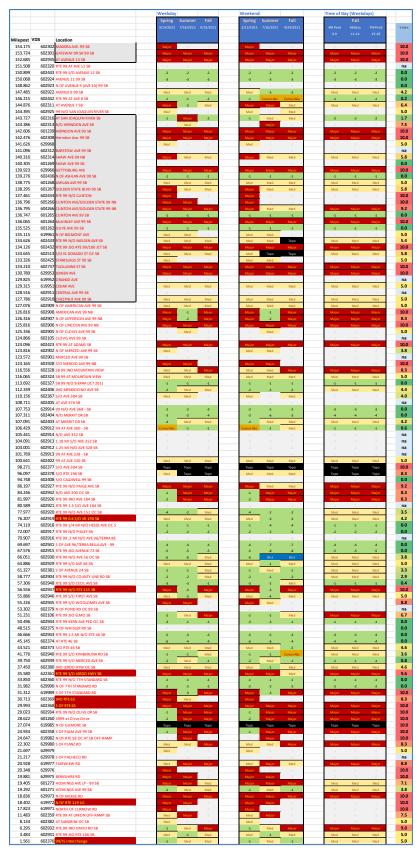


Figure A-26: Evaluation Results – SR-99 S - Part 3

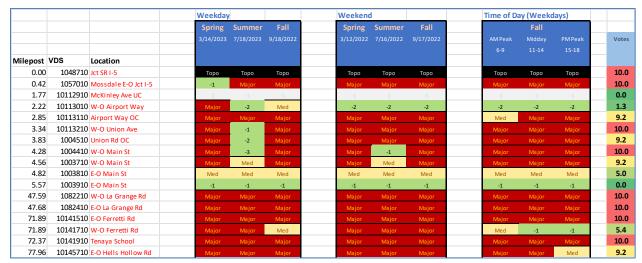


Figure A-27: Evaluation Results - SR-120 E



Figure A-28: Evaluation Results - SR-120 W

APPENDIX B. SURVEY QUESTIONS

Introduction

Why do we need a questionnaire/survey?

Increasing numbers of private firms are leveraging "big data" to provide continually expanding varieties of products, apps, and traffic data solutions. A hybrid data approach with purchased data used to augment existing data collection systems may be the way of the future. A key question being asked by Caltrans is: "How does the availability of privately-sourced traffic data affect traffic sensor deployment strategies of other agencies and state DOTs (Departments of Transportation)?" This questionnaire/survey is designed to address this question.

Who will receive the questionnaire/survey?

This survey is intended for transportation agencies and state DOTs in North America. The project team will reach out to agencies who collect and process data for traffic operations and for reporting purposes.

What kinds of data are covered by this questionnaire/survey?

This survey is focused on the collection of traditional traffic data such as flow, speeds, and densities that are used to calculate performance measures such as vehicle miles and vehicle hours traveled. However, it also includes questions related to more inclusive measures of travel activity including bikes and pedestrians.

Who is sponsoring the study?

The California Department of Transportation (Caltrans) is sponsoring this study. Caltrans collects gigabytes of data every day using dedicated traffic sensing infrastructure. The data provide support for traffic management and system performance monitoring that are crucial for supporting Caltrans mission, vision, and strategic goals to strengthen stewardship and drive efficiency.

How will the results of the study be used?

Caltrans and other State and local agencies may use the information derived from this study to inform decisions regarding their future portfolio of data sources including private "big data" vendors and targeted deployments of traffic sensors.

Who is conducting the study?

California PATH at the University of California Berkeley is a pioneering research organization that has been dedicated to transportation systems operations and traffic engineering since 1986. In partnership with Caltrans, a research team from California PATH will conduct the survey, collect responses, and consolidate the inputs into a summary report.

What are the next steps following the questionnaire/survey?

We estimate the survey will take about 10 minutes to complete. In each question, there may be multiple levels of information that are relevant to the question. The questions asked during the survey may be personalized depending on answers to prior questions. If you need further explanation or clarification regarding the survey questions or the research project itself, please email <melissa.clark@dot.ca.gov>.

Thank you for your time and consideration.

Q2: Select your organization from the list. If your organization is not listed, please select "other"

• Please enter the name of your organization

Performance Measures

Q3: Please rank the relative importance of the following performance measures used for decisions related to traffic operations for your organization today.

- Vehicle Counts / Annual Average Daily Traffic (AADT)
- Truck Classification / Counts
- Vehicle Speeds / Travel Times
- Vehicle Miles Traveled / Vehicles Hours Traveled (VMT/VHT)
- Bicycle Counts
- Pedestrian Counts

Q4: Over the next 10 years, and for your organization, will certain performance measures become more or less important for decision making? If so, which ones?

	Expect more important	Expect about the same	Expect less important
Vehicle Counts / Annual Average Daily Traffic (AADT)			
Truck Classification / Counts			
Vehicle Speeds / Travel Times			
Vehicle Miles Traveled / Vehicle Hours Traveled (VMT/VHT)			
Bicycle Counts			
Pedestrian Counts			

Q5: Is your organization considering the use of new performance measures, not listed? If so, please specify.

Infrastructure-based sensors

The term infrastructure-based sensor refers to a physical sensor such as an inductive loop, radar, or camera that is installed along the right-of-way to obtain traffic data. This data is distinct from "big data" obtained through GPS-based apps or navigational devices that provide location data to third parties. Infrastructure-based sensors can be installed permanently or deployed temporarily, over a study period.

Q7: How many permanently deployed infrastructure-based sensors does your organization maintain? Please provide approximate numbers that provide a good order-of-magnitude.

- Inductive Loops
- Radar
- License plate readers
- Transponder readers (FasTrak/EZ-Pass)
- Cameras
- Weigh-in-Motion Sensors

Q8: Over the next 10 years, and for your organization, will more of the same detectors or alternative detectors to be deployed widely? Please indicate for each sensor type, whether more or fewer are expected to be actively deployed in the future.

	Expect more	Expect about the same	Expect fewer
Overall total number of permanent sensors			
Inductive loops			
Radar			
License plate readers			
Transponder readers (FasTrak/EZ-Pass)			
Cameras			
Weigh-in-Motion Sensors			
Temporary sensors			

Q9: Is your organization considering new sensor types for deployment? If so, which ones?

Purchasing Data from Vendors

Increasing numbers of private firms utilize "big data" to provide continually expanding varieties of products, apps, and traffic and mobility data.

Q11: Does your organization purchase data from private vendors?

- Yes
- No
- Don't know

Q12: From whom does your organization purchase data?

- HERE Technologies
- INRIX
- TomTom
- StreetLight
- Citilabs
- Replica
- Waze
- Other (please specify)

Q13: What kinds of data does your organization purchase?

- Vehicle Flow /Volume
- Vehicle speeds / Travel times
- Vehicle Miles Traveled /Vehicle Hours Traveled (VMT / VHT)
- Trip and Mode Choice
- Origin-Destination Demand Information
- Intersection Turning Counts
- GPS Traces
- Freight movement
- Bicycle counts
- Pedestrian counts
- Other (Please specify)

Q14: Has the purchased data been useful?

- Extremely useless
- Moderately useless
- Slightly useless
- Neither useful nor useless
- Slightly useful
- Moderately useful
- Extremely useful
- Don't know

Future Plans for Purchasing Data

Q15: In the future, does your organization intend to purchase data from private vendors?

- Yes
- No
- Don't know

Q16: From whom does your organization expect to purchase data?

- HERE Technologies
- INRIX
- TomTom
- StreetLight
- Citilabs
- Replica
- Waze
- Other (please specify)

Q13: What kinds of data does your organization intend to purchase?

- Vehicle Flow /Volume
- Vehicle speeds / Travel times

- Vehicle Miles Traveled / Vehicle Hours Traveled (VMT / VHT)
- Trip and Mode Choice
- Origin-Destination Demand Information
- Intersection Turning Counts
- GPS Traces
- Freight movement
- Bicycle counts
- Pedestrian counts
- Other (Please specify)

Q18: How does your organization intend to use this data?

Data Analysis

Raw field data is typically filtered and aggregated to generate performance measures. High quality data from infrastructure-based sensors can be used to scale or to calibrate data from other sources.

Q20: For the data your organization collects, is it easy to relate multiple datasets from multiple (internal and external) sources for comparison and analysis?

- Definitely not
- Probably not
- Might or might not
- Probably yes
- Definitely yes
- Don't know

Q21: Does your organization use data from multiple (internal and external) sources to calculate performance measures to make business decisions?

- Definitely not
- Probably not
- Might or might not
- Probably yes
- Definitely yes
- Don't know

Q22: Does your organization have specifications for allowable errors in purchased data, infrastructure-based sensor data, and in performance measures?

- Definitely not
- Probably not
- Might or might not
- Probably yes
- Definitely yes
- Don't know

Thank you

Q23: Thank you for completing this survey. May we contact you/interview your agency further to help clarify any questions or consider your agency for a potential use case write-up? If so, please provide your contact Information.

- First name
- Last name
- Title
- Organization
- Email
- Phone
- Optional comment