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### Title

The effect of temporal wave averaging on the performance of an empirical shoreline evolution model

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The data associated with this publication are available upon request.

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1     **The Effect of Temporal Wave Averaging on the Performance of an**  
2                     **Empirical Shoreline Evolution Model**

3

4                     **M.A. Davidson, I.L. Turner and R.T. Guza**

5

6                                     **Abstract**

7     The effect of using time-averaged wave statistics in a simple empirical model for  
8     shoreline change is investigated. The model was first calibrated with a six-year time  
9     series of hourly wave conditions and weekly shoreline position at the Gold Coast,  
10    Australia. The model was then recalibrated with the hourly waves averaged over  
11    intervals up to 1 year. With wave averaging up to 2 days, model performance was  
12    approximately constant (squared correlation  $r^2 \sim 0.61-0.62$ ), with only small changes in  
13    the values of empirical model parameters (e.g. the beach response coefficient  $c$  varied  
14    by less than 4%). With between 2 and 40 day averaging, individual storms are not  
15    resolved; model skill decreased only modestly ( $r^2 \sim 0.55$ ), but  $c$  varied erratically by  
16    up to 40% of the original value. That is, optimal model coefficients depend on wave  
17    averaging, an undesirable result. With increased averaging (>40 days) seasonal  
18    variability in the wave field is not resolved well and model skill declined markedly.  
19    Thus, temporal averaging of wave conditions increases numerical efficiency, but  
20    over-averaging degrades model performance and distorts best-fit values of model free  
21    parameters.

22

## 23 **1. Introduction**

24 Coastal management would benefit from realistic prediction of long-term (multi-year)  
25 coastal variability and change. However, such predictions are beyond the capability  
26 of process-based, coastal evolution models [De Vriend et al., 1993; Van Rijn et al.,  
27 2003]. Process models based on detailed physics of hydrodynamic and sediment  
28 transport processes (e.g., Mike21, Delft 3D and Telemac) are hindered at long time-  
29 scales by both excessive computation time and poor model accuracy.

30 To bypass these difficulties, empirical models with reduced computation loads have  
31 been developed recently for shoreline position [e.g., Miller and Dean, 2004, Yates et  
32 al., 2009], sandbar location [e.g., Plant et al. 2006, Pape et al 2010] and beach  
33 gradient [e.g., Madsen and Plant, 2001]. Computational speed is obtained through  
34 both drastic simplification of the underlying equations, and through larger model time  
35 steps.

36 Accuracy is (hopefully) provided by extensive model calibration. However, the  
37 impact of using time-averaged wave parameters on shoreline model skill is unclear.  
38 Here, a six-year, highly temporally resolved (hourly waves and weekly shoreline  
39 position) data set is used to investigate the impact of wave-averaging on the  
40 performance of a simple empirical model for shoreline evolution [Davidson, Lewis  
41 and Turner (2010), hereafter DLT10].

42 Yates et al. (2009) developed a shoreline model similar to DLT10, based on wave  
43 energy disequilibrium, and presented preliminary evidence that excessive wave  
44 averaging degrades model performance by blurring the time history of storm waves.

45 For example, averaging wave parameters over the time period between sand level  
46 surveys (weekly to monthly) vastly simplified the numerics of calculating optimal  
47 values of model free parameter compared with hourly wave measurements, but model  
48 performance was reduced substantially. Owing to the numerical complexity of  
49 finding optimal free parameters of this model, the tipping points for model  
50 degradation as functions of the degree of wave averaging were not established. This  
51 illustrates the need to ascertain limits on wave averaging, even with simple empirical  
52 models for coastal change. The DLT10 numerics for optimal free parameters are  
53 much simpler, and therefore allow straightforward investigation of the impact of wave

54 averaging on model performance over a broad range of time-scales. DL10 is viewed  
55 here as a generic, fast, empirical model for shoreline evolution.

56 The transfer functions for linear running average filters are well known. The cut-off  
57 characteristics are notoriously broad and are described well using the Dirichlet  
58 function. The (-3dB) cut-off frequency may be approximated by  $0.433/Mdt$ , where  $dt$   
59 is the sampling interval and  $M$  is the number of points in the averaging window.  
60 Thus, the impact of the filter stretches to frequencies that are considerably lower than  
61 the reciprocal averaging window duration ( $1/Mdt$ ).

62 The field site and observations are described in Section 2. Although the shoreline  
63 model itself is not the topic of the present paper, the model is briefly reviewed in  
64 section 3 for clarity, and the reader is referred to DLT10 for further information. The  
65 effect on model performance of increased temporal averaging of the wave field is  
66 presented in Section 4. Conclusions and implications for further model development  
67 and application are summarised in Section 5.

68

## 69 **2. Observations**

70 A six-year record of wave and shoreline data from the Gold Coast, located on the SE  
71 Australian coastline is used (see Davidson and Turner, 2009, for details.) Wave  
72 parameters are reported hourly from a wave-rider buoy located approximately 2 km  
73 offshore of the study site in 16 m of water. Mean sea level shoreline locations are  
74 extracted weekly from a coastal video system. The shoreline data are averaged over  
75 500 m of the coastline to remove small-scale variability. Waves are energetic with  
76 significant offshore wave heights exceeding 7 m, and annual shoreline displacements  
77 exceed 50 m. The comparative spectral distribution of variance in the shoreline and  
78 hydrodynamic (dimensionless fall velocity) time-series are shown in Figure 1. Here  
79 spectral estimates have been computed after de-trending the data and application of a  
80 Hanning window. The spectral estimates have 19 degrees of freedom and a bandwidth  
81 of 0.0028 Hz. Both the shoreline and hydrodynamic spectra are red in form,  
82 dominated by a seasonal peak at 0.0028 cycles/d. Variance of the shoreline  
83 displacement is roughly divided between seasonal/interannual, trend and storm as  
84 55%, 35% and 10% respectively (DLT10). Note that the hydrodynamic spectrum of  
85 dimensionless fall velocity has significantly more high frequency (0.01 cycles/d to 0.1

86 cycles/d) content than the shoreline series. There is a small diurnal peak in the fall  
87 velocity spectrum at 1 cycle/d, but very little variance above this point.

88 The tidal range is microtidal with spring tidal ranges of 1.8 m. The beach sediments  
89 have a median grain size and mean fall velocity of 0.25 mm and 0.03 m/s  
90 respectively.

91

### 92 **3. Model**

93 The 1-D scheme of DLT10 (building upon the earlier 2-D ‘behavioural-template’  
94 scheme of Davidson and Turner, 2009) was used to investigate the impact of temporal  
95 wave-averaging on empirical shoreline evolution models. The cross-shore shoreline  
96 position  $x$  at time  $t$  is:

$$97 \quad \frac{dx}{dt} = b + c(\Omega_0 - \Omega)\Omega \quad (1)$$

98 where  $\Omega$  is the time-varying dimensionless fall velocity ( $= H/\omega T$ ),  $\omega$  is the sediment  
99 fall velocity,  $T$  is the peak wave period and  $H$  is the significant offshore wave height.  
100  $\Omega_0$  is the time-averaged, equilibrium dimensionless fall velocity that causes no net  
101 shoreline change in equation 1, (DLT10). A linear shoreline trend (if present) is  
102 given by  $b$ . The rate of shoreline change in response to time-varying wave forcing is  
103 governed by the reciprocal response time coefficient ( $c$ ), wave steepness ( $H/T$ ), and  
104 the disequilibrium magnitude ( $\Omega_0 - \Omega$ ). Although other empirical schemes (refer  
105 Section 1) could have been chosen here, the model represented by Equation 1 is  
106 simple and transparent, computationally efficient, stable over long (decadal) model  
107 runs, and most importantly, skilfully hindcasts seasonal and multi-year shoreline  
108 change at the test case site (DLT10).

109

110 Temporal analytical integration of Equation 1 includes antecedent conditions and  
111 enables an analytic solution for the least squares calibration of the three unknown  
112 coefficients; a constant shoreline offset  $a$  (units of m), a linear trend  $b$  ( $\text{ms}^{-1}$ ) and the  
113 shoreline response parameter  $c$  ( $\text{ms}^{-1}$ ) (DLT10).

114

115 To isolate the affect of using different wave averaging times, the model time-step ( $dt$ )  
116 was held constant at 1 hour. The averaging period ( $\Delta$ ) for the forcing wave data ( $T$   
117 and  $H$ ) was progressively increased from hourly (as observed) up to 1 year. For each  
118  $\Delta$ , the model was re-calibrated yielding values for model coefficients  $a$ ,  $b$  and  $c$ , and a  
119 hindcast of the 6-year shoreline position. Model performance relative to the observed  
120 weekly shoreline measurements was quantified by the squared correlation ( $r^2$ ). The  
121 transfer function for a 2 and 40 day moving average filter function is also included in  
122 Figure 1, so that the influence of the filter on model forcing parameters may be fully  
123 appreciated. Notice that the impact of the filter function encompasses much lower  
124 frequencies than one might intuitively expect. The temporal integration of ordinary  
125 differential equation (1) leads to downshifting of the frequency response, thus  
126 propagating the impact of time-averaging forcing parameters to still lower  
127 frequencies. Thus, it is unclear, without numerical experimentation such as this, what  
128 the impacts of frequency averaging on predictions of shoreline response will be.

129

#### 130 **4. Results**

131 Using hourly waves and optimal values for free parameters, the model captures both  
132 the seasonal variability and the rapid shoreline retreat associated with energetic  
133 storms at the start of 2001, 2004 and 2006 (Figure 2). However, the model fails to  
134 reproduce all the high frequency variability in the observed shoreline location and the  
135 squared model-data correlation  $r^2 \sim 0.62$ .

136 Model performance, and the value of optimal model free parameters, varies as wave  
137 averaging is increased from 1 hour to 1 year (Figure 3). With wave averaging up to 2  
138 days, model performance is approximately constant (squared correlation  $r^2 \sim 0.6$ ), with  
139 only small changes ( $< 4\%$ ) in the reciprocal response time,  $c$ . Thus, the time step can  
140 be increased (from hourly) by a factor 50, without degrading model performance or  
141 substantially distorting free parameter values. With between 2 and 10 day averaging  
142 individual storms are not resolved; model skill decreased only modestly ( $r^2 \sim 0.55$ ),  
143 but  $c$  varies erratically by up to 45% of the hourly value. With further increases in  
144 averaging ( $> 40$  days), seasonal variability in the wave field is not resolved and model  
145 skill declines markedly. Brier skill scores, using the linear trend as the base  
146 prediction (not shown), are very similar to  $r^2$ .

147 Pape et al (2010) showed that a model for sand bar location, with structure similar to  
148 the present shoreline model (1), is sensitive to wave averaging that blurs storms. For  
149 both shoreline and sandbar location models, temporal averaging of wave conditions  
150 increases numerical efficiency, but over-averaging degrades model performance  
151 and/or distorts best-fit values of model free parameters (e.g. response time).

152

## 153 **5. Conclusion**

154 Time-averaging of the waves forcing morphologic change models must be done  
155 carefully. For the wave climate at the Gold Coast test site, model performance  
156 deteriorates with averaging between 2-10 days, as short-duration storm events become  
157 poorly resolved. The model skill again degrades with wave averaging greater than  
158 about 40 days, as seasonal variations are progressively smoothed.

159 Declining model hindcast skill and variation in model optimal free parameter values  
160 resulting from time-averaging of the seasonal wave component is more significant  
161 than the impact of averaging over individual storms. This is consistent with the  
162 distribution of shoreline variance in this dataset: seasonal/interannual band (55%)  
163 with relatively small contributions at storm frequency (10%).

164 Another likely contributing factor was that, although the model when forced with  
165 hourly wave parameters successfully predicts the larger shoreline recession events  
166 associated with the major storms in this time-series (start of 2001, 2004 and 2006 -  
167 Figure 2), it does not reproduce all of the observed high frequency variability. With  
168 an alternative model that better predicts high-frequency shoreline variability; the  
169 impact of averaging over storm times-scales will be more significant. Similarly,  
170 smoothing over storms may be more detrimental at other coastal sites where storm  
171 frequency variance contributes a higher percentage of the total shoreline variance.  
172 Storms and seasonality are the two most important drivers of wave-forced shoreline  
173 change, so it is anticipated that the two key time-average thresholds ( $\geq 2$  days and  $\geq 40$   
174 days) corresponding to the initial and further degradation of model skill and  
175 fluctuation in free parameter values, are likely more generically applicable to other  
176 models and sites. This assertion warrants further investigation.

177

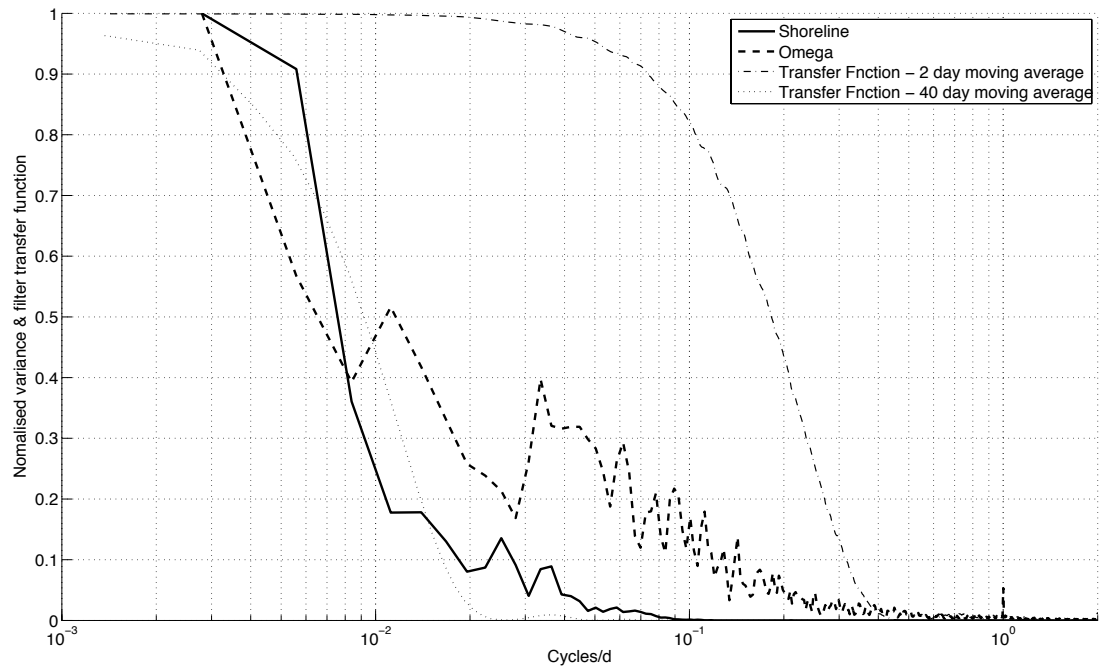
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182 station from which weekly shorelines were derived. The Gold Coast wave data was  
183 provided by QLD EPA.

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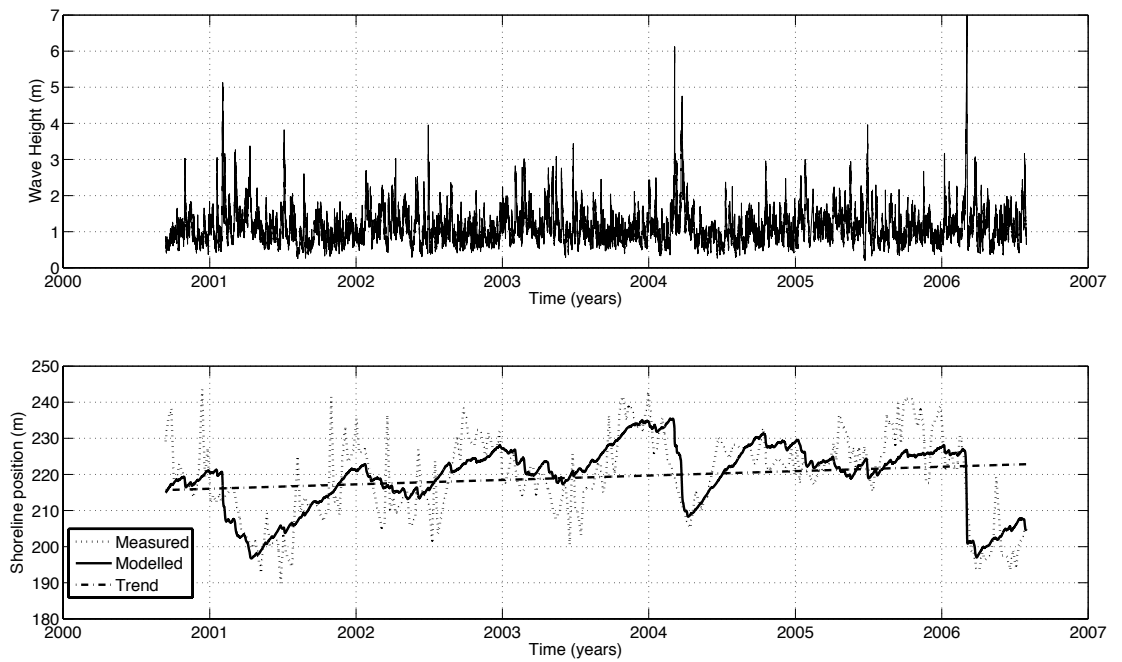




208

209 Figure 1. Spectral estimates of shoreline position and dimensionless fall velocity  
 210 (omega) plotted together with moving average filter transfer functions with  
 211 windows of 2 and 40 days.

212



214

215 Figure 2. (top) Observed hourly significant offshore wave height, and (bottom)

216 shoreline positions observed (dotted) and modelled (solid). The dashed

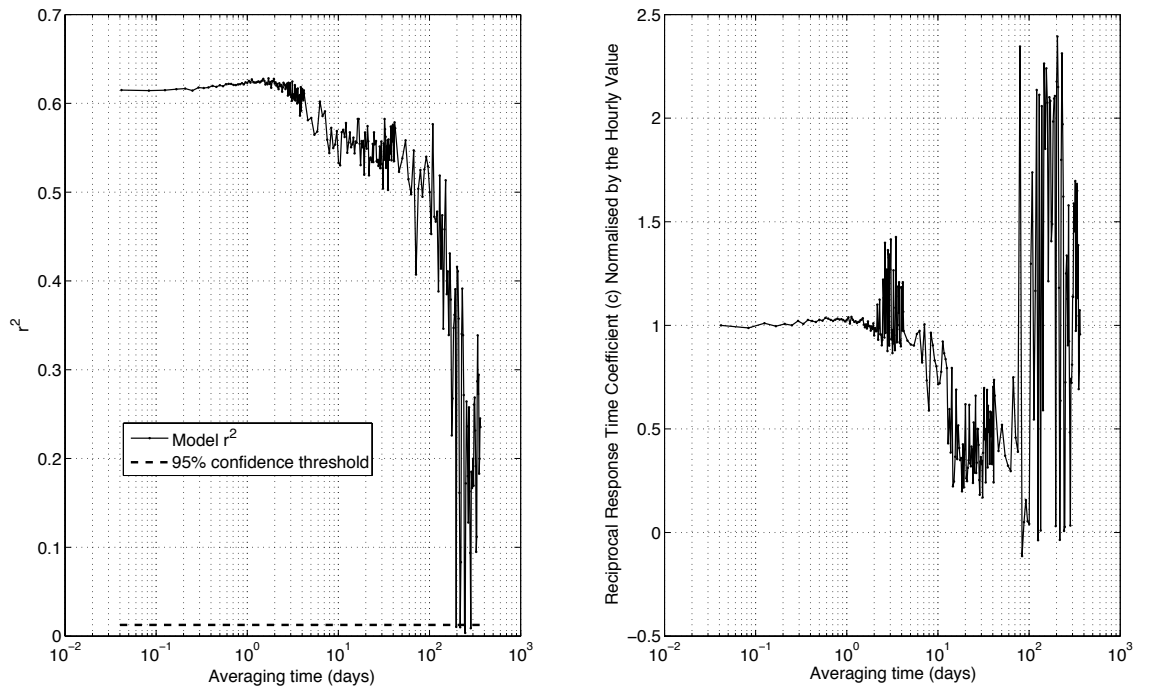
217 broken line is a linear trend.

218

219

220

221



222

223 Figure 3. (left) Squared correlation ( $r^2$ ) between model and observed shoreline  
224 position and 95% confidence threshold (dotted). (right) Reciprocal response  
225 coefficient  $c$ , normalised by the value for no wave averaging (hourly  
226 sampled wave parameters), versus wave averaging time (days) used in the  
227 model.

228