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### Title

Valley-scale morphology drives differences in fluvial sediment budgets and incision rates during contrasting flow regimes

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1 Valley-scale morphology drives differences in fluvial sediment budgets and incision  
2 rates during contrasting flow regimes

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15

16 **Abstract**

17 High-resolution topographic surveys using LiDAR and multibeam sonar can be used to  
18 characterize and quantify fluvial change. This study used repeat surveys to explore how  
19 topographic change, fluvial processes, sediment budgets, and aggradation and incision  
20 rates vary across spatial scales and across two contrasting decadal flow regimes in a  
21 regulated gravel/cobble river. A novel method for quantifying digital elevation model  
22 uncertainty was developed and applied to a topographic change detection analysis from  
23 2006/2008 to 2014. During this period, which had four modest ~3-5 year floods, most  
24 sediment was laterally redistributed through bank erosion and channel migration.  
25 Erosion primarily occurred in the floodplain (97,000 m<sup>3</sup>), terraces (80,000 m<sup>3</sup>), and  
26 lateral bars (58,000 m<sup>3</sup>); while deposition occurred in the adjacent pools (73,000 m<sup>3</sup>),  
27 fast glides (48,000 m<sup>3</sup>), and runs (36,000 m<sup>3</sup>). In contrast, significantly higher magnitude  
28 and longer duration floods from 1999 to 2006/2008 caused sediment to be displaced  
29 longitudinally, with the upstream reaches exporting sediment and the downstream  
30 reaches aggrading. The river maintained floodplain connectivity during both periods,  
31 despite different processes dominating the type of connectivity. Larger floods promoted  
32 overbank scour and avulsion, while smaller floods promoted bank erosion and lateral  
33 migration. This study explores and illustrates how the geomorphic response to  
34 contrasting flood regimes in a nonuniform river is highly dependent on which landforms  
35 are controlling hydraulics.

36 *Keywords:* topographic change detection; DEM differencing; river morphology; fluvial  
37 sediment budgets

## 38 1. Introduction

39 Gravel/cobble-bedded rivers with diverse fluvial landforms exist worldwide (Shen et al.,  
40 1981; Buffington and Montgomery, 1999; Wheaton et al., 2015) and have been highly  
41 degraded by cumulative anthropogenic impacts on all aspects of their  
42 ecogeomorphology (Ligon et al., 1995; Liébault and Piégay, 2001; Hancock, 2002; Lisle  
43 and Church, 2002). Nevertheless, these systems remain important because of their  
44 ecosystem functions and services, including aquatic habitat provision and renewal,  
45 riparian inundation, fish passage, sediment and nutrient outflux, local land loss, flood  
46 control, water supply, and navigation (e.g., Boulton et al., 2008; Arthington et al., 2010).

47 While much effort has been put into understanding the physics, geomorphology, and  
48 sediment transport of uniform gravel river channels (often using rectangular, uniform  
49 flumes; e.g., Parker, 1979; Wilcock, 1997; Millar, 2005), Ashworth and Ferguson (1986)  
50 observed a lack of studies that described and explained nonuniform rivers, including  
51 their morphodynamic patterns and mechanisms. That essential finding holds today,  
52 though meaningful progress has been made with flume experiments (e.g., Wu and Yeh,  
53 2005; Wilkinson et al., 2008; Tal and Paola, 2010), numerical modeling (e.g., Nicholas  
54 et al., 2013; Oorschot et al., 2015), and field-based approaches (e.g., Carbonneau et  
55 al., 2012; Pasternack and Wyrick, 2016). More recently, Kleinhans (2010) reviewed  
56 studies of river patterns and concluded that the remaining inability to obtain dynamic  
57 meandering and braiding in laboratory experiments and physics-based models is  
58 indicative of our lack of understanding of morphodynamic mechanisms. Furthermore,  
59 White et al. (2010) compiled literature and evidence that make the case against  
60 assuming uniform flow for understanding many gravel rivers, and Gonzalez and

61 Pasternack (2015) showed that cross-sectional sampling as a ubiquitous methodology  
62 in fluvial geomorphology does not yield representative gravel-river attributes the way it  
63 is often assumed. Clearly, different approaches are needed to take the next steps to  
64 improve our understanding of nonuniform gravel/cobble rivers.

65 Thanks to technological advances (Passalacqua et al., 2015), submeter-scale  
66 topographic mapping of shallow gravel/cobble rivers has rapidly escalated with the  
67 emergence of commercially available multibeam sonar (MBS) systems (Hazel et al.,  
68 2010; Hensleigh, 2014), airborne bathymetric LiDAR (Hilldale and Raff, 2008;  
69 Mandlburger et al., 2015), terrestrial laser scanning (Vericat et al., 2014; Williams et al.,  
70 2014), structure-from-motion collected with unmanned aerial vehicles (Westoby et al.,  
71 2012; Fonstad et al., 2013; Javernick et al., 2014), and multispectral remote sensing  
72 data (Legleiter et al., 2009). These surveying methods allow researchers to explore the  
73 diversity and patterning of fluvial landforms (e.g., Wyrick and Pasternack, 2014; Casado  
74 et al., 2015; Brown and Pasternack, 2017), simulate how nonuniform topography drives  
75 a diversity of hydraulic patch behaviors (Strom et al., 2016), and more accessibly obtain  
76 repeat topographic surveys in support of topographic change detection (TCD) to reveal  
77 fluvial morphodynamics (e.g., Wheaton et al., 2013; Mandlburger et al., 2015; Wyrick  
78 and Pasternack, 2015).

79 Many gravel/cobble rivers behave differently than assumed because they have multiple  
80 layers of nested topographic heterogeneity (Gangodagamage et al., 2007; Brown and  
81 Pasternack, 2017). This means that the topographic patterns that steer flow within the  
82 base-flow channel are not the same as those for the larger bankfull channel or the

83 floodprone valley– drawing on three common scales of scientific interest. This concept  
84 of stage-dependent topographic steering of flow produces patches of diverse hydraulic  
85 behaviors that change as flow increases and different landforms yield or take control of  
86 flow paths (Macwilliams et al., 2006; Strom et al., 2016). Brown and Pasternack (2014)  
87 applied this concept to explain topographic change and described bedload deposition  
88 mechanisms in a mountain river. This finding provides the impetus to explore fluvial  
89 morphodynamics at multiple sites, across a range of discharges, and using high-  
90 resolution topographic mapping to better understand the spatial structure of fluvial  
91 topographic change.

92 Considering this emerging understanding, we present and analyze novel field results  
93 comparing how fluvial morphodynamics in a topographically heterogeneous gravel-bed  
94 river corridor differ when forced by two significantly different hydrologic regimes. Both  
95 regimes involved a similar average annual runoff; but in the first period the water was  
96 distributed with larger floods and lower base flows, while the second period had long  
97 durations of moderate base flows and a few short, modest floods. This study illustrates  
98 how the geomorphic response to contrasting flood regimes in a nonuniform river is  
99 highly dependent on which landforms are activated and for how long.

### 100 *1.1. Modern topographic change detection methods*

101 In a topographic change detection (TCD) analysis (sometimes referred to as a  
102 geomorphic change detection analysis or GCD), two digital elevation models (DEMs)  
103 are differenced, and the raw results are adjusted based on an estimate of uncertainty  
104 within each DEM. Currently, most TCD analyses involve differencing raster-based

105 DEMs with spatially distributed error estimates. Researchers generally use one of the  
106 following strategies to account for DEM uncertainty: applying the fuzzy inference system  
107 (FIS) presented by Wheaton et al. (2010), using bootstrapping or Monte Carlo  
108 simulation to create multiple realizations of the DEM (Wechsler and Kroll, 2006;  
109 Wheaton et al., 2008), developing error functions based on the survey method and  
110 measures of topographic variability (Heritage et al., 2009; Milan et al., 2011; Carley et  
111 al., 2012), or using a geostatistical approach such as kriging (Kraus et al., 2006;  
112 Mandlbürger et al., 2015). All the methods above represent legitimate ways to  
113 incorporate uncertainty into a TCD analysis.

114 No proposed method can unequivocally account for the uncertainty associated with a  
115 sampled topographic surface. Thus, each method has its limitations. The FIS relies on  
116 user-defined error thresholds and values to apply to combinations of categories (e.g.,  
117 low/high point density or gentle/steep slopes) to prescribe a level of vertical elevation  
118 uncertainty. This provides a framework for incorporating multiple layers of uncertainty,  
119 but submeter resolution data sets may allow the development of deterministic statistical  
120 measures of uncertainty, which reduce the subjective nature of FIS. Bootstrapping  
121 methods are useful for interpreting the sensitivity of the resulting topographic map by  
122 subsampling the data, but bootstrapping methods are computationally expensive to  
123 create confidence intervals for large data sets. Field campaigns that relate spatial  
124 uncertainty to topographic variables are infeasible for large topographic surveys (e.g.,  
125 >10 km of river corridor) and using past reported values may be insufficient given the  
126 unique characteristics of each landscape and survey method. Lastly, surfaces made  
127 using kriging do not adhere to the actual survey point data, which may not be

128 appropriate for high-resolution data in steep and topographically complex terrain. With  
129 these concerns in mind, this study developed a new deterministic and efficient approach  
130 to characterize uncertainty using traditional statistical metrics at the raster-cell level.

### 131 *1.2. Study objectives*

132 The purpose of this study was to use the lower Yuba River (LYR), a regulated gravel-  
133 cobble river in northern California, as a testbed to explore gravel/cobble river  
134 morphodynamics during two decadal time periods with contrasting flow regimes. The  
135 specific scientific objectives were to compare topographic change at multiple spatial  
136 scales during each time period by (i) differentiating sediment budgets, (ii) calculating  
137 rates of vertical change for different inundated areas, and (iii) identifying the landforms  
138 that were the sources and sinks of sediment.

139 The two time periods analyzed were 1999 to 2006/2008 (time period 1) and 2006/2008  
140 to 2014 (time period 2). Carley et al. (2012), Wyrick and Pasternack (2015), and  
141 Pasternack and Wyrick (2016) detailed the topographic change within LYR for time  
142 period 1, a period that included an ~23-year flood event (see section 2.4). This study  
143 analyzes topographic change in LYR using high-resolution topographic surveys  
144 (airborne NIR and bathymetric LiDAR and boat-based multibeam sonar) for time period  
145 2, a period with four modest 3-5 year flood events (see section 2.4). Where applicable,  
146 we compared the results from time period 2 to the results for time period 1. Thus, the  
147 overall goal of this study was to revisit LYR, analyze topographic change during time  
148 period 2, and quantify fluvial topographic change and sediment budgets at multiple  
149 scales: the river segment (~100-1000 channel widths), geomorphic reach (~10-100

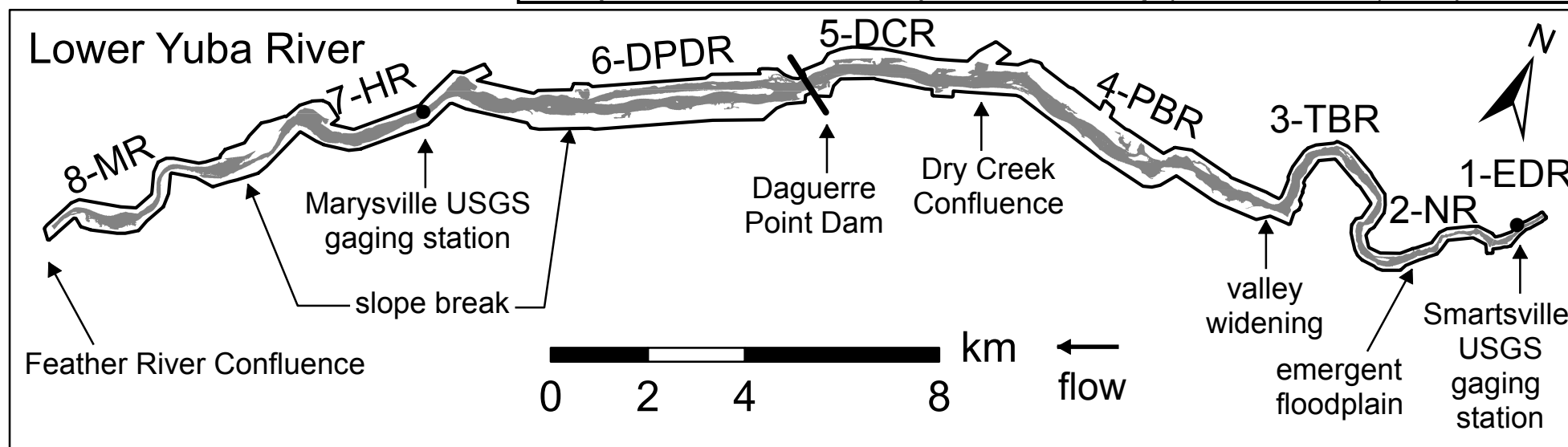
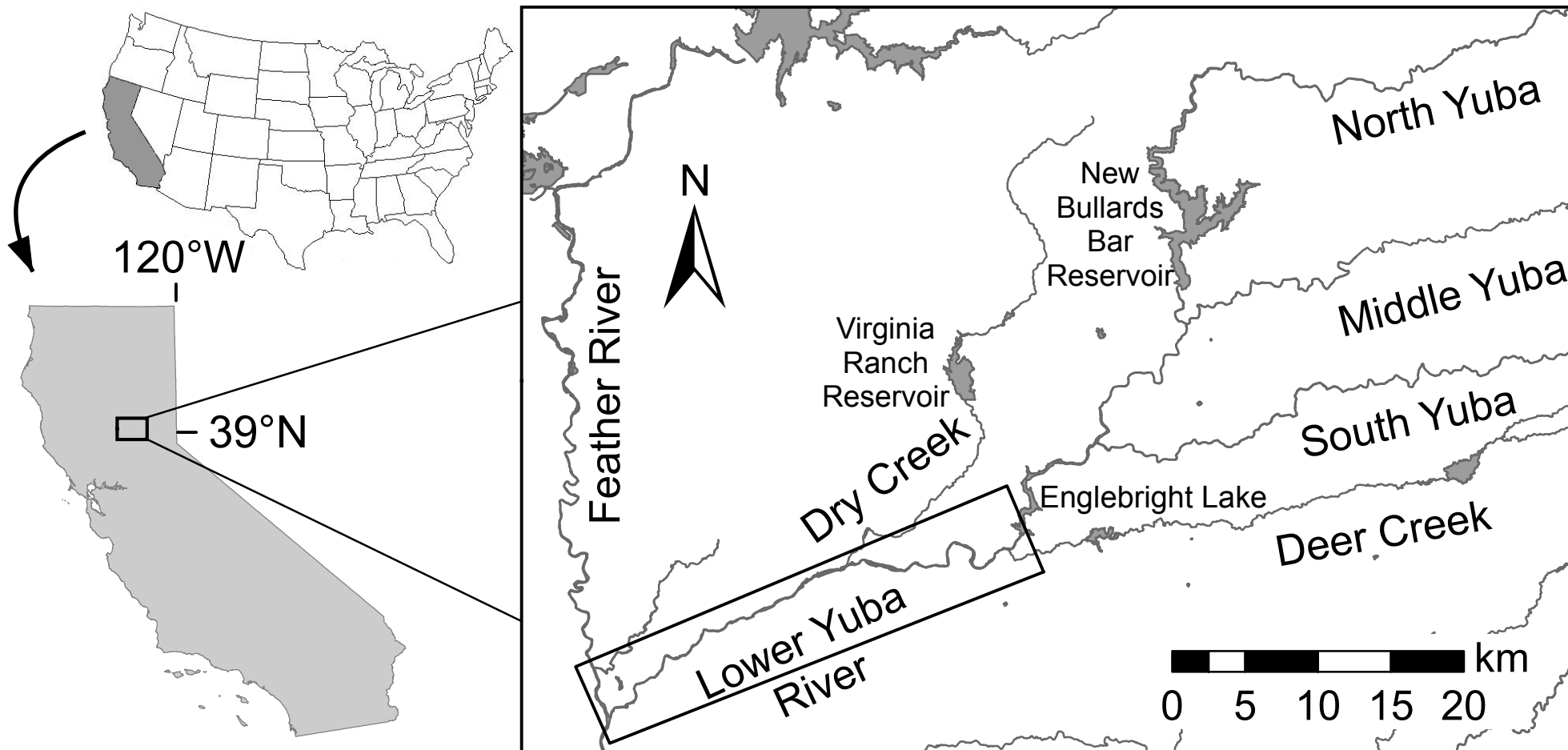


150 channel widths), and morphological unit (~0.1-10 channel widths) scales. We  
151 hypothesized that stage-dependent topographic steering would yield significantly  
152 different patterns of topographic change at each of the spatial scales for the two  
153 periods.

154 In the following sections, we provide an overview of LYR, topographic data collection  
155 efforts, and the analytical methods for this paper. More information on LYR, topographic  
156 data processing, and topographic change uncertainty methods are provided in the  
157 supplementary materials.

## 158 **2. Study site – lower Yuba River**

159 The Yuba River is a tributary of the Sacramento River in north-central California that  
160 drains 3480 km<sup>2</sup> of the western Sierra Nevada range (Fig. 1). The study segment is the  
161 alluvial lower Yuba River (LYR), a 37.1-km stretch that flows from east to west  
162 downstream of Englebright Dam to its confluence with the Feather River. This river  
163 segment is largely a single-threaded channel with ~20 emergent bars/islands at bankfull  
164 discharge, low sinuosity, high width-to-depth ratio, and slight to no entrenchment  
165 (Wyrick and Pasternack, 2012). Flows into LYR come primarily from three tributaries:  
166 the North, Middle, and South Yuba Rivers. Although the North Yuba tributary has a  
167 large reservoir that heavily regulates its outflow year-round, the absence of large  
168 reservoirs on the Middle and South Yuba tributaries translates to a broad range of  
169 discharges for LYR. Flows frequently (i.e., nearly annually) overtop Englebright Dam  
170 during large winter storms and spring snowmelt. Daguerre Point Dam (DPD), an 8-m-  
171 high run-of-the-river dam, is located near the middle of the study segment, 17.8 river



172 kilometers (Rkm) upstream from the Feather River. Storage behind DPD is filled with  
173 sediment, allowing bedload to pass downstream during flood events.

#### 174 *2.1. Geomorphic reaches*

175 Wyrick and Pasternack (2012) identified eight geomorphic reaches within LYR based on  
176 the location of tributary junctions and changes in lateral confinement and bed slope. The  
177 Englebright Dam reach (1-EDR) and Narrows reach (2-NR) define the upper portion of  
178 LYR, where the river corridor is confined within a steep-walled bedrock canyon.

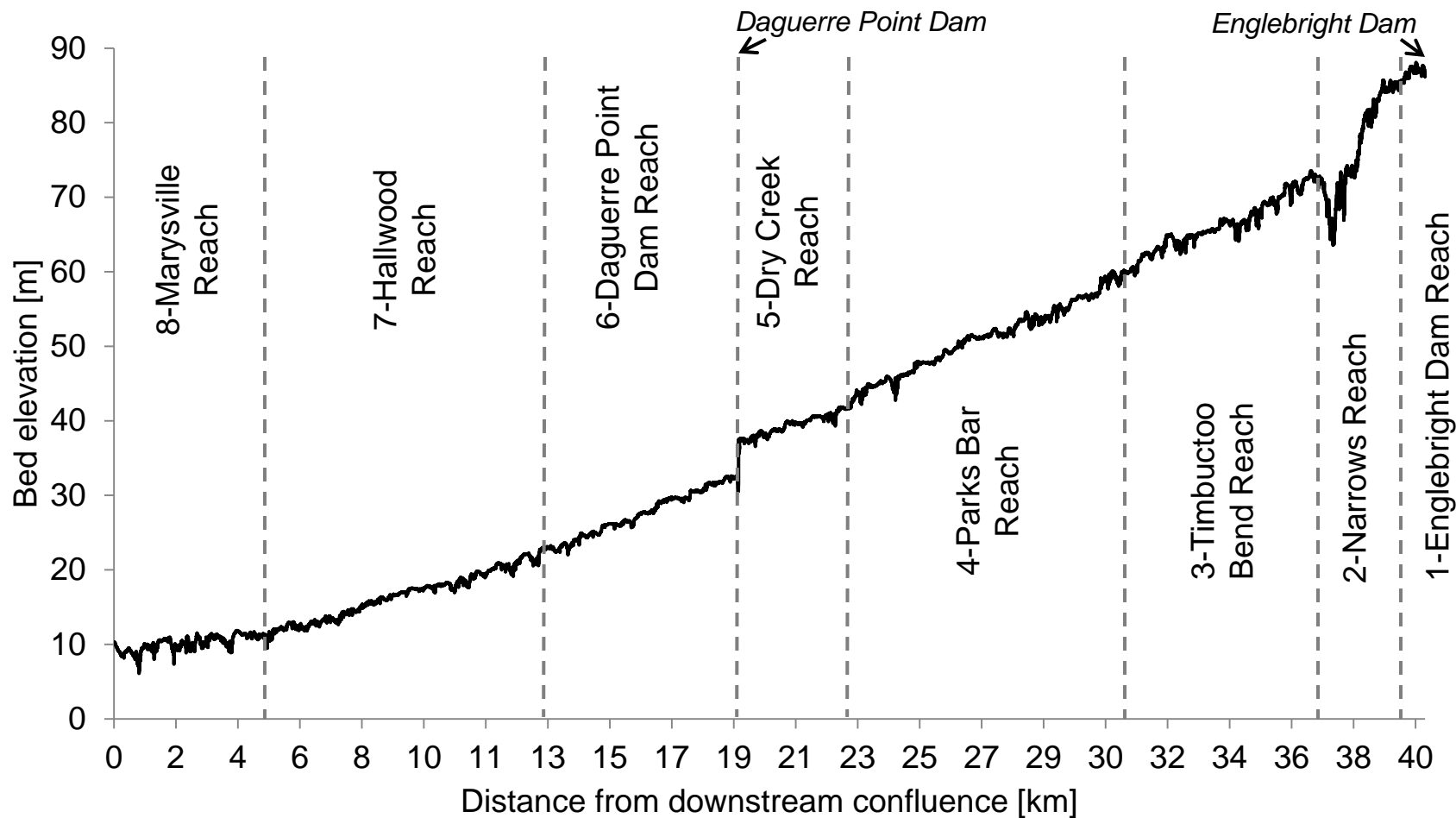
179 Because of the lack of alluvium, these reaches are excluded from the TCD analysis.

180 The Timbuctoo Bend reach (3-TBR) marks the emergence of the alluvial river valley,  
181 followed by significant valley widening in the Parks Bar reach (4-PBR). The Deer Creek  
182 reach (5-DCR) and Daguerre Point Dam reach (6-DPDR) have the widest floodplain.

183 Finally, in the Hallwood reach (7-HR) and Marysville reach (8-MR), the bed slope  
184 significantly decreases; and the river corridor becomes laterally constrained by levees  
185 that protect the City of Marysville. Geomorphic reach breaks are presented in Figs. 1  
186 and 2, and more information on the geomorphic reaches is provided in the  
187 supplementary materials.

#### 188 *2.2. Morphological units*

189 Morphological units (MUs) are distinct landforms delineated at the scale of ~0.1-10  
190 channel widths. In-channel bed MUs (e.g., pool, riffle, and chute) for LYR were  
191 delineated by Wyrick et al. (2014) using a two-dimensional (2D) hydrodynamic model of  
192 the 2006/2008 topography at a base-flow discharge. Other MUs within the active valley  
193 corridor (e.g., medial bar, floodplain, and terrace) were mapped by Wyrick and



194 Pasternack (2012) with the aid of 2D hydrodynamic modeling of flood discharges, which  
195 were reported by Barker (2012) and Abu-Aly et al. (2014). Thirty-one MUs were  
196 delineated and grouped into four categories defined by their inundation thresholds: in-  
197 channel bed MUs (inundated at a base-flow discharge), in-channel bank MUs  
198 (inundated at bankfull discharge), floodway MUs (inundated at the floodway filling  
199 discharge), and valley MUs (areas outside the floodway). A description of each of the  
200 MUs is provided in the supplementary materials.

### 201 *2.3. Hydraulic mining and geomorphic history*

202 The LYR has a complex geomorphic history because of the cumulative impacts of  
203 several historical human activities: deposition of large volumes of fill derived from  
204 historic hydraulic gold mining in the watershed (Gilbert, 1917; Adler, 1980; James,  
205 2005; James et al., 2009), dredging of the ~4000-ha Yuba Goldfields and other areas in  
206 the ancestral river migration belt (James et al., 2009), installation of a high concrete  
207 arch sediment-barrier dam (Englebright Dam) in the canyon at the entrance to LYR in  
208 1941 (Snyder et al., 2004, 2006), and moderate flow regulation from a suite of  
209 hydroelectric facilities throughout the catchment.

210 From 1852 to 1906, LYR aggraded by as much as 26 m near the exit of 3-TBR,  
211 declining to ~8 m of aggradation near the confluence with the Feather River (Adler,  
212 1980). This large wedge of hydraulic mining sediment is up to 5 km in width through an  
213 area called the Yuba Goldfields. The California Debris Commission estimated that the  
214 LYR valley accumulated 10,380,000 m<sup>3</sup> of mining sediment, nearly 90% of which was  
215 still contained within the valley in 1980 (Adler, 1980). This period of rapid aggradation

216 turned LYR into an anastomosing channel that was void of vegetation. Aggradation was  
217 followed by rapid channel incision after hydraulic mining ceased and the California  
218 Debris Commission began installing sediment control structures to limit sediment from  
219 being exported downstream and provide flood protection for the City of Marysville. Adler  
220 (1980) calculated that the channel from 1906-1912 underwent 33.5 cm/y of incision,  
221 which tapered off to an average of 6.4 cm/y from 1912 to 1979. Pasternack and Wyrick  
222 (2016) recently reported mean incision rates from 1999 to 2006/2008 with 3-TBR and 5-  
223 DCR incising ~4.5 and 5.9 cm/y, respectively. In the same study, the other geomorphic  
224 reaches analyzed were slightly aggrading, with 4-PBR, 6-DPDR, 7-HR, and 8-MR  
225 averaging ~0.1, 1.9, 1.6, and 0.1 cm/y of aggradation, respectively.

#### 226 *2.4. Flow regime*

227 Hydrological records for LYR are available from three USGS flow gages: Smartsville  
228 (#11419000) below Englebright Dam, Marysville (#11421000) near the confluence with  
229 the Feather River, and Deer Creek (#11418500), a tributary. Pasternack et al. (2014)  
230 conducted a flood frequency analysis using data from 1970-2010. Relevant flows for  
231 this paper include 28.32 m<sup>3</sup>/s as the representative base flow, 141.6 m<sup>3</sup>/s as the  
232 estimated bankfull discharge, and 597.5 m<sup>3</sup>/s as the floodplain-filling discharge. The  
233 return period of the bankfull discharge is ~1.25 years, which is more frequent than other  
234 similar rivers. The implication of this (and the high frequency of floodplain-filling flows) is  
235 that the channel may be undersized and flows spill onto the floodplain more often than  
236 expected.

237 Despite time period 1 (1999-2006/2008) and time period 2 (2006/2008-2014) having  
238 nearly similar total water and average annual flows (Table 1), the two time periods  
239 provide a contrast in the timing, duration, and magnitude of peak floods (Fig. 3). For the  
240 current study period (time period 2), four floodplain-filling events occurred, ranging from  
241 838.2 to 1246 m<sup>3</sup>/s instantaneous flow, which corresponds to ~3-5 year recurrence  
242 interval events (Wyrick and Pasternack, 2012) and 6-9 times bankfull discharge. These  
243 floods were short-duration events spread over three years. The prior TCD analysis (time  
244 period 1) included a maximum recorded instantaneous flow of 3207 m<sup>3</sup>/s on 31  
245 December 2005, which corresponds to a 23-year event and 22 times bankfull discharge.  
246 This event was preceded by a flood of 1480 m<sup>3</sup>/s on 20 May 2005 and was followed by  
247 a series of three floods ranging from 637.0 to 1056 m<sup>3</sup>/s in April 2006. Thus, time period  
248 1 experienced three floods of much greater magnitude over a shorter period than time  
249 period 2. The duration above bankfull discharge was 200 days for time period 1 and 163  
250 days for time period 2. The duration above floodplain-filling discharge was 18 days for  
251 time period 1 and 4 days for time period 2. Table 1 provides an overview of the peak  
252 annual flows for each time period, the average annual volume of water, and a  
253 characterization of each water year with respect to historical averages.

### 254 **3. Methods**

255 This study analyzed and compared topographic data from two high-resolution  
256 topographic surveys, one conducted in 2006/2008 and one conducted in 2014. To  
257 address the study objectives, we developed a new spatially distributed method for  
258 estimating topographic uncertainty for raster DEMs. This uncertainty analysis was  
259 applied to the two topographic data sets and the DEM difference raster computed from

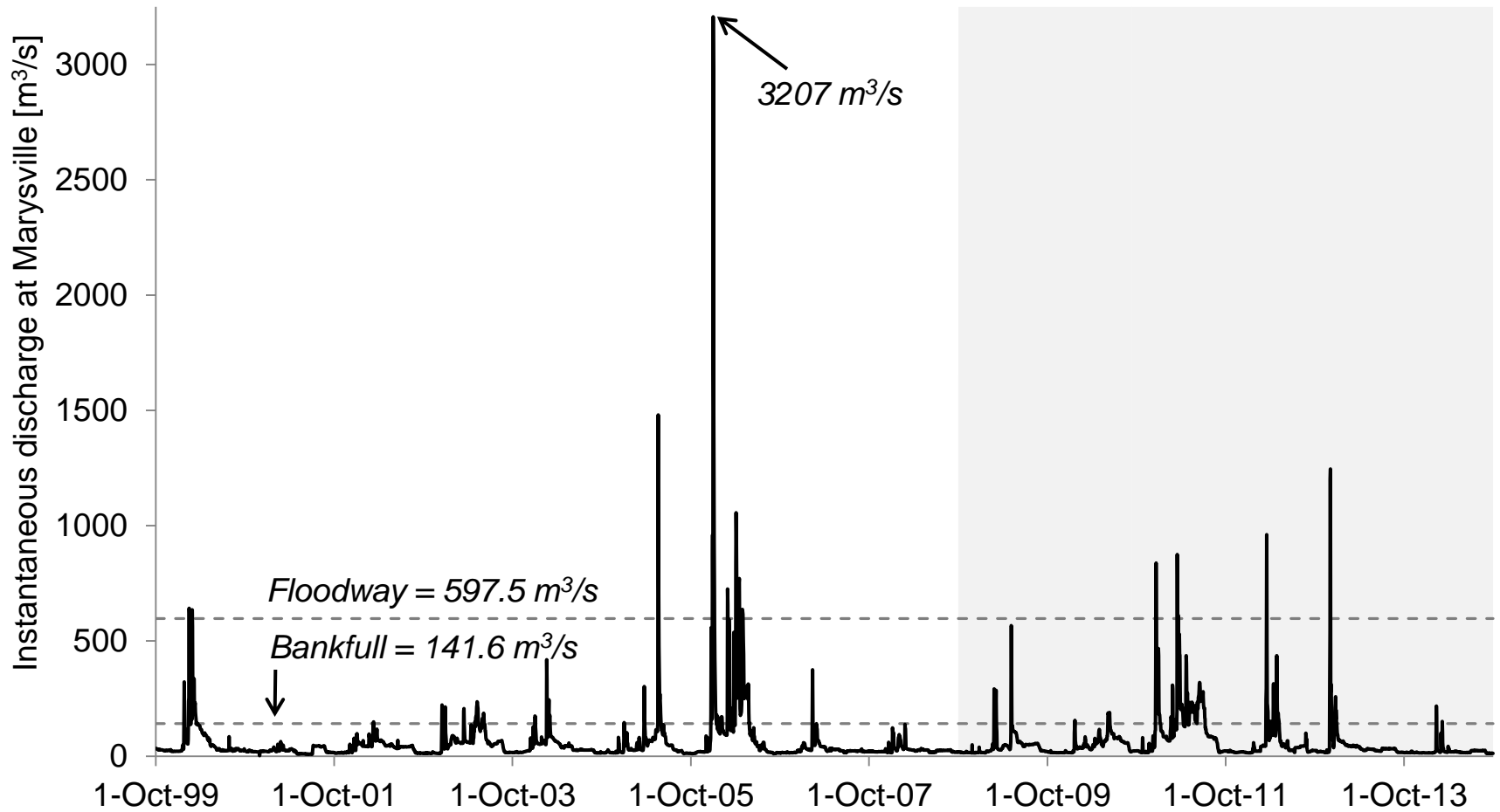
Table 1

Summary of water year<sup>a</sup> statistics for the study period (time period 2 is shaded)

Water year <sup>a</sup>	Instantaneous peak (m <sup>3</sup> /s)	Daily avg. peak (m <sup>3</sup> /s)	Avg. volume (10 <sup>6</sup> m <sup>3</sup> /y)	Water year type
2000	642.0	608.8	1779.0	Above Average
2001	63.5	55.8	696.0	Dry
2002	149.3	142.7	1241.0	Below Average
2003	236.2	231.1	1830.0	Above Average
2004	418.9	345.5	1354.0	Below Average
2005	1480.0	1229.0	1829.0	Above Average
2006	3207.0	2384.0	4794.0	Wet
2007	375.0	283.2	904.0	Dry
2008	138.2	130.3	817.0	Dry
2009	566.3	356.8	1243.0	Below Average
2010	189.6	181.2	1422.0	Below Average
2011	875.0	761.7	3837.0	Wet
2012	961.1	603.1	1367.0	Below Average
2013	1246.0	628.6	1265.0	Below Average
2014	217.8	178.1	595.0	Dry
Period 1 ('00-'08)	3207.0	2384.0	1694.0	N/A
Period 2 ('09-'14)	1246.0	761.7	1621.0	N/A

<sup>a</sup>The California water year begins on 1 October and ends on 30 September. For example, water year 2000 began on 1 October 1999 and ended on 30 September 2000.





260 them. Volumetric sediment budgets were calculated at multiple spatial scales to assess  
261 the patterns of erosion and deposition within LYR. The sediment budget for LYR  
262 assumes no sediment input from the upper watershed, as Englebright Dam blocks all  
263 bedload and Deer and Dry Creeks contribute negligible amounts of sediment. In  
264 addition, no landslides or other significant lateral sediment fluxes occurred from outside  
265 the TCD region. Thus, the sediment budget must result in net erosion with the value  
266 yielding the net volumetric export of sediment out of LYR to the Feather River.

267 At each of three relevant spatial scales, the river segment (~100-1000 channel widths),  
268 the geomorphic reach (~10-100 channel widths), and the morphological unit (~0.1-10  
269 channel widths) scales, a series of analyses were conducted to assess the river's  
270 topographic adjustment. At the segment scale, longitudinal profiles of volumetric change  
271 and percent area of change aimed to provide an understanding of how patterns of  
272 erosion and deposition are organized. At the segment and geomorphic reach scales,  
273 the 2008 bankfull channel was delineated and used to stratify results into in-channel vs.  
274 overbank areas. This analysis aimed to provide insight into sediment connectivity  
275 between the channel and the floodplain and to help answer whether morphological  
276 diversity is maintained through channel migration or avulsion events. Furthermore,  
277 vertical topographic change rates were calculated across incrementally wetted areas  
278 provided by a 2D hydrodynamic model. These results were designed to explicitly test  
279 whether the river is becoming more entrenched by comparing the rate of vertical change  
280 at different inundation levels. Finally, at the morphological unit scale, volumetric  
281 sediment budgets were calculated to reveal the landforms that are the sources and  
282 sinks of sediment within LYR.

283 Notably, a different set of survey methods, point densities, and error analyses were  
284 used for time period 1 (Carley et al., 2012). Uncertainties for time period 1 are higher,  
285 as the first survey in 1999 did not involve meter-scale topographic mapping with  
286 terrestrial LiDAR. Nevertheless, the values reported are the best estimates of  
287 topographic change within LYR for each time period.

### 288 3.1. Topographic data collection

#### 289 3.1.1. 2006/2008 surveys

290 For the 2006/2008 DEM, 3-TBR was surveyed in 2006 using a robotic total station for  
291 terrestrial areas and wadable bathymetry, and boat-mounted single-beam sonar (SBS)  
292 was used for unwadable bathymetry. The reaches downstream of 3-TBR were primarily  
293 surveyed in 2008. Aero-Metric, Inc. (Seattle, WA) flew airborne near-infrared (NIR)  
294 LiDAR for terrestrial mapping on 21 September 2008 when flows were  $24.4 \text{ m}^3/\text{s}$  above  
295 DPD and  $17.6 \text{ m}^3/\text{s}$  below DPD (the difference caused by irrigation diversions at DPD).  
296 River bathymetry was primarily mapped by SBS, except for inaccessible areas that  
297 were surveyed by a real-time kinematic global positioning system (RTK-GPS) or robotic  
298 total station. The 2006/2008 surveying effort was reported by Carley et al. (2012) and is  
299 summarized in the supplementary materials.

#### 300 3.1.2. 2014 Surveys

301 The 2014 DEM involved a new survey of LYR during an extended drought that left LYR  
302 at its lowest point since new environmental flow schedules were fully implemented by  
303 the Lower Yuba River Accord in 2008. Quantum Spatial, Inc. (Sheboygan, WI) collected  
304 airborne bathymetric LiDAR data (using combined NIR and green laser scanners) on 27

305 September 2014 when flows were 15.3 m<sup>3</sup>/s above DPD and 11.3 m<sup>3</sup>/s below DPD.  
306 Low flows allowed more of the river valley to be mapped by the NIR laser and reduced  
307 the depths required for the green laser to penetrate through the water column. To map  
308 the riverbed at depths greater than the green laser could penetrate, Seafloor Systems,  
309 Inc. (El Dorado Hills, CA) collected MBS data on 11-14 August 2014, when flows were  
310 40.0 m<sup>3</sup>/s above DPD and 22.7-24.5 m<sup>3</sup>/s below DPD. The higher water levels facilitated  
311 overlapping of MBS and LiDAR data. Several days were spent using SBS and RTK-  
312 GPS survey points to map data gaps and areas where submerged vegetation precluded  
313 accurate aerial and sonar mapping. A full account of 2014 mapping efforts and post-  
314 processing of data is included in the supplementary materials. Data resolutions by land  
315 cover type are presented in Table 2 for the 2006/2008 DEM and the 2014 DEM.

### 316 3.2. DEM uncertainty for the 2006/2008-2014 TCD

317 Even though topographic data involves points, a raster cell is commonly the  
318 fundamental unit for most topographic change analyses. Topographic change is  
319 reported when elevation change within a raster cell,  $\Delta Z_r$ , exceeds the level of detection  
320 (*LOD*) determined by the DEM uncertainty method.

321 The DEM uncertainty method developed for this study creates confidence limits for each  
322 raster cell using a traditional statistical approach for Gaussian distributions by  
323 calculating the standard error of the mean (*SEM*), yet this has not been done before in  
324 the published literature. The approach uses the density of the survey data, *N* (which is  
325 the number of surveyed ground points contained by each raster cell) and the standard  
326 deviation, *SD* (which is an estimate of the variability of the survey data, to calculate the

Table 2

Data resolution (pts/m<sup>2</sup>) comparisons by land-cover type

Land cover	2006/2008 DEM	2014 DEM
Bare Earth	5.71	13.17
Water	0.59	5.12
Vegetated Ground	1.37	3.05

327 *SEM*; Eq. 1). The *SEM* is governed by a simple equation that is functionally equivalent  
328 to doing statistical bootstrapping, where a sample mean is calculated from a specified  
329 number of observations (*N*) that are pulled randomly from a population of data with a  
330 given standard deviation (*SD*). If this is done enough times, the resulting distribution of  
331 errors in sample mean is characterized by the *SEM*, which follows a normal distribution.  
332 An *SEM* is calculated for each survey period (Eq. 1) for every raster cell, and then the  
333 two *SEM* rasters are combined at the 95% confidence interval to obtain the *LOD* (Eq.  
334 2),

$$335 \quad SEM_{2014} = \frac{SD_{2014}}{\sqrt{N_{2014}}} \quad (1)$$

$$336 \quad LOD_{95} = \sqrt{(1.96 * SEM_{2008})^2 + (1.96 * SEM_{2014})^2} \quad (2)$$

337 where the subscripts on the right side of the equation are the survey year (Lane et al.,  
338 2003), and *SD* is an estimate of the variability of the data.

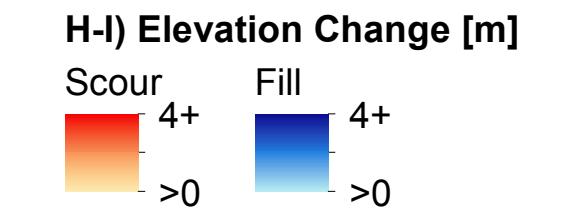
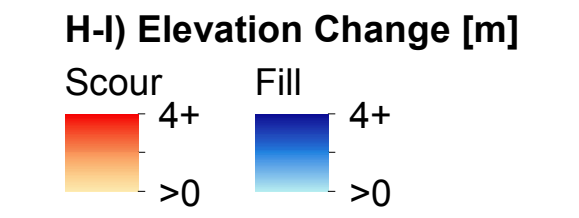
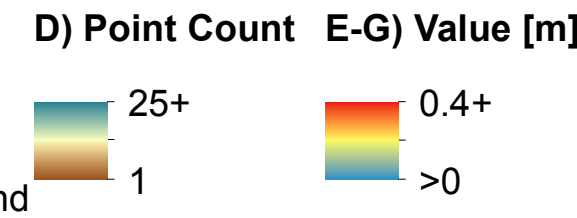
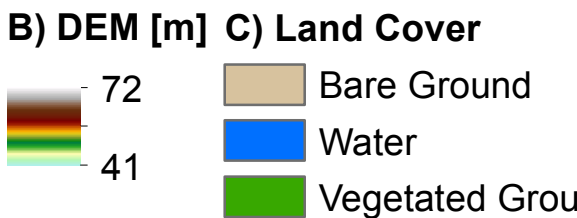
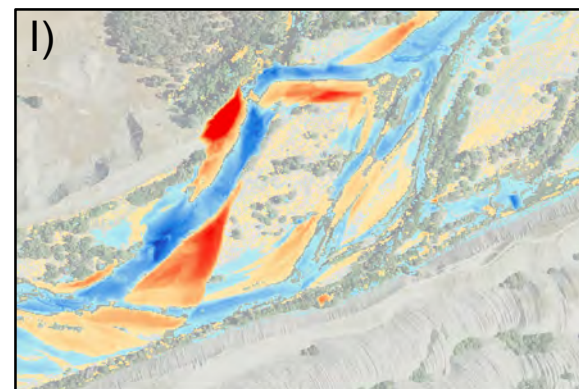
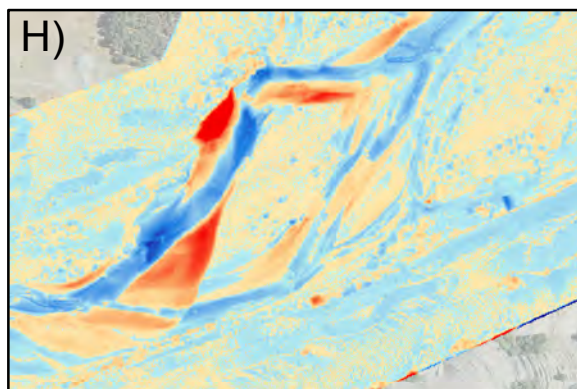
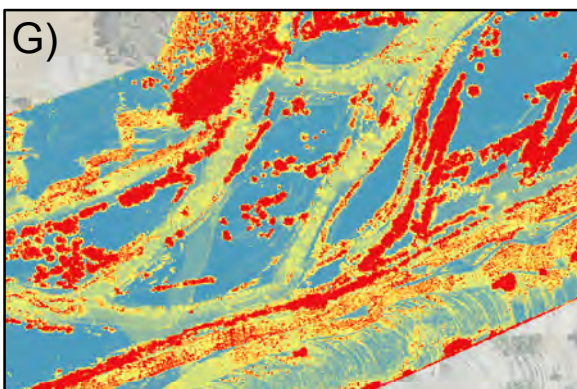
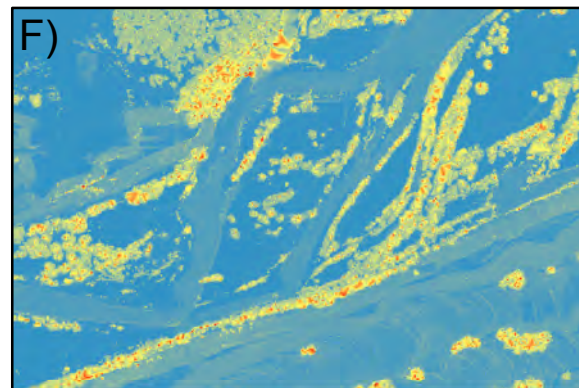
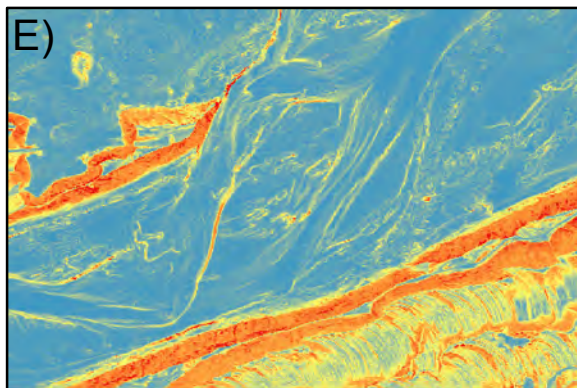
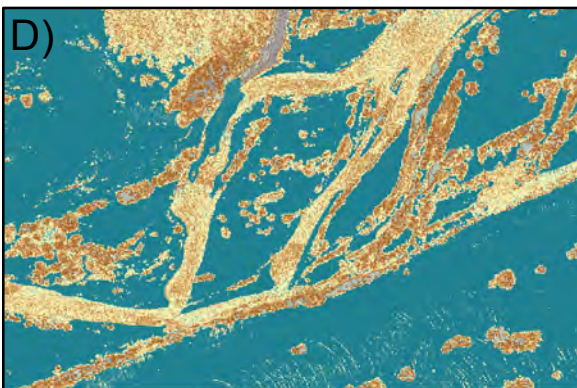
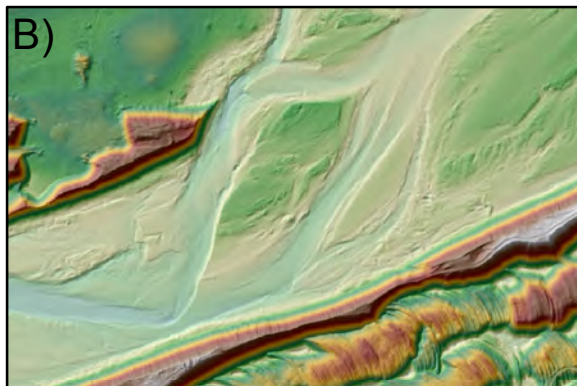
339 To apply Eqs. (1) and (2), *SD* is needed for each raster cell. This variability can come  
340 from at least two sources: topographic variability (e.g., substrate size or slope) or survey  
341 error (*SE*). For remote sensing methods like LiDAR and MBS, a significant proportion of  
342 the *SE* involves distinguishing between ground vs. nonground returns, which depends  
343 on the land cover. For this method, *SD* is determined by incorporating both sources of  
344 variability. Topographic variability, *SD<sub>Z</sub>*, is calculated from the ground-classified point  
345 cloud for the area contained by each raster cell as well as by the method outlined in

346 Carley et al. (2012). The larger of the two  $SD_Z$  values is used. The  $SEs$  were  
347 calculated for different land cover classes (bare ground, water, and vegetated ground)  
348 and represent the ability for a survey method to correctly identify ground within each  
349 land cover. A detailed explanation of how the  $SEs$  were calculated is included in the  
350 supplementary materials. Lastly, both sources of variability are combined by taking the  
351 square root of the sum of the squares (Eq. 3):

$$352 \quad SD_{2014} = \sqrt{(SD_{Z_{2014}})^2 + (SE_{2014})^2} \quad (3)$$

353 An overview of the steps involved for the DEM uncertainty method is included below,  
354 and a visualization of the outputs is provided in Fig. 4.

- 355 1. Use a LiDAR or point cloud software package (e.g., LAStools or TerraScan) to  
356 classify all survey points as ground, vegetation, structure, or noise.
- 357 2. Use the ground classified points to create a raster DEM where  $Z_r$  is the height in  
358 each raster cell (Fig. 4B).
- 359 3. Create a land-cover raster (Fig. 4C) from the point classifications, and assign the  
360 survey error ( $SE$ ) associated with each land-cover class.
- 361 4. Calculate the topographic variability,  $SD_Z$ , in each raster cell using the  
362 approach by Milan et al. (2011) or Brasington et al. (2012) (Fig. 4E).
- 363 5. Calculate an estimate of the total variability,  $SD$ , by applying Eq. (3).
- 364 6. Create a raster with the number of ground points per raster cell,  $N$  (Fig. 4D).
- 365 7. Calculate the standard error of the mean,  $SEM$ , using Eq. (1) (Fig. 4F).





- 366 8. Complete the steps above for each year's DEM. Then calculate the 95%  
367 confidence level of detection,  $LOD_{95}$ , using Eq. (2) (Fig. 4G).
- 368 9. Difference the two DEMs (Fig. 4H) and subtract the  $LOD_{95}$  raster values from the  
369 raw results to yield the statistically significant erosion or deposition that occurred  
370 within each raster cell (Fig. 4I).

### 371 3.3. *Sediment Budgeting*

372 After completing the DEM uncertainty analysis and subtracting the  $LOD_{95}$  from the raw  
373 DEM-difference raster, sediment budgets were created at river-segment, geomorphic-  
374 reach, and morphological-unit scales. This computation assumed no nontransport  
375 mechanisms of volumetric change, previously termed bed *deflation* or *contraction* and  
376 *inflation* or *dilation* (Merz et al., 2006; Marquis and Roy, 2012). In addition, if a raster  
377 cell erodes and then fills in between surveys, no change will be detected in that cell: a  
378 process known as compensating scour and fill (Lindsay and Ashmore, 2002).

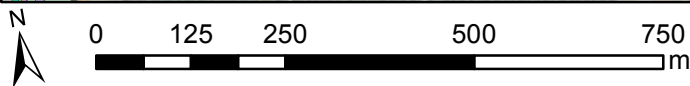
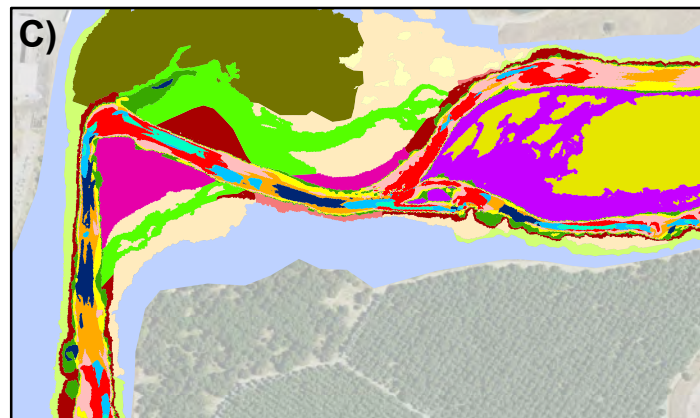
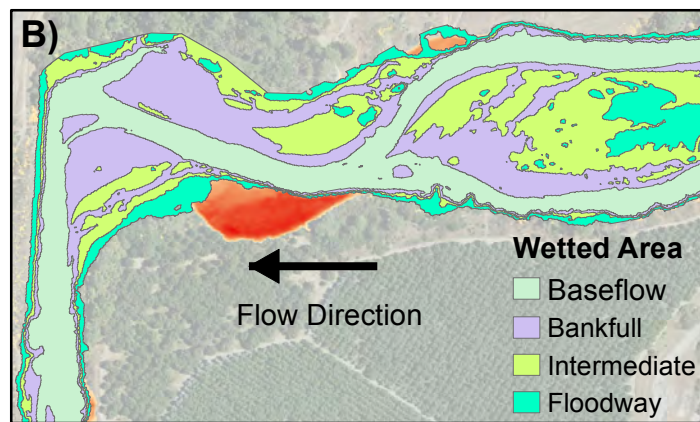
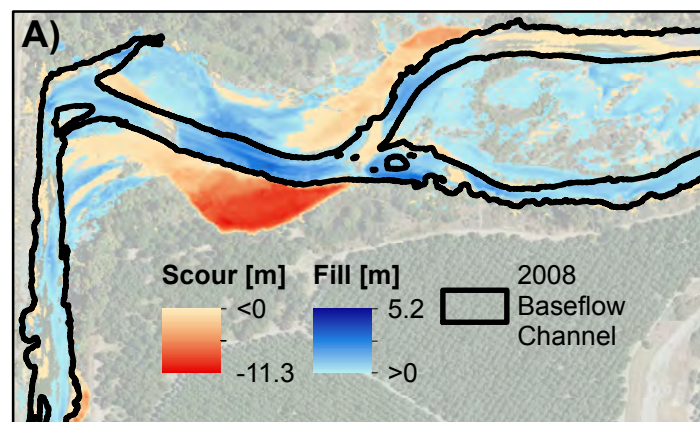
#### 379 3.3.1. *River-Segment Scale*

380 Longitudinal profiles of erosion and deposition were created by delineating the valley  
381 centerline in ArcGIS and creating regularly spaced rectangular station boxes (shown in  
382 the supplementary materials). The station boxes are orthogonal to the valley centerline,  
383 span the width of the river valley, and are 1.524 m in width. Their position along the  
384 valley centerline represents the distance upstream from the confluence with the Feather  
385 River. The total volume [ $m^3$ ] of erosion and deposition within each station box was  
386 calculated, as well as the percent area of erosion, deposition, or no detectable level of  
387 change within the floodway. To explore the correlation between locations of peak

388 erosion and deposition, the Pearson's product-moment correlation was calculated using  
389 the longitudinal series of erosion and deposition volumes. The correlation values range  
390 from -1 to 1, with positive values indicating that the locations of peak erosion and  
391 deposition coincide. A negative value indicates that peak locations of erosion coincide  
392 with the low values of deposition and vice versa. Lastly, the volumetric rate of change  
393 [ $\text{m}^3/\text{y}$ ] for the entire study area was calculated for erosion and deposition. The difference  
394 between these two values represents the average rate of sediment eroded out of the  
395 LYR valley.

### 396 3.3.2. *Geomorphic-Reach Scale*

397 At the geomorphic-reach scale, the volumetric rate of change [ $\text{m}^3/\text{y}$ ] was calculated for  
398 each geomorphic reach to understand the relative balance of erosion and deposition  
399 within each reach. Because 3-TBR was mapped in 2006, it is averaged over 8 years  
400 (2006-2014), while the rest of LYR is averaged over 6 years (2008-2014). Results were  
401 also stratified across in-channel vs. overbank areas. The in-channel areas are defined  
402 by a wetted area polygon determined by a 2D hydrodynamic model of the 2006/2008  
403 topography at bankfull discharge ( $141.6 \text{ m}^3/\text{s}$ ). Lastly, net rates of vertical change were  
404 estimated by calculating the net volumetric rate of change [ $\text{m}^3/\text{y}$ ] within a wetted area  
405 polygon developed by the 2D hydrodynamic model at several discharges (28.32, 141.6,  
406 283.2, and  $597.5 \text{ m}^3/\text{s}$ ; see below) and dividing by the inundated area [ $\text{m}^2$ ] of the model  
407 to yield the average vertical change per year [ $\text{mm}/\text{y}$ ]. This analysis was applied to the  
408 areas that are incrementally wetted as discharge increased from  $28.32 \text{ m}^3/\text{s}$  (base flow),  
409  $141.6 \text{ m}^3/\text{s}$  (bankfull),  $283.2 \text{ m}^3/\text{s}$  (intermediate), and  $597.5 \text{ m}^3/\text{s}$  (floodplain filling) (Fig.  
410 5B). For example, the values reported for the bankfull wetted area exclude the areas



**C) Morphological Units**



411 that are wetted at base flow discharge. This analysis explores how vertical topographic  
412 change occurs at different positions within the river valley (e.g., how is the channel  
413 responding in contrast to different floodplain areas?) and provides estimates of valley-  
414 wide incision rates, which were previously reported by Adler (1980) and Pasternack and  
415 Wyrick (2016) discussed in section 2.3.

### 416 3.3.3. *Morphological-unit scale*

417 Sediment budgets presented at the morphological-unit scale were developed using the  
418 MUs discussed in section 2.2 and are expanded upon in the supplementary materials.  
419 The sediment budgets calculated for the MUs provide a summary of the topographic  
420 change that occurred to the former 2006/2008 landforms within LYR. This provides an  
421 understanding of the sources and sinks of sediment movement; however, it does not  
422 represent the net gain or loss of a morphological unit. For example, as a river channel  
423 migrates or avulses, new in-channel MUs are created in areas that were previously  
424 floodplain. Therefore, even though 2006/2008 pools may show net fill, this does not  
425 necessarily indicate that pool habitats have filled in or decreased overall. Instead, the  
426 former location of a pool filled, whereas a new pool may be created in an area that was  
427 previously floodplain. Further analysis would be needed to delineate the new 2014 MUs  
428 to assess net impacts to the MUs and is beyond the scope of this study.

## 429 **4. Results**

### 430 *4.1. DEM Uncertainty*

431 The *SEs* calculated by land cover type for the 2006/2008 to 2014 TCD were 0.039 m for  
432 bare ground and 0.074 m for water. A value of 0.30 m was chosen for vegetated

433 ground, which comes from a review of the relevant literature for vertical error estimates  
434 for LiDAR in leaf-on forested settings (Reutebuch et al., 2003; Hodgson and Bresnahan,  
435 2004; Gould et al., 2013; Tinkham et al., 2013; Edson and Wing, 2015). Figure 4 shows  
436 how uncertainty was influenced by land cover, point density, and topographic variability.  
437 In bare ground areas, point densities were high and survey errors were low, resulting in  
438 small raster elevation uncertainties, even in topographically complex areas. In  
439 vegetated areas, point densities were low and survey errors were high, yielding larger  
440 uncertainties.

#### 441 *4.2. River-Segment Scale*

442 For the flow regime in time period 2, erosion was the dominant process in LYR. The  
443 TCD analysis estimates 103,000 m<sup>3</sup>/y of erosion and 80,800 m<sup>3</sup>/y of deposition within  
444 the LYR valley (i.e., in-channel and overbank areas), a net change of 22,200 m<sup>3</sup>/y of  
445 erosion. This net change represents the average annual volume of sediment exported  
446 to the Feather River.

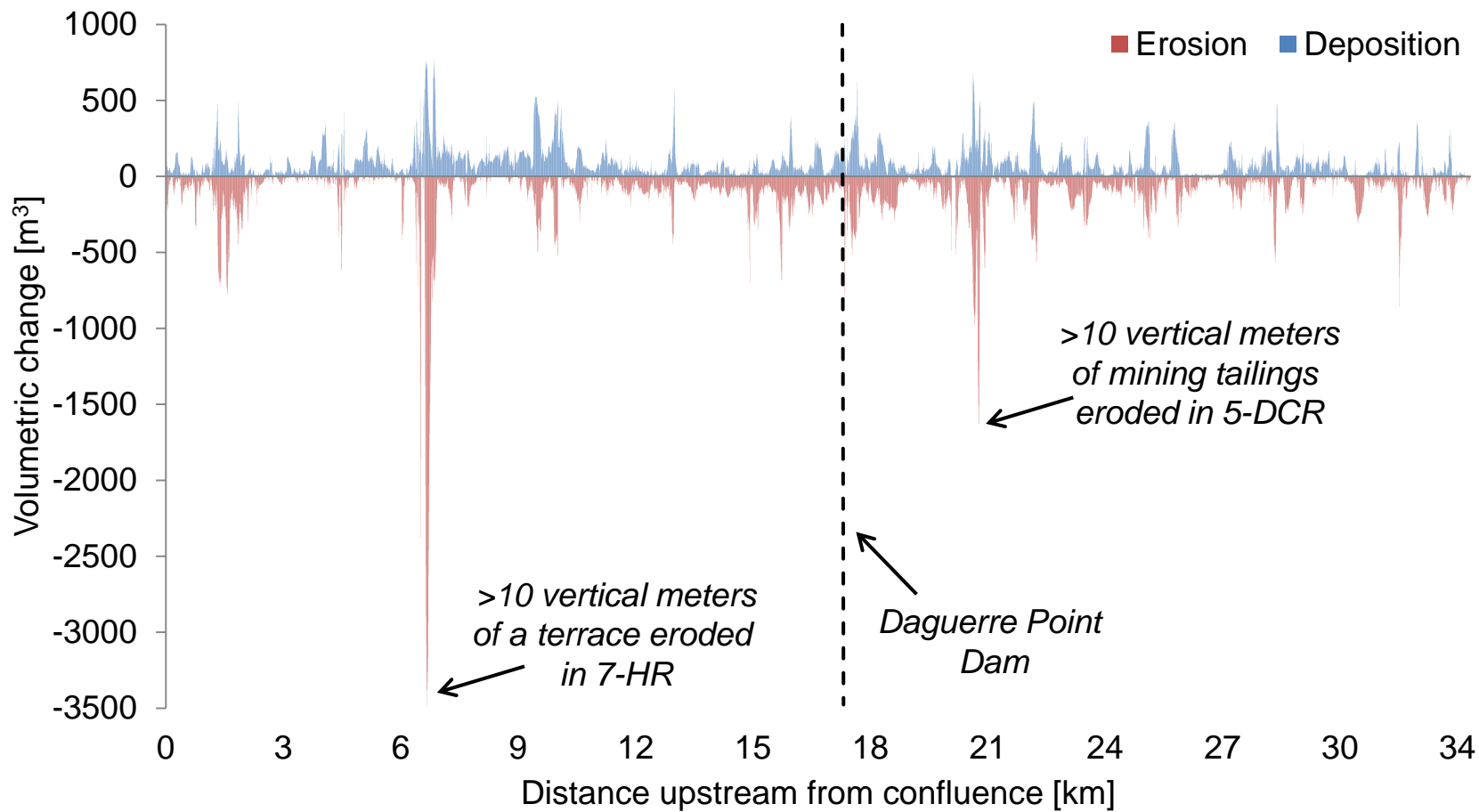
447 A comparison of net topographic change between in-channel vs. overbank areas  
448 revealed that, during time period 2, LYR was strongly depositional within the 2006/2008  
449 bankfull channel, with a net topographic change of 20,300 m<sup>3</sup>/y of deposition, and  
450 strongly erosional in the overbank region, with a net topographic change of 41,600 m<sup>3</sup>/y  
451 of erosion. The large bank collapse in 7-HR accounts for 13,700 m<sup>3</sup>/y of erosion, which  
452 is 13.3% of the total erosion within the LYR valley and 33.9% of the net erosion in the  
453 overbank areas.

454 The longitudinal series of deposition and erosion volumes for time period 2 show that  
455 the locations of peak values of deposition and erosion are moderately correlated with  
456 one another (Fig. 6). The Pearson's product-moment correlation of erosion and  
457 deposition volumes was 0.47 ( $p < 0.001$ ). The longitudinal series of deposition and  
458 erosion by area (Fig. 7) shows significant variation, with erosion being the dominant  
459 process by area within 6-DPDR, deposition being the dominant process by area in 7-  
460 HR, and more balance across the other geomorphic reaches. Averaged across the  
461 entire floodway, 22% of the area experienced erosion, 25% experienced deposition, and  
462 53% indicated no change.

#### 463 4.3. *Geomorphic-Reach Scale*

464 At the geomorphic-reach scale, all reaches were net erosional for time period 2, except  
465 for 7-HR, which had a nearly equal balance of valley-wide erosion and deposition (Fig.  
466 8B). The 5-DCR and 6-DPDR were the most imbalanced reaches with 51% and 77%  
467 more erosion than deposition, respectively. Every geomorphic reach during time period  
468 2 was strongly erosional for overbank areas, but the results for in-channel areas vary by  
469 reach (Table 3). All in-channel areas of reaches were net depositional, except for 3-TBR  
470 ( $230 \text{ m}^3/\text{y}$  of net erosion) and 6-DPDR ( $500 \text{ m}^3/\text{y}$  of net erosion).

471 Rates of vertical change varied significantly by geomorphic reach and by the  
472 incremental wetted areas. The 2008 base-flow channel (wetted by  $28.32 \text{ m}^3/\text{s}$ )  
473 aggraded in all geomorphic reaches, though 3-TBR was nearly neutral with just 0.86  
474 mm/y of aggradation (Table 4). The wetted areas above the base-flow channel were all  
475 erosional, with incision across all geomorphic reaches. Net volumetric changes



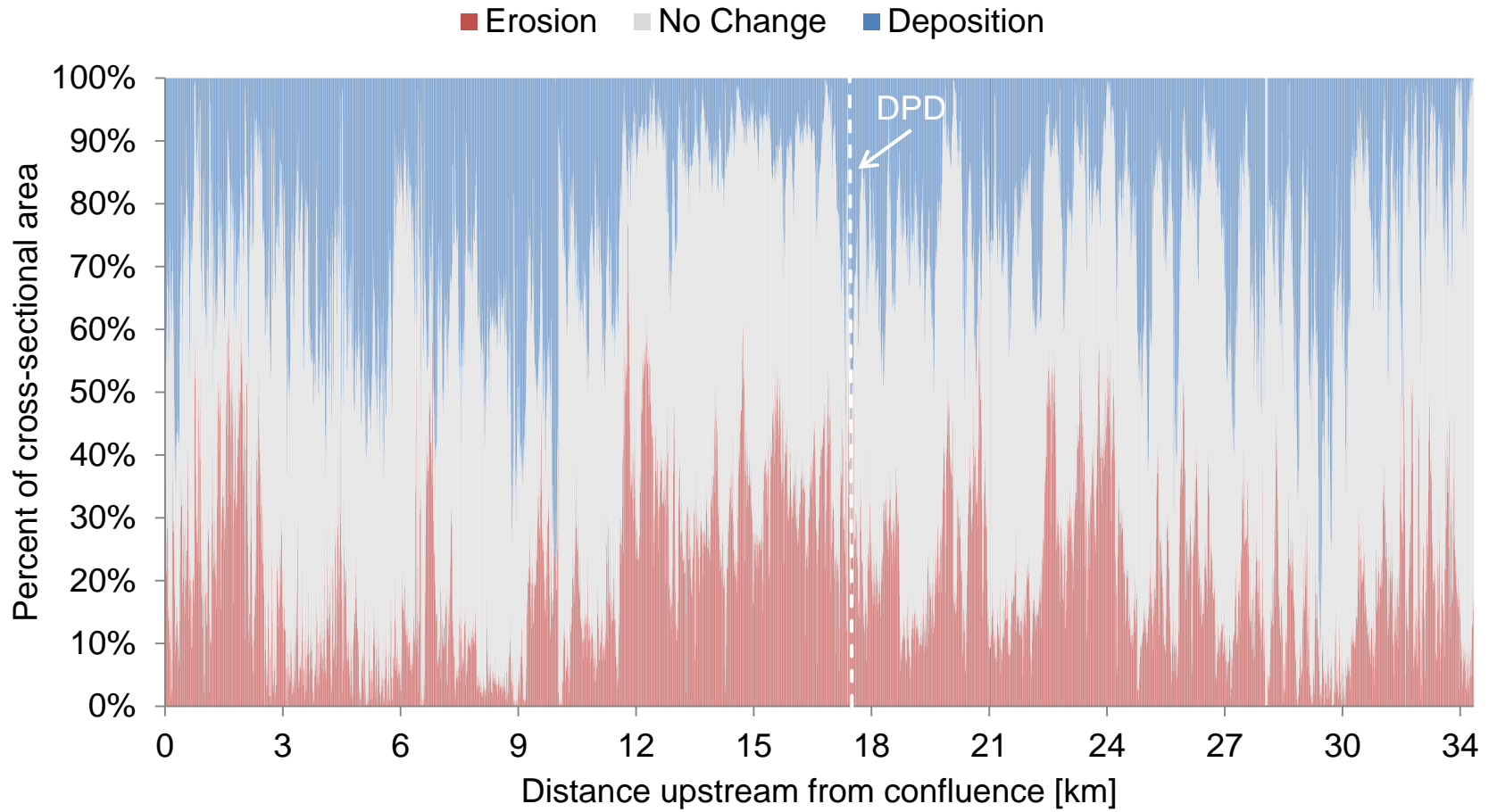




Table 3

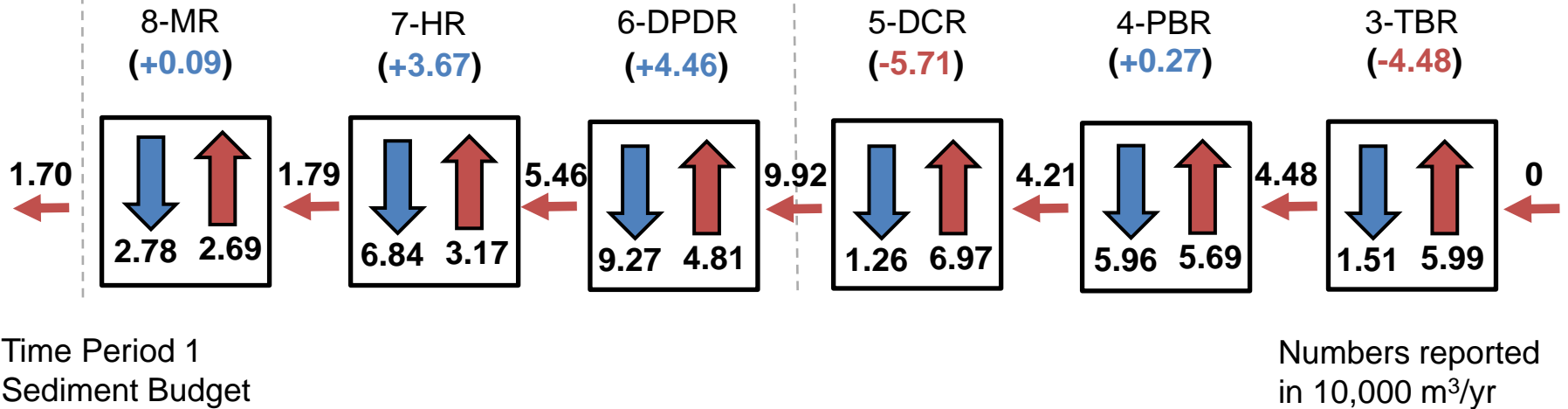
Volumetric change per year [1000 m<sup>3</sup>/y] as separated by in-channel (i.e., bankfull) vs. overbank areas; the numbers reported are the net results within each area (the in-channel vs. overbank analysis was not possible for time period 1; time period 2 is shaded)

Reach	Time period 1		Time period 2		
	Net all	Net in-channel	Net in-channel	Net overbank	Net all
3-TBR	-45.0		-0.2	-1.8	-2.0
4-PBR	2.7		0.3	-4.7	-4.4
5-DCR	-57.0		0.5	-6.4	-5.9
6-DPDR	45.0		-0.5	-6.4	-6.9
7-HR	37.0		17.0	-17.0	-0.3
8-MR	0.9		2.4	-5.1	-2.7
All LYR	-17.0		20.0	-42.0	-22.0

A)

*Feather River  
Confluence*

*Daguerre Point Dam*



B)

*Feather River  
Confluence*

*Daguerre Point Dam*

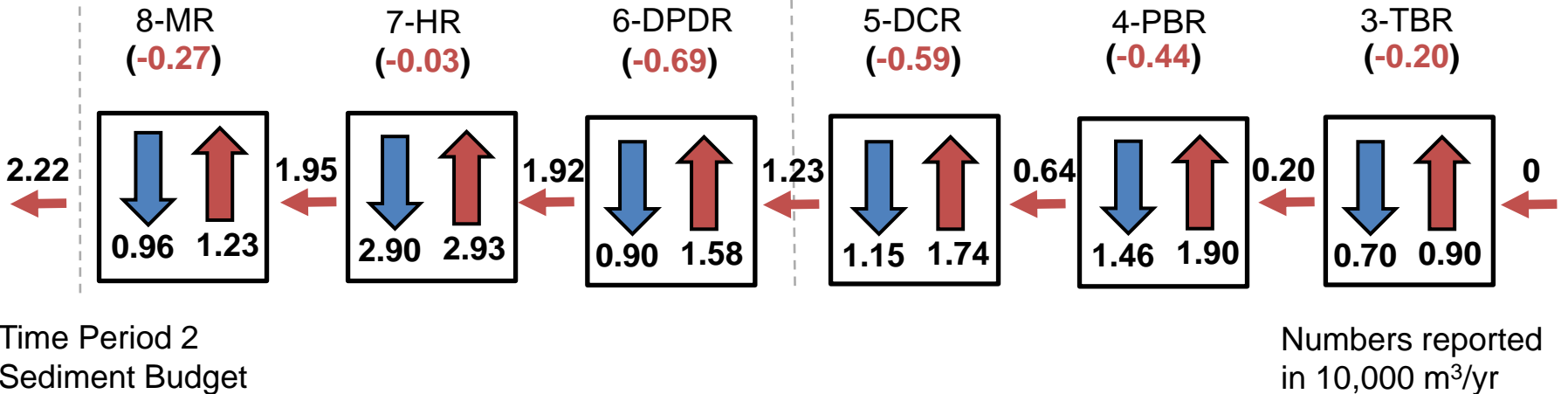


Table 4

Average vertical change per year [mm/y] as analyzed by the incrementally added wetted area for each increase in discharge; the time period averages represent the average vertical change across the entire wetted area of the floodway-filling discharge of 597.5 m<sup>3</sup>/s (time period 2 is shaded)

Area	8-MR	7-HR	6-DPDR	5-DCR	4-PBR	3-TBR	All LYR
Time period 1 Avg.	1.0	16.0	19.0	-59.0	1.3	-45.0	-2.5
Base flow	19.0	45.0	6.7	13.0	3.6	0.9	16.0
Bankfull	-29.0	-5.6	-12.0	-15.0	-5.2	-3.8	-9.5
Intermediate	-17.0	-5.9	-3.1	-1.0	-1.4	-2.5	-3.9
Floodway	-25.0	-5.1	-4.6	-7.9	-3.8	-4.1	-6.0
Time period 2 Avg.	-2.3	9.7	-3.1	-1.5	-1.5	-1.5	-1.5

476 averaged across the 2008 floodway (wetted by 597.5 m<sup>3</sup>/s) show minor incision of 1.5-  
477 3.1 mm/y for all but 7-HR, which aggraded at 9.7 mm/y during the study period.

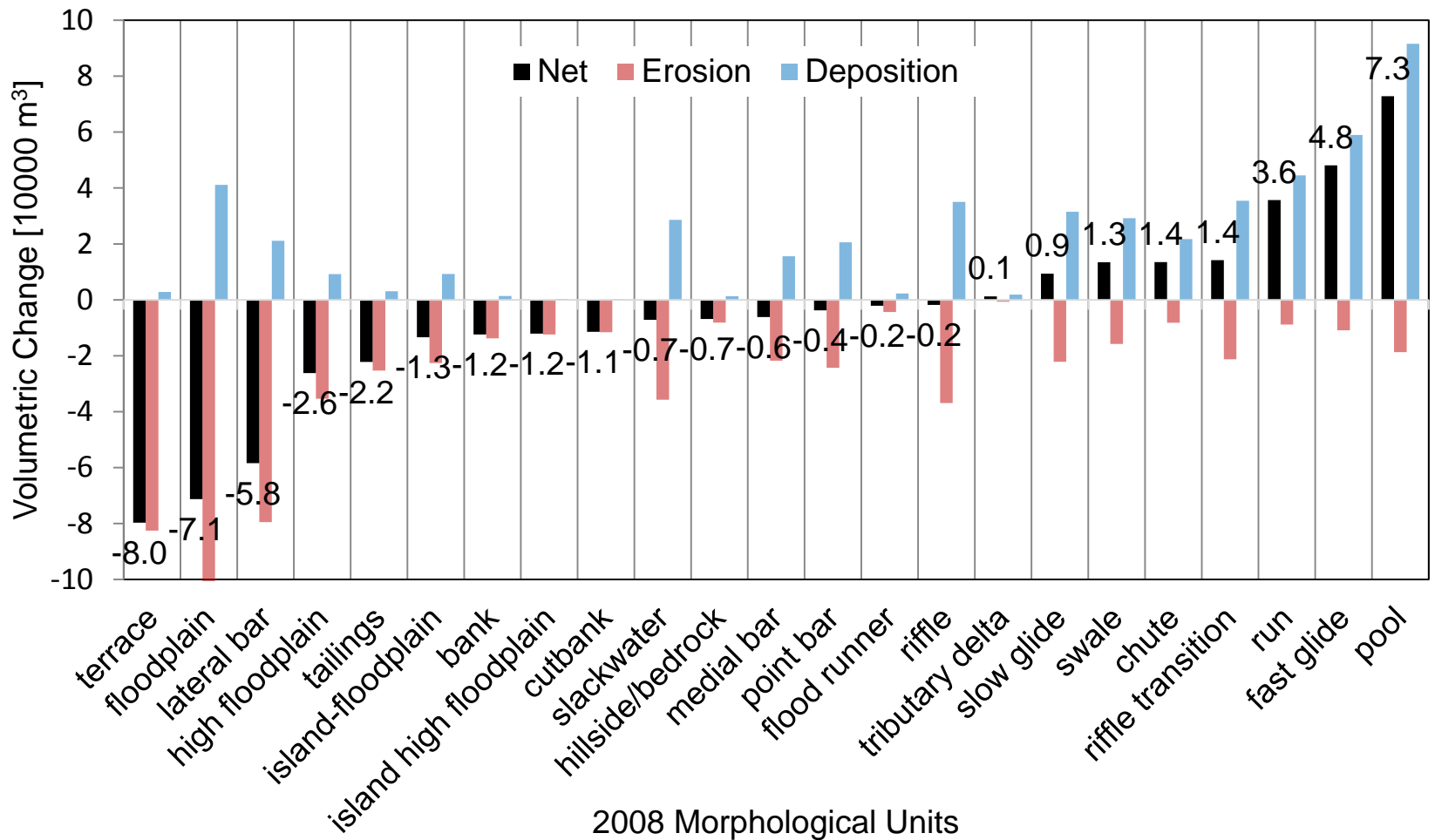
#### 478 4.4. Morphological-Unit Scale

479 Topographic change results during time period 2 for the 2008 MUs are consistent with  
480 observations at the segment and reach scale. Most in-channel base-flow MUs (e.g.,  
481 runs, fast glides, and pools) were strongly depositional; whereas overbank MUs (e.g.,  
482 terraces, floodplains, and high floodplains) were strongly erosional (Fig. 9). However,  
483 several in-channel bank units were net erosional, including lateral bars (58,000 m<sup>3</sup>),  
484 banks (12,000 m<sup>3</sup>), cutbanks (11,000 m<sup>3</sup>), medial bars (6000 m<sup>3</sup>), and point bars (4000  
485 m<sup>3</sup>). Two in-channel bed MUs were net erosional: slackwaters (7000 m<sup>3</sup>) and riffles  
486 (2000 m<sup>3</sup>). Though within those results, slackwaters, medial bars, point bars, and riffles  
487 had similar magnitudes of erosion and deposition.

### 488 5. Discussion

#### 489 5.1. Segment-scale sediment budget differences

490 The TCD analysis for time period 2 (2006/2008 to 2014) yielded different magnitudes,  
491 patterns, and rates of erosion and deposition compared to the TCD reported for time  
492 period 1 (1999 to 2006/2008) by Carley et al. (2012). The longitudinal profiles of erosion  
493 and deposition show that the locations of each process are more balanced in time  
494 period 2 than in time period 1. In time period 2, locations of erosion and deposition were  
495 positively correlated with each other. This is typical for patterns of lateral migration,  
496 where erosion and deposition occur simultaneously in the same cross section (i.e.,  
497 erosion at the cutbank and deposition on the point bar). Visual inspection of the TCD



498 raster and local knowledge of the river suggest that eroded material moves a short  
499 distance downstream before being deposited. Locations of intense scour were often  
500 followed by locations of intense deposition. Thus, in time period 2, sediment was  
501 laterally redistributed within LYR with all reaches net erosional, whereas in time period  
502 1, higher flow events caused more vertically dominated processes (e.g., avulsion,  
503 overbank scour, and downcutting) that yielded either net erosion or deposition by reach.

504 Despite the higher flood peaks and longer durations of flood flows during time period 1,  
505 the magnitude of net annual export of sediment to the Feather River was similar for the  
506 two time periods, with 25% more sediment export during the drought period (time period  
507 2) than during the flood period (17,000 m<sup>3</sup>/y in time period 1 vs. 21,300 m<sup>3</sup>/y in time  
508 period 2). Time period 1 had 20% more days of bankfull flow than time period 2, but it  
509 had 25% less net export.

## 510 *5.2. Reach-scale sediment budget differences*

511 At the reach-scale, erosion and deposition were more imbalanced between the  
512 geomorphic reaches for time period 1 than time period 2, with 3-TBR and 5-DCR being  
513 strongly erosional and 6-DPDR and 7-HR being strongly depositional. To thoroughly  
514 understand this would take an evaluation of the hydraulics during the events in these  
515 time periods, but these topographic change patterns correspond well with valley-scale  
516 morphological differences. Specifically, we believe that differences in topographic  
517 steering between modest vs. large floods are a key contributor to the observed  
518 differences in topographic change. Abu-Aly et al. (2014) conducted 2D hydrodynamic  
519 flood modeling for LYR (accounting for spatially distributed vegetation roughness), and

520 some of the wetted extents they obtained for relevant flows are provided in Figure 10 to  
521 help illustrate the hydraulic mechanism that explains the reach-scale erosion and  
522 sedimentation results found in this study.

523 The two strongly aggradational reaches during time period 1, 6-DPDR and 7-HR, are  
524 wider and upstream of 8-MR, which is leveed (Fig. 10C, see zone of constriction). The  
525 engineered levees constrict the river corridor in 8-MR by nearly two-thirds (average  
526 floodway width decreases from 313 m in 6-DPDR to 116 m in 8-MR). During flood  
527 events, this lateral topographic constriction causes water stages to rise higher than they  
528 would without the levees, a phenomenon called *levee surcharge* by Heine and Pinter  
529 (2012). The levee surcharge would cause a backwater effect upstream of the levees,  
530 decrease the water surface slope, and reduce transport capacity. Furthermore, at high  
531 discharge, a large secondary channel becomes active within 6-DPDR, creating the  
532 widest active floodway within LYR (Fig. 10c, see zone of expansion) and reducing the  
533 rate of increase in velocity relative to the other geomorphic reaches (Strom et al., 2016).  
534 Together, the flood-stage backwater effect of the levees and the wide floodway in 6-  
535 DPDR and 7-HR yield lower flood velocities in these reaches and appear to promote  
536 deposition of sediment. Although not part of this study, reach-scale at-a-station  
537 hydraulic geometry relations were developed for LYR using 2D hydrodynamic modeling  
538 for both in-channel (Gonzalez and Pasternack, 2015) and overbank hydraulics, and the  
539 results show reach-scale velocity reversals. First, the mean velocity in 8-MR exceeds  
540 that in 7-HR at 212.4 m<sup>3</sup>/s, and then it exceeds that in 6-DPDR at 283.2 m<sup>3</sup>/s. Above  
541 this flow, 8-MR has a significantly higher average velocity than 6-DPDR and 7-HR. In  
542 fact, the levees in 8-MR were created to do this by design for the purpose of slowing





543 down the export of LYR hydraulic mining debris to the Feather and Sacramento rivers  
544 (Adler, 1980).

545 In contrast to the dynamics in 6-DPDR, 7-HR, and 8-MR, the reaches that eroded  
546 during time period 1, 3-TBR and 5-DCR have the lowest entrenchment ratio (i.e., most  
547 entrenched), meaning that flood flows in these reaches are constrained within a  
548 narrower floodway. As a result, they have higher reach-average velocities and  
549 preferentially scour during floods. Overall, the dominant mechanism at work during time  
550 period 1 appears to be driven by valley-scale morphological differences within each  
551 reach as large floods activate these hydraulic controls.

552 Given the smaller floods and a shorter overall duration of floodplain-filling flows during  
553 time period 2, the hydraulics would have been less impacted by the valley-scale  
554 differences between the geomorphic reaches. Furthermore, even though time period 1  
555 had a significantly longer duration of large flooding, the duration above bankfull was  
556 only 20% more in time period 1 than in time period 2. This can explain why lateral  
557 channel migration processes dominated in time period 2 and why the erosion and  
558 deposition patterns were more balanced between the geomorphic reaches. Specifically,  
559 at flows at and just above bankfull flow, erosive forces are focused in the channel and  
560 on the banks; whereas, after the water spills onto the floodplain, channel velocity can  
561 decline, and the patches of peak velocity shift onto the floodplain (Abu-Aly et al., 2014).  
562 Thus, apparently the hydraulics in time period 2 were substantially different than in time  
563 period 1, and as a result, the topographic change processes and net export were  
564 different. This finding provides an important lesson, that more flow does not always

565 mean more erosion (as assumed by specific stream power analysis and commonly  
566 applied in geomorphic studies), because the processes of topographic change are  
567 stage dependent.

568 In addition to the differences in flood magnitudes and their associated topographic  
569 change processes, topographic changes during time period 2 may be heavily influenced  
570 by what happened in time period 1 and the effect that prior floods had on sediment  
571 supply and accommodation space. In time period 1, 6-DPDR was strongly depositional,  
572 but in time period 2 it was net erosional. Furthermore, for time period 2, 6-DPDR is one  
573 of only two reaches where net erosion occurred within the 2008 bankfull channel, and it  
574 is the only reach where erosion was the dominant process by area. During time period  
575 2, the channel and floodplain may have been responding to the previous wave of  
576 sediment deposition. This highlights the need for repeat topographic change analyses  
577 over many time periods in order to understand the role of antecedent conditions prior to  
578 interpreting fluvial topographic change results. Along these lines, a large ~ 34-year flood  
579 occurred in 1997, and how this changed the distribution of sediment in the river  
580 segment is not known. However, it certainly would have established conditions that  
581 would influence the results of the 1999 to 2006/2008 TCD analysis.

### 582 *5.3. Incision rates*

583 Average vertical change rates for time period 1 and time period 2 were similar with 2.4  
584 mm/y of incision for time period 1 and 1.5-3.1 mm/y of incision for time period 2.  
585 Although the average values were similar, results at the reach scale were significantly

586 different, with reaches during time period 1 aggrading by as much as 19 mm/y (6-  
587 DPDR) and incising by as much as 69 mm/y (5-DCR).

588 The analysis of vertical change by inundation threshold for time period 2 showed that,  
589 while each geomorphic reach was net erosional, incision rates were low (~1.5 to 3.1  
590 mm/y) for the floodway corridor. Furthermore, the base-flow channel aggraded across  
591 all geomorphic reaches, whereas the incrementally added wetted areas above base-  
592 flow discharge eroded for all geomorphic reaches across all higher discharges. This  
593 result further confirms that even as LYR continues to slowly incise, the river is staying  
594 well connected to its current floodplain through lateral migration.

595 The vertical change results for 7-HR may appear to be contradictory as net erosion was  
596 calculated for the reach ( $310 \text{ m}^3/\text{y}$ ), while 9.1 mm/y of aggradation occurred during time  
597 period 2 for the 2008 floodway area. This is because of a large bank collapse that  
598 eroded a terrace outside of the 2008 floodway (Fig. 5B). Thus, a large amount of  
599 erosion occurred outside of the 2008 floodway as the river migrated laterally and  
600 deposited material within the former channel. Events like this occurred in several other  
601 locations, such as in 5-DCR, where the river eroded into tailings of hydraulic mining  
602 debris.

#### 603 *5.4. Sources and sinks of sediment*

604 The bulk of the sediment eroded within LYR between 2006/2008 and 2014 came from  
605 three MUs: floodplain ( $112,000 \text{ m}^3$ ), terraces ( $82,600 \text{ m}^3$ ), and in-channel lateral bars  
606 ( $79,500 \text{ m}^3$ ). A smaller but significant amount of sediment came from riffles ( $36,700 \text{ m}^3$ ),

607 slackwater (35,700 m<sup>3</sup>), high floodplain (35,400 m<sup>3</sup>), and tailing (25,300 m<sup>3</sup>) MUs. The  
608 sediment sourced from terraces, high floodplains, and tailings represent landforms that  
609 are outside of the 2008 floodway but are being reincorporated into the active river valley  
610 as the river migrates. This analysis shows that these landforms higher on the landscape  
611 can be a significant component of the sediment dynamics within LYR, with 26% of the  
612 sediment being sourced outside of the floodway during time period 2. At the same time,  
613 the results continue to support the conclusion that the floodplain and channel are the  
614 dominant sources of sediment, even though tailings and terraces produce more visible  
615 patterns of topographic change.

#### 616 *5.5. Contrast with other regulated gravel/cobble rivers*

617 Typically, rivers that incise abandon their floodplains. This study presents an uncommon  
618 case where the river is incising and evacuating sediment while maintaining channel-  
619 floodplain connectivity. In some places along the river, such as in 7-HR and 5-DCR, the  
620 extent of the floodplain expanded as the river migrated laterally into former terraces and  
621 mining tailings. Historically, LYR did incise and abandon its floodplain in some places,  
622 as evident by the presence of intermittent terraces; but from 1999 to 2014, the flood  
623 flows and sediment regime remain well connected to the existing floodplain. In time  
624 period 1, the sediment connectivity was largely through overbank scour and avulsion;  
625 whereas, in time period 2, the connectivity was largely through lateral channel  
626 migration.

627 The LYR, despite being a regulated river, has been able to resist significant,  
628 interdecadal bed armoring because of an abundance of alluvium and a heterogeneous

629 flow regime that diversifies its sediment transport mechanisms (Parker et al., 2003).  
630 Both time periods found that sediment was preferentially scoured from the floodplain  
631 and deposited within the former channel. This redistribution of sediment prevents the  
632 persistence of an armored layer. The abundant alluvial fill within the floodway consists  
633 of a variety of sediment sizes, with about 10-20% fine gravel [2-32 mm], 20-40% small  
634 cobble [32-90 mm], 20-30% cobble [90-128 mm], and 10-20% large cobble [128-256  
635 mm] (Jackson et al., 2013). Furthermore, as LYR migrates laterally and erodes terraces  
636 and mining tailings, new material is added to the floodway. Terrace sediments are  
637 poorly sorted and consist of sands, gravels, and cobbles (Jackson et al., 2013).

## 638 **6. Conclusions**

639 This study provided insights into how sediment budgets and incision/aggradation rates  
640 differ within LYR during two contrasting decadal flow regimes. During the large flood  
641 events of time period 1, sediment within LYR was displaced longitudinally, with reaches  
642 above DPD eroding sediment and reaches below DPD accumulating sediment. This  
643 finding corresponds with the valley-scale morphological features that are activated at  
644 high river stages, such as the levees in 8-MR and 7-HR, the secondary bypass channel  
645 in 6-DPDR, and the narrow valley walls in 3-TBR. These valley-scale morphological  
646 features drive the hydraulics and sediment movement within LYR at high flow. During  
647 the modest floods of time period 2, from 2006/2008 to 2014, the majority of the  
648 sediment was laterally redistributed with erosion outside of the base-flow channel (e.g.,  
649 floodplain, terrace, and lateral bar MUs) and deposition within the former base-flow  
650 channel (e.g., pools, runs, and fast glide MUs) as the river channel migrates. The  
651 results from 6-DPDR during time period 2 suggest that the previous wave of sediment

652 deposition in that reach influenced the type of topographic change processes that  
653 occurred. This finding highlights the need for repeat topographic change studies to  
654 understand antecedent conditions and interpret the results. Modern day incision rates  
655 averaged across the entire floodway were ~2.5 mm/y for time period 1 and 1.5 mm/y for  
656 time period 2. These incision rates are significantly slower than the 6.4 cm/y previously  
657 reported by Adler (1980) from 1912 to 1979. Overall, this paper presents the  
658 complexities of a gravel/cobble river that experiences vastly different morphodynamics  
659 based on the interaction of a dynamic flood regime and multiple layers of topographic  
660 heterogeneity.

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882

883

884 Figure Captions

885

886 Fig. 1. Site map of the lower Yuba River (LYR) in northern California. The grey area in  
887 the bottom map depicts the estimated inundated area for a flow of 1195 m<sup>3</sup>/s.  
888 LYR begins at the downstream end of Englebright Dam and continues until its  
889 confluence with the Feather River.

890 Fig. 2. Longitudinal profile of the 2014 bed elevation along the thalweg for the lower  
891 Yuba River with geomorphic reaches delineated. Elevations were derived from  
892 bathymetric LiDAR and multibeam sonar data with respect to NAVD88.

893 Fig. 3. Instantaneous flow hydrograph for time period 1 (1999-2006/2008) and time  
894 period 2 (2006/2008-2014) for LYR at the Marysville USGS gage (11421000).  
895 Time period 2 is shaded.

896 Fig. 4. Example outputs from the DEM uncertainty and TCD analysis using the 2014  
897 data. (A) 2014 aerial photo. (B) 2014 raster DEM. (C) 2014 land cover raster. (D)  
898 2014 point density raster. (E) 2014 SD\_Z raster. (F) 2014 SEM raster. (G) 2008-  
899 2014 LOD95 raster. (H) Raw topographic change results. (I) Statistically  
900 significant topographic change results.

901 Fig. 5. (A) Example of TCD results in the Hallwood Reach, where lateral migration  
902 caused the collapse of a forested terrace and orchard with >10 m of vertical  
903 scour. Deposition primarily occurred within the 2008 base-flow channel. (B) Map  
904 of wetted areas produced by a 2D hydrodynamic model. The location of scour  
905 outside the floodway demonstrates how the river expanded its active floodway in

906 this stretch by eroding a terrace. (C) Map of the 2008 MUs delineated by Wyrick  
907 and Pasternack (2012, 2014).

908 Fig. 6. Longitudinal series of deposition and erosion volumes in the lower Yuba River  
909 valley for the 2006/2008 to 2014 time period.

910 Fig. 7. Longitudinal series of deposition and erosion as a percentage of the floodway  
911 area for the 2006/2008 to 2014 time period. On average, 22% of the floodway  
912 experienced erosion, 25% deposition, and 53% did not change, though  
913 significant variation exists among the geomorphic reaches.

914 Fig. 8. Sediment budget by geomorphic reach for time period 1 (A) and time period 2  
915 (B). The numbers are reported as volumetric rates in  $10,000 \text{ m}^3/\text{y}$ . Each  
916 geomorphic reach is presented as a control volume with their inputs and exports  
917 of sediment shown by the horizontal arrows. The net change in sediment storage  
918 for each reach is shown in parentheses below the reach label, where positive  
919 values indicate net deposition and negative values indicate net erosion. Within  
920 each reach, the downward arrows indicate the gross rates of deposition and the  
921 upward arrows indicate the gross rates of erosion within each reach. The value  
922 left of the Feather River confluence marker is the average annual volume of  
923 sediment exported out of the LYR valley to the Feather River.

924 Fig. 9. Volumetric change within the 2006/2008 morphological unit delineations during  
925 time period 2. The black bars and data labels show the net volumetric change in  
926  $10,000 \text{ m}^3$ .

927 Fig. 10. The panels show wetted extents from Abu-Aly et al. (2014) for three different  
928 discharges. The lines show geomorphic reach breaks with reach labels in panel

929 A. (A) Wetted extent for bankfull discharge ( $141.6 \text{ m}^3/\text{s}$ ). (B) Wetted extent for the  
930 floodway filling discharge ( $597.5 \text{ m}^3/\text{s}$ ), which is approximately the peak daily-  
931 averaged flow for time period 2. Notice that the sinuosity of the base-flow  
932 channel is still apparent in the wetted extent for this discharge, indicating that the  
933 bankfull channel is still steering the hydraulics. (C) Wetted extent for  $2390 \text{ m}^3/\text{s}$ ,  
934 which is approximately the peak daily-averaged flow for time period 1. Notice that  
935 the sinuosity of the base-flow channel is no longer visible in the wetted extent for  
936 this discharge. Instead, expansion and contraction of the wetted extent occurs at  
937 multiple scales. An overall zone of expansion begins just below the entrance of  
938 6-DPDR, where the floodway splits in two. Then, an overall contraction occurs at  
939 the downstream end of the figure in 7-HR, where the wetted width more than  
940 halves. In addition, multiple smaller scale expansion and contractions occur  
941 throughout this section of river.

## **Supplementary Materials**

Valley-scale morphology drives differences in fluvial sediment budgets and incision rates during contrasting flow regimes

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### **S1. Introduction**

The supplementary materials provide information on the lower Yuba River's geomorphic reaches (Section S2.1) and morphological units (Section S2.2), the topographic data collection efforts for the 2006/2008 topographic map (Section S4.1) and the 2014 topographic map (Section S4.2), the DEM uncertainty method (Section S5.1), and cross-section boxes used to aggregate results (Section S5.1). In addition, overview maps of the 2006/2008 – 2014 topographic change results are presented in a separate PDF.

## **S2. Study site – Lower Yuba River Supplements**

### *S2.1. Geomorphic Reaches*

The six alluvial geomorphic reaches of LYR discussed in this study are Timbuctoo Bend Reach (3-TBR), Parks Bar Reach (4-PBR), Deer Creek Reach (5-DCR), Daguerre Point Dam Reach (6-DPDR), Hallwood Reach (7-HR), and Marysville Reach (8-MR). 3-TBR begins 34.0 river kilometers (RKm) upstream from the confluence with the Feather River. 3-TBR marks the emergence of a gravel/cobble floodplain as the river transitions from a narrow bedrock canyon to a wider bedrock valley with some river meandering. In 4-PBR (21.3-28.3 RKm), the river valley width nearly doubles as the river enters a broad alluvial valley with few locations of bedrock controls. The Dry Creek Reach (5-DCR) begins where Dry Creek, an ungaged tributary, enters LYR at RKm 21.3. The bed slope significantly decreases in 5-DCR (from 0.0019 in 4-PBR to 0.0014 in 5-DCR), which is associated with the backwater effects from Daguerre Point Dam (DPD). 6-DPDR begins downstream of DPD (RKm 17.8). It has the widest active valley with a secondary channel (i.e., flood runner) that is occupied with water when flows reach above 330 m<sup>3</sup>/s. 7-HR marks another slope break (from 0.0018 in 6-DPDR to 0.0013 in 7-HR), and the river corridor begins to be laterally constricted from levees. The last geomorphic reach is the Marysville Reach (8-MR), which is defined by a very low bed slope (0.0005) and backwater effects from the Feather River. It is laterally constricted and heavily channelized from levees that protect the City of Marysville. A summary of geomorphic reach statistics is provided in Table S1.



**Table S1.** Geomorphic reach metrics for the alluvial lower Yuba River

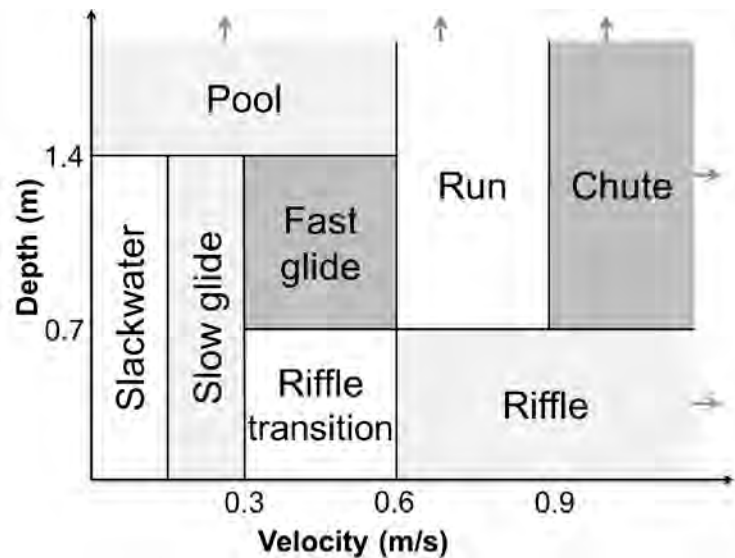
Reach	Mean Bankfull Width [m]	Mean Floodway Width [m]	Entrenchment Ratio	Width:Depth Ratio	Sinuosity	Slope	Substrate $d_{50}$ [mm]
3-TBR	84.4	134.4	2.12	82.4	1.10	0.0020	164
4-PBR	96.3	206.7	2.93	107.9	1.14	0.0019	117
5-DCR	130.1	263.7	2.45	122.3	1.06	0.0014	87
6-DPDR	119.8	313.3	3.54	85.4	1.13	0.0018	87
7-HR	102.1	210.9	2.61	70.8	1.08	0.0013	61
8-MR	70.4	115.5	2.61	23.1	1.07	0.0005	40

## S2.2. Morphological Units

Morphological Units (MUs) are river landforms mapped at the scale of ~0.1-10 channel widths and are considered the basic building blocks of fluvial morphology (Grant et al., 1990; Wadeson, 1994; Brierley and Fryirs, 2000; Wheaton et al., 2015). Names used in the literature for these discernable units include “physical biotope” (Newson and Newson, 2000), “channel geomorphic unit” (Hawkins et al., 1993), “channel unit” (Grant et al., 1990), and “morphological unit” (Wadeson, 1994).

The MUs delineated for LYR were developed based on the 2006/2008 topographic map and are categorized into four categories: in-channel bed MUs, in-channel bar MUs, floodway MUs, and valley MUs. These categories are segregated by inundation thresholds determined by a 2-dimensional (2D) hydrodynamic model (SRH-2D) with in-channel bed MUs delineated at baseflow discharge ( $24.92 \text{ m}^3/\text{s}$  above DPD and  $15.01 \text{ m}^3/\text{s}$  below DPD), in-channel bar MUs delineated within the incrementally added area at bankfull discharge ( $141.6 \text{ m}^3/\text{s}$ ), floodway MUs delineated within the incrementally added area at the floodway filling discharge ( $597.5 \text{ m}^3/\text{s}$ ), and valley MUs delineated in areas outside of the floodway. There are eight in-channel bed MUs. The method for delineating these eight MUs was presented in (Wyrick et al., 2014), Fig. S1 shows the

combinations of depth and velocity that were used to delineate each MU, and Table S2 provides a qualitative description of each of the in-channel bed MUs. The other categorical MUs were delineated manually in ArcGIS on an expert-basis using information obtained from field surveys, topographic indicators (e.g., slope breaks), and topographic change maps from 1999-2008. Details on this mapping effort was provided in (Wyrick and Pasternack, 2012), Table S3-5 provide qualitative descriptions of each of the MUs, and Fig. S2 provides an example map of the MUs for 4-PBR.



**Fig. S1.** Combinations of depth and velocity used to delineate in-channel bed morphological units for the lower Yuba River using the outputs of a 2-dimensional hydrodynamic model at baseflow discharge. Reproduced from Wyrick et al. (2014).

**Table S2.** Qualitative descriptions of in-channel bed morphological units mapped in LYR. Reproduced from Wyrick and Pasternack (2012).

Unit Name	Description
Chute	Area of high velocity, steep water surface slope, and moderate to high depth located in the channel thalweg. Chutes are often located in a convergent constriction downstream of a riffle as it transitions into a run, forced pool, pool, or glide.
Fast Glide	Area of moderate velocity and depth and low water surface slope. Commonly occur along periphery of channel and flanking pools. Also exist in straight sections of low bed slope.
Pool / Forced Pool	Pools are areas of high depth and low velocity, and low water surface slope. The distinction between 'forced pool' and 'pool' cannot be made automatically within GIS. A 'forced pool' is one that is typically along the periphery of the channel and is "over-deepened" from local convective acceleration and scour during floods that is associated with static structures such as wood, boulders, and mostly bedrock outcrops. A 'pool' is not formed by a forcing obstruction.
Riffle	Area with shallow depths, moderate to high velocities, rough water surface texture, and steep water surface slope. Riffles are associated with the crest and backslope of a transverse bar.
Riffle Transition	Typically a transitional area between an upstream morphological unit into a riffle, or from a riffle into a downstream morphological unit. Water depth is relatively low. Velocity is low, but increases downstream due to convective acceleration toward the shallow riffle crest that is caused by lateral and vertical flow convergence. The upstream limit is at the approximate location where there is a transition from a divergent to convergent flow pattern. The downstream limit is at the slope break of the channel bed termed the riffle crest.
Run	Area with a moderate velocity, high depths, and moderate water surface slope. Runs typically occur in straight sections that exhibit a moderate water surface texture and tend not to be located over transverse bars.
Slow Glide	Area of low velocity and low to moderate depths and low water surface slope. May be located near water's edge as a morphological unit along the channel thalweg transitions laterally towards the stream margins.
Slackwater	Shallow, low-velocity regions of the stream that are typically located in adjacent embayments, side channels, or along channel margins. Velocities are near stagnant during baseflow conditions and rise slower than other bed units' as stage increases.

**Table S3.** Qualitative descriptions of in-channel bar morphological units mapped in LYR. Reproduced from Wyrick and Pasternack (2012).

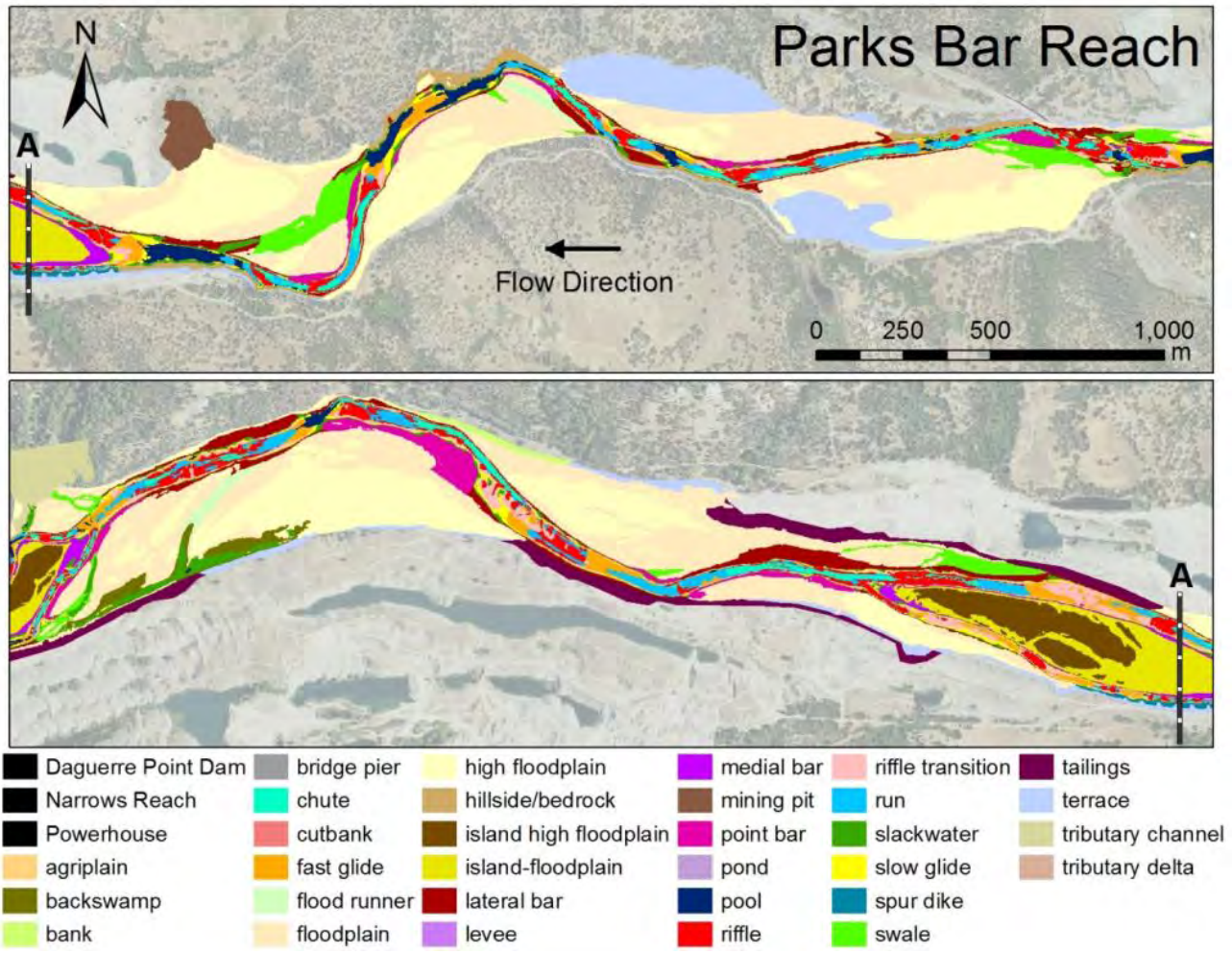
Unit Name	Description
Lateral Bar	Area located at the channel margins at an elevation band between the autumnal low-flow stage and bankfull stage. Lateral bars are orientated parallel to the flow. The feature slopes toward the channel thalweg with an associated increase in both flow depth and velocity when submerged. Sediment size tends to be smaller than in adjacent sections of the channel.
Medial Bar	Area that is separated from the channel banks at low-flow stages at an elevation band between low-flow and bankfull stages. Can be accreting or eroding.
Point Bar	Accreting area located on the inside of a meander bend at an elevation band between the low-flow stage and bankfull stage. Point bars are curved and begin where there is clear evidence of point-bar deposition. The feature slopes toward the channel thalweg with an associated increase in both flow depth and velocity when submerged. Sediment size tends to be smaller than in adjacent sections of the channel.
Swale	A weakly-defined geometric channel or adjacent bench on the floodplain that only conveys flow at stages above low-flow.
Bridge Pier*	Man-made structural supports for road and rail crossings. Typically composed on concrete and steel. Units also exist at stages above Bankfull flow to a lesser extent.

**Table S4.** Qualitative descriptions of floodway morphological units mapped in LYR. Reproduced from Wyrick and Pasternack (2012).

Unit Name	Description
Floodplain	Natural alluvium located at an elevation higher than the bankfull channel and lower than the upper wetted extent of the floodway (defined as 21,100 cfs here).
Flood Runner	Relatively straight floodplain channel with uniform geometry and low depths that conveys a concentrated flow at stages above bankfull.
Island-Floodplain	Natural alluvium on a medial bar located at an elevation higher than the bankfull channel and lower than the upper wetted extent of the floodway (defined as 21,100 cfs here).
Mining Pit	Artificial depression created for mining purposes that is adjacent to the flow channel and continuously wetted. May have an artificial connection to floodway channel that is normal to the flow direction.
Backswamp*	Natural depression within the floodplain whose bed elevation intersects with the groundwater table creating a continuously wetted or swampy area. Typically contains vegetation. Units also exist within Bankfull and Valley boundaries to a lesser extent.
Pond*	Natural depression with a continuously measurable depth located on the floodplain and is not attached to the main channel by a surface opening during the low flow at which the in-channel bed morphological units are mapped. Units also exist within Bankfull boundaries to a lesser extent.
Tributary Channel*	Those sections of perennial tributary streams that are located within the bankfull and higher wetted areas of the main channel. Units also exist within Bankfull and Valley boundaries to a lesser extent.
Spur Dike*	Artificial bank protection composed of very large riprap. Usually located along steep banks to prevent further erosion. Units also exist within Bankfull and Valley boundaries to a lesser extent.

**Table S5.** Qualitative descriptions of morphological units mapped in LYR that are off-channel, but within the active geomorphic valley width. Reproduced from Wyrick and Pasternack (2012).

Unit Name	Description
Terrace	A natural alluvial deposit separated from the floodplain surface by a vertical topographic riser. Terraces are generally abandoned floodplains that have been separated from the channel by vertical incision of lateral migration.
High Floodplain	Natural alluvium located between the terrace riser and the 21,100 cfs wetted area floodplain.
Island-High Floodplain	Natural alluvial deposit on a medial bar located at an elevation higher than the island-floodplain surface.
Levee	Artificially-built flood control berm located parallel to the channel.
Hillside / Bedrock*	Natural colluvium and bedrock at an elevation greater than the valley toe slope break. Units also exist within the Bankfull and Floodway boundaries to a lesser extent.
Bank*	Steep, near-vertical bank that separates bar units from terraces. Gravel/cobble alluvium that line the main channel and not actively experiencing lateral erosion. Units also exist within the Bankfull and Floodway boundaries to a lesser extent.
Cutbank*	Steep, near-vertical bank that separates bar units from terraces. Located on the outside of a meander bend and created by active lateral erosion through local alluvia. Units also exist within the Bankfull and Floodway boundaries to a lesser extent.
Agriplain*	Agriculture field inundated at flows higher than bankfull. These units also exist within the Floodway boundary to a lesser extent.
Tailings*	Steep alluvium artificially piled up adjacent to the channel during historic gold dredging operations. Units also exist within the Bankfull and Floodway boundaries to a lesser extent.
Tributary Delta*	Alluvial fans penetrating the floodplain and main channel at tributary junctions. Units also exist within the Bankfull and Floodway boundaries to a lesser extent.



**Fig. S2.** Morphological unit map within the Parks Bar Reach. Reproduced from Wyrick and Pasternack (2012).

#### S4. Data

##### S4.1. 2006/2008 Topographic Map Supplements

###### S.4.1.1. Data Collection

A 2006/2008 topographic map of the lower Yuba River (LYR) was produced through a phased effort as funding and need permitted. The period between June 2006 and March 2009 was dry with low flows, so it was reasonable to extend mapping over the period to meet project constraints. Data gaps surveyed in November 2009 were shallow backwater, side-channel, and near-bank areas that were unlikely to experience significant change during the modest overbank flows of March and May 2009.

During the dry season of 2006, 3-TBR was mapped using a robotic total station in terrestrial and wadable bathymetric areas. Data was collected in a  $\sim 3 \times 3 \text{ m}^2$  grid in the wetted channel and a  $\sim 6 \times 6 \text{ m}^2$  grid outside of it. For unwadable bathymetry, a professional hydrography firm (Environmental Data Solutions, San Rafael, CA) was contracted to collect bathymetric points along longitudinal and cross-channel lines, meeting the class 1 standard ( $\pm 0.15 \text{ m}$  vertical accuracy). The topographic and bathymetric procedural details used were explained in Sawyer et al. (2010) for a smaller mapping effort done in 3-TBR in 2005. The overall point spacing for 3-TBR is presented in Table S6.

Downstream of 3-TBR, a mix of LiDAR, single-beam sonar (SBS), total station and RTK-GPS surveys were used to map the river corridor. Aero-Metric, Inc. (Seattle, WA) acquired near-infrared (NIR) LiDAR for the river corridor during a period of low flow on 21 September 2008 ( $24.35 \text{ m}^3/\text{s}$  upstream of DPD and  $17.61 \text{ m}^3/\text{s}$  downstream of DPD). Boat-based SBS was acquired by Environmental Data Solutions during 2008 low flow conditions ( $\sim 25.5\text{--}42.5 \text{ m}^3/\text{s}$ ) in August and September. A limited amount of SBS surveying was conducted at higher flows in March and May 2009 ( $\sim 200\text{--}370 \text{ m}^3/\text{s}$ ). Key data gaps were surveyed using total station, RTK GPS, and SBS methods in November 2009. These areas largely consist of shallow backwater, side-channel, and near-bank areas that were unlikely to experience significant change during the higher flows of March and May 2009. Overall point densities are presented in Table S6.

**Table S6.** Density of data for the 2006/2008 topographic map

Geomorphic Reaches	3-TBR (2006)	All Else (2008)
Bathymetric Resolution	0.28 pts/m <sup>2</sup>	0.60 pts/m <sup>2</sup>
Terrestrial Resolution	0.11 pts/m <sup>2</sup>	5.54 ts/m <sup>2</sup>

#### *S4.1.2. QA/QC*

To ensure that the data sets were accurate and comparable, overlapping data sets were compared between ground-based and boat-based surveys and ground-based and LiDAR surveys. In addition, the accuracy of ground-based surveys was checked at known benchmarks. Overall, the mean differences among the survey methods were within the river's mean grain size (97mm).

For SBS data compared to the total station surveys, 50% of the data is within 0.15 m, 75% of the data is within 0.18 m, and 94% of the data is within 0.3 m. 8769 ground-based RTK GPS points were compared to the LiDAR data along flat surfaces. 54% of LiDAR points were within 0.03 m, 84.7% of LiDAR points were within 0.061 m, and 98.7% were within 0.12 m. Regular total station control point checks yielded accuracies of 0.0091-0.018 m. RTK GPS observations had vertical precisions of 0.018 m. Comparison of LiDAR water edge points versus the RTK GPS yielded observed differences of 30% within 0.03 m, 57% within 0.061 m and 92% within 0.15 m.

#### *S4.2. 2014 Topographic Map Supplements*

##### *S4.2.1. Data Collection*

Data for the 2014 LYR topographic map comprises of airborne bathymetric LiDAR, multi-beam sonar (MBS), single-beam sonar (SBS), and real-time kinematic (RTK) GPS survey points. The vast majority of the data comes from the airborne bathymetric LiDAR, which uses a green laser to penetrate the water column and a near-infrared (NIR) laser that provides better delineation of terrestrial features. MBS data was used to



fill in the data gaps in deep pools where the green LiDAR was not able to penetrate through to the streambed, and SBS and RTK-GPS survey points were used to fill in the few remaining gaps and places where aquatic vegetation affected the LiDAR returns.

Table S7 outlines each survey method, location(s), and date(s) of acquisition.

**Table S7.** Data sources for 2014 Lower Yuba River

Data	Location(s)	Dates(s)
SBS	1-EDR	2013
SBS	2-NR Pool	5/29/13-5/30/13,
SBS	2-NR	10/16/13-10/18/13
RTK-GPS	2-NR	9/11/13, 11/25/13
RTK-GPS	Backwater in 6-DPDR	3/6/13-2/6/14
MBS	LYR pools except in 2-NR & 1-EDR	8/11/14-8/14/14
NIR & Green LiDAR	All LYR	9/27/14
RTK-GPS	backwater in 6-DPDR	4/2/15
SBS	data gaps below DPD	6/3/15
RTK-GPS	data gaps below DPD	6/3/15
SBS	Dry Creek confluence	6/8/15
RTK-GPS	Dry Creek confluence	6/8/15, 6/16/15

*Updated Coordinated System*

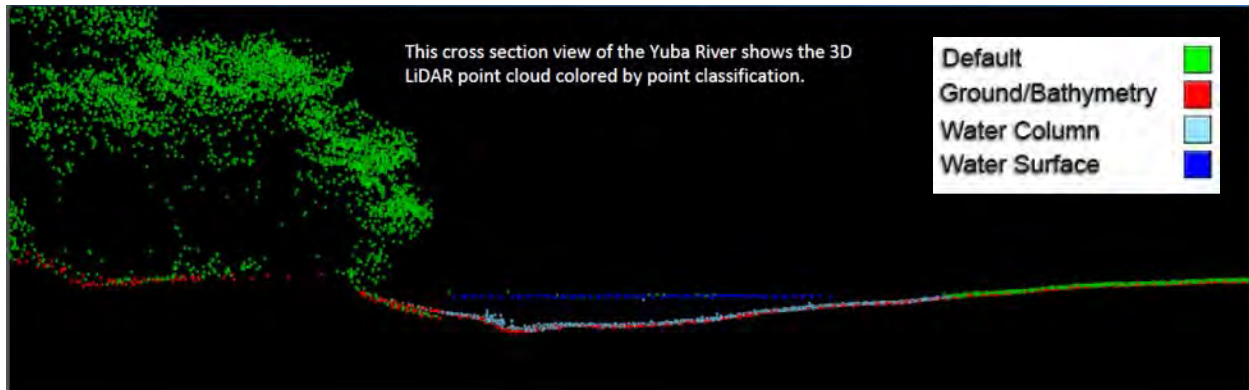
All of the data, except for the 2014 LiDAR, was collected in California State Plane Zone 2 NAD83 U.S. Survey Feet. In 2011 there was an update to the NAD83 datum and the 2014 LiDAR data was collected using the 2011 update, California State Plane Zone 2 NAD83 (2011) U.S. Survey Feet. All other survey data has been re-projected into this new coordinate system in ArcGIS, including the 2006/2008 mapping efforts, to allow direct comparisons.

*Airborne Bathymetric LiDAR Data*

Quantum Spatial, Inc. (Sheboygan, WI) collected combined near-infrared (NIR) and green LiDAR data on 27 September 2014 during a period of low flow, approximately

15.3 m<sup>3</sup>/s above Daguerre Point Dam (DPD) and 11.3 m<sup>3</sup>/s below DPD. Low flows expose more of the river corridor for the NIR laser and reduce the depth needed for the green laser to penetrate through the water column. Processing of the data by Quantum Spatial, Inc. (QS) prior to delivery included:

- 1) Resolving the kinematic corrections for aircraft position data and developing a smoothed best estimate of trajectory (SBET).
- 2) Calculating the laser point positions using the SBET.
- 3) Performing a manual relative accuracy calibration using the tie-plane methodology and filtering erroneous points.
- 4) Using ground classified points to test relative accuracy (i.e. agreement between overlapping flight lines). Performing line-to-line calibrations for attitude parameters (pitch, roll, heading), mirror flex, and GPS drift.
- 5) Creating a water's edge breakline to distinguish between bathymetric returns and terrestrial returns.
- 6) Correcting for refraction through the water column for bathymetric returns.
- 7) Classifying the resulting data into ground (includes bathymetric points), water surface, water column, noise, and default (includes structures, vegetation, and noise) point classes.
- 8) Assessing the statistical absolute accuracy via direct comparisons of ground classified points to ground control survey data.
- 9) Manually reviewing and finalizing data classifications.
- 10) Generating 3.0 foot resolution bare earth and highest return rasters.
- 11) Exporting 1.5 foot NIR and green laser intensity rasters.



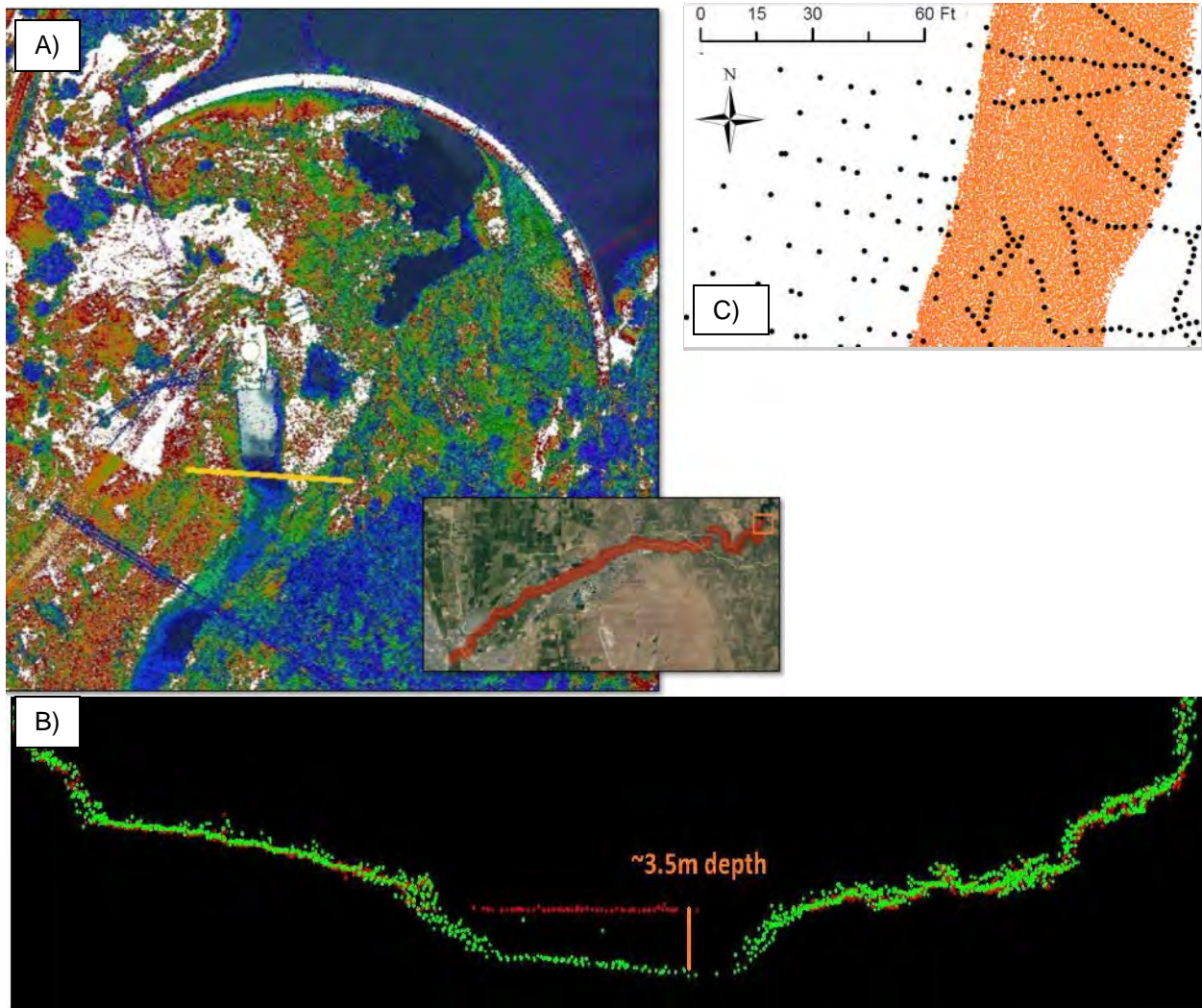
**Fig. S3.** Cross-section showing Quantum Spatial's LiDAR point classifications.

Fig. S3 shows a cross-section of the LiDAR data with the point classifications that QS used. The water surface points come from reflectance off the water surface by the NIR laser. The ground/bathymetry points come from the green and NIR laser in the terrestrial environment and only the green laser within the wetted channel. Note how there appears to be many ground returns that exist within the default classification in bare earth areas.

Good water clarity allowed the green laser to penetrate approximately 3-3.5 meters in depth (Fig. S4) yielding over 90% coverage of the streambed for LYR. First return densities give a representation of the number of laser pulse returns per area (excluding echoes). The data shows that actual point densities far exceeded the target point densities with first return point densities for the green and NIR sensors above 12 pts/m<sup>2</sup> (Table S8).

**Table S8.** First return point densities

First Return Type	Target Point Density	Actual Point Density
Green Sensor	4 pts/m <sup>2</sup>	13.50 pts/m <sup>2</sup>
NIR Sensor	8 pts/m <sup>2</sup>	12.23 pts/m <sup>2</sup>
Combined	12 pts/m <sup>2</sup>	25.67 pts/m <sup>2</sup>



**Fig. S4.** A) Plan view of LiDAR returns colored by intensity below Englebright Dam. B) Profile view of the Englebright Dam pool which shows the penetration depth of the green LiDAR (shown in green) and reflectance off the water surface by the NIR LiDAR (shown in red). C) Contrasting the previous total station and single-beam sonar mapping (black dots) in 3-TBR with the density of the current multi-beam sonar data (orange dots).

After ground points were classified by QS, the density of ground points was analyzed and is presented in Table S9.

**Table S9.** Ground classified point densities

Ground Return Type	Point Density
All Ground Classified Returns	3.96 pts/m <sup>2</sup>
Bathymetric Bottom Returns	2.30 pts/m <sup>2</sup>

Absolute accuracy is an estimate of the error of the LiDAR derived ground surface when compared to a more accurate survey method. QS compared the LiDAR ground surface to 23 ground check points and 24 bathymetric check points that were developed from an RTK-GPS survey. The Fundamental Vertical Accuracy (FVA) is a measure of error reported at the 95% confidence level, i.e.  $1.96 \times \text{Root Mean Square Error (RMSE)}$ . The FVA for ground points and bathymetric points is 0.123 ft and 0.384 ft, respectively (Table S10).

**Table S10.** Summary of absolute accuracy statistics

	Sample	Average	Median	RMSE	Standard Deviation	FVA
Ground Check Points	23	0.008 m	0.006 m	0.019 m	0.018 m	0.038 m
Bathymetric Check Points	24	0.030 m	0.042 m	0.060 m	0.052 m	0.117 m

Relative accuracy is an estimate of the internal consistency of the LiDAR data. It is checked by comparing the identification of the same surface by overlapping flight lines. QS calculated the relative accuracy of the NIR and green laser using 125 and 122 surfaces, respectively. The relative accuracy reported at the 95-percentile,  $1.96 \times \text{Standard Deviation (SD)}$ , is 0.032 m and 0.048 m for the NIR and green lasers, respectively (Table S11).

**Table S11.** Summary of relative accuracy statistics

	Sample	Average	Median	RMSE	Standard Deviation	$1.96 \times \text{SD}$
NIR Laser (terrestrial)	125 surfaces	0.030 m	0.043 m	0.047 m	0.016 m	0.032 m
Green Laser (bathymetric)	122 surfaces	0.049 m	0.076 m	0.072 m	0.025 m	0.048 m

### *Multi-beam and Single-beam Sonar Data*

The MBS data was collected over a period of four days from 11-14 August 2014 when flows on LYR were 40.0 m<sup>3</sup>/s above DPD and 22.7-24.5 m<sup>3</sup>/s below DPD. The higher water levels facilitate overlapping of the MBS and LiDAR data. The MBS data covers most of the deep pools from the confluence of the Yuba with the Feather River to the Narrow's canyon where a rapid prevents upstream access by boat. The MBS data has an average point density of 43.7 pts/m<sup>2</sup>. Fig. S4c shows a visual comparison of the density of MBS points compared to previous SBS and total station mapping in 3-TBR.

SBS data was collected in 1-EDR upstream of 2-NR to Englebright Dam in 2013 by kayak. SBS data was also collected by jet boat on 3 June 2015 to fill in the remaining small data gaps in areas below DPD and on 8 June 2015 by kayak where Dry Creek flows into the lower Yuba River.

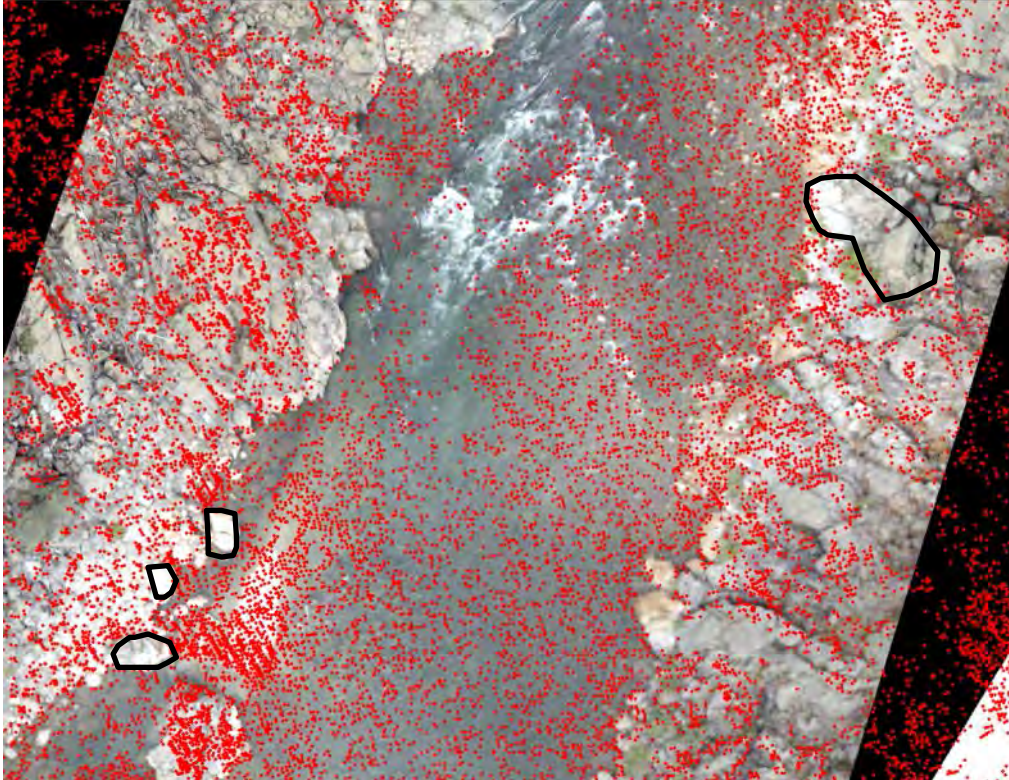
The MBS and SBS data were filtered and compared to the LiDAR data to determine their offsets and relative accuracy.

### *RTK-GPS Data*

Several small areas that were affected by vegetation were surveyed with an RTK-GPS, including a backwater area in 6-DPDR called the Blue Lagoon, the Dry Creek tributary, and a few small backwater areas below DPD.

#### *S4.2.2. Correcting Missing Boulder and Bedrock Data*

Although QS classified the LiDAR data using automated algorithms, careful inspection revealed that significant areas of boulders and exposed bedrock features were filtered out of the ground/bathymetry point classification (Fig. S5). In general, bare-earth areas were smoothed out (i.e., over-filtered) and topographically complex features like boulders and exposed bedrock were often removed entirely as if the points were vegetation. LiDAR intensity imagery clearly distinguished these areas as bare earth, warranting additional filtering, but a trade-off exists between retaining fine-scale topography and erroneously classifying non-ground returns (e.g., low-lying vegetation or small man-made structures) as ground. Thus, a two-stage filtering process was developed to return the over-filtered points to the ground and bathymetry classification without introducing erroneous data.



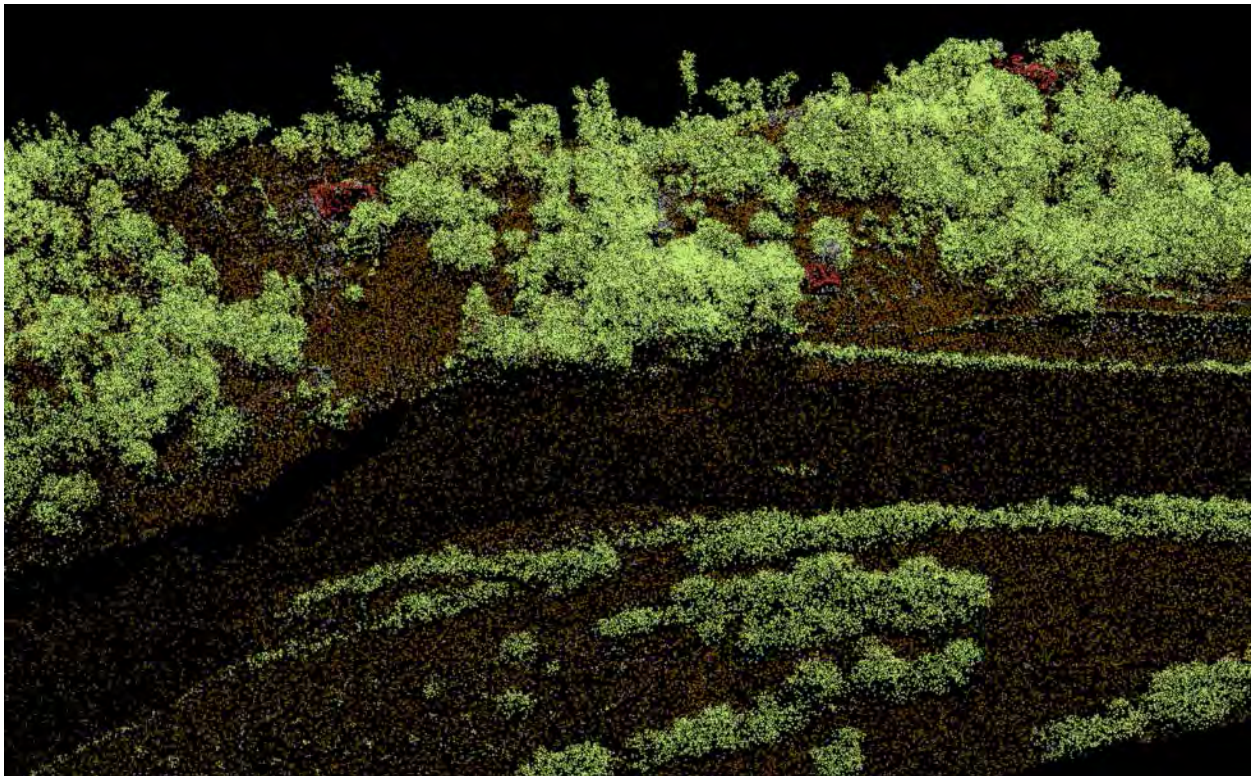
**Fig. S5.** Several examples of over-filtered ground returns outlined in black in the Englebright Dam reach, but the phenomenon is widespread throughout the image. Low cracks in the bedrock received many points as evident by lines of points and higher bedrock tops were filtered out leaving them unrepresented in the ground/bathymetry point file.

### *Improving Identification of Terrestrial Rock Features*

The first step was to take all of the returns not classified as ground, and identify a new ground surface using the `lasground_new.exe` program within LAsTools (Gilching, Germany), which uses a variation of Axelsson (2000) to generate an adaptive triangulated irregular network (TIN). The user must select a “step size”, which controls the size of the grid cells that initialize the TIN by choosing the lowest point in each grid cell. Smaller step sizes allow for finer-scale features to be identified in the ground-detection process. The “-wilderness” setting was used for filtering this data, which uses a three-meter step size. A second stage of filtering removed the areas that are influenced by vegetation or structures by creating a clip polygon of vegetated areas and man-made features. This was done by using additional tools within LAsTools that



distinguish between vegetation and structures by calculating the standard deviation of points above the ground-identified surface. Fig. S6 shows an example of the data after the LiDAR points have been reclassified and Fig. S7 shows the resulting land cover raster. Finally, a polygon file is made that includes the areas where points should not be added back into the ground point classification: vegetated ground, water, and structures. This polygon was visually verified and adjusted, where necessary, by using the LiDAR intensity imagery, the 2014 NAIP normalized-difference vegetation index, and the first-return elevation raster. This process ensures that only points from the bare-earth land cover are added back to the ground classification.



**Fig. S6.** Lidar points after reclassifying the default points. Vegetation is green, ground is brown, and structures are red.



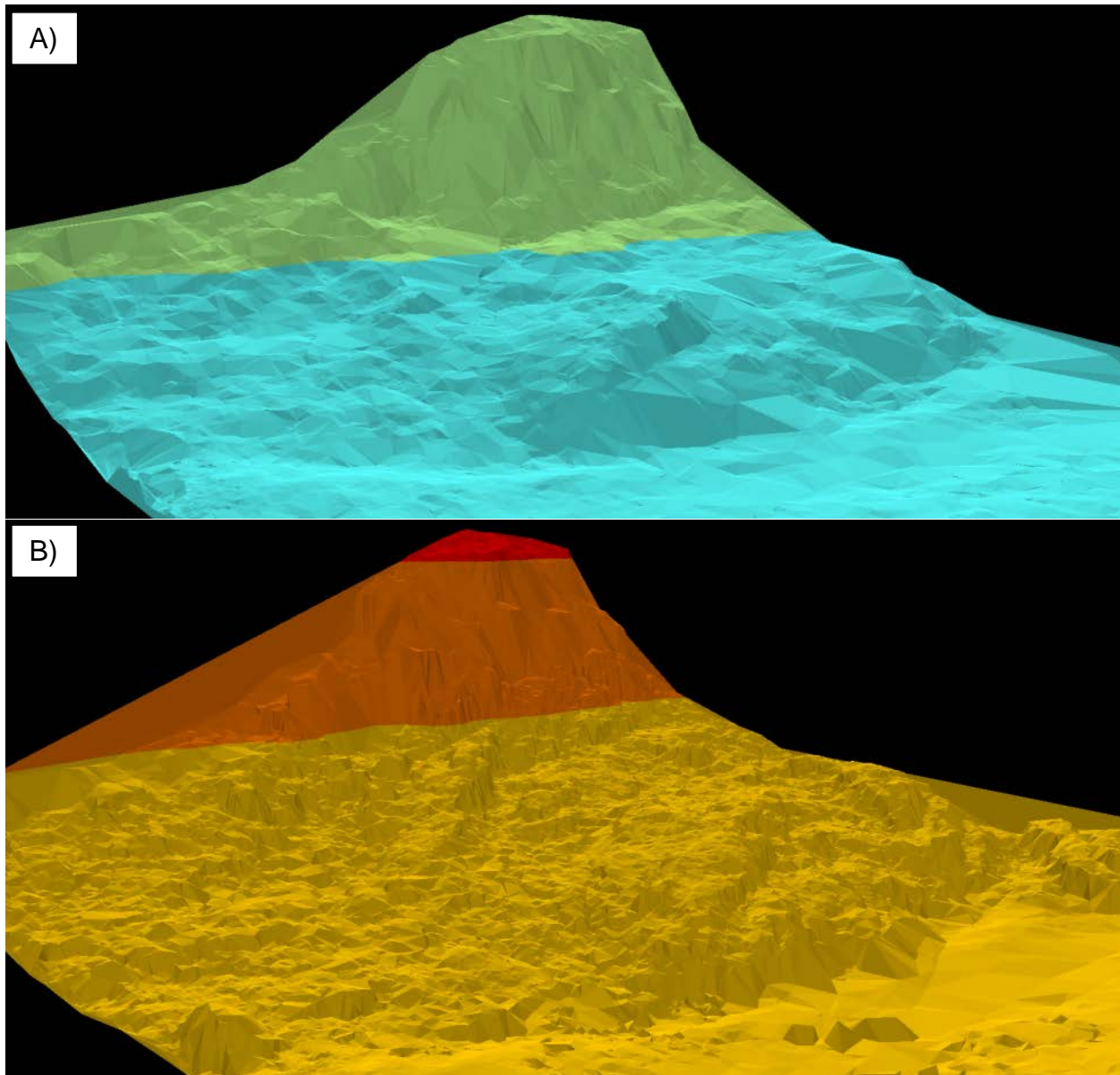
**Fig. S7.** Land cover raster showing vegetation as green, water as blue, and bare-earth as light brown.

#### *Delineating In-Channel Rock Features*

Many of the in-channel areas where there were exposed or submerged rocks suffer from low surrounding point densities making ground classification unreliable (e.g., a large boulder in an otherwise deep pool that is void of bed returns). In these areas, rock features were manually delineated by visualizing the LiDAR point clouds in ArcGIS and confirming their presence with aerial imagery.

#### *Results from Re-filtering*

The points added back into the ground/bathymetry classification improved the resolution of the land surface without introducing significant errors. After this process was completed, the resulting point densities for bare earth areas increased from approximately 4pts/m<sup>2</sup> to 13pts/m<sup>2</sup> and the added detail within the map is visually apparent Fig. S8.



**Fig. S8.** A) An oblique view of a section of the Englebright Dam Reach before refiltering the data. B) That same view after the data was refiltered. Notice the presence of new boulder/bedrock clusters.

#### *S4.2.3. Merging Data*

The LiDAR data serves as the reference datum for all other survey point inputs. In order to merge the MBS and SBS data with the LiDAR data, the sonar data had to be filtered to remove bad returns caused by things like suspended sediment, bubbles, or vegetation. Then the sonar points were compared to the nearest LiDAR points and analyzed for systematic offset as detailed below.

### *Filtering Multi-beam and Single-beam Sonar Data*

The SBS data is too sparse to use filtering algorithms like LAStools, so the best way to assess the quality of the data is to compare it to the LiDAR (if nearby LiDAR points exist) and/or visualize the data in 3D. Using a TIN colored by slope values, spires and elevated plateaus of points that are caused by bubbles or sediment are easily located and removed.

The MBS data can be filtered just like the LiDAR data using lasground\_new.exe in LAStools, but issues with horizontal offsets may exist when comparing the sonar data to the LiDAR data. These areas can be quickly identified by spatially joining the MBS data to the nearest LiDAR point and highlighting the points with the largest vertical offsets. There are two probable causes of these horizontal offsets. First, the cliff face could be overhung such that the MBS is correctly identifying undercut caverns beneath the overlying terrain whose surface the LiDAR is mapping. This might occur for bedrock walls and cohesive hillsides. For jumbles of boulders and riprap, it is similarly possible that the MBS is obtaining returns through connected pore spaces deep into the pile. Second, it is very likely that there are “multipath errors”. These occur when some or all MB signals do not take a direct path back to the detector, and instead reflect off one or more other surfaces. This adds extra travel time causing an apparent extra distance. This is likely when there are vertical boundaries close to horizontal ones, such as when the boat is close to a steep bank or bridge pier. In at least some of the cases, we know that the MBS data is misbehaving because this issue occurs along vertical bridge piers where the LiDAR and MBS data should be in agreement. Thus, unless there is on-site

evidence that the cliff face is overhung, the LiDAR data should be used to delineate the edges where the two data sources do not agree.

*Merging the Multi-beam and Single-beam Sonar Data with the LiDAR Data*

After the MBS and SBS data are filtered, the sonar points can be compared to the LiDAR points to assess vertical offsets. The goal for maximum precision and accuracy is to make a unique shift for each dataset rather than lump all of the data and do one shift. First, a polygon is created to clip the LiDAR points that overlap with the sonar data. Then, using the spatial join tool in ArcGIS, the LiDAR points that are within 5 cm horizontally of the sonar data are analyzed for their vertical offsets. To calculate the needed offset, the mean signed error for all the points that have a vertical difference of less than 0.3048 m (1 ft) was used. After applying the shifts, the vertical offsets were re-analyzed and the mean error assessed for all the points to verify that the shift performed as expected. Table S12 summarizes the number of observations, mean error, and standard deviations for the sonar and LiDAR comparisons.

**Table S12.** Vertical Comparisons of the Shifted Sonar Points to the LiDAR Points

Reach	MBS and LiDAR Comparisons					SBS and LiDAR Comparisons		
	8-MR/7-HR	6-DPDR	4-PBR	3-TBR	2-NR	6-DPDR	2-NR	1-EDR
Number of points	13127	9323	25156	71529	11925	5029	117	505
Mean Error [m]	0.008	0.003	-0.012	-0.001	0.058	-0.015	0.013	-0.002
St. Dev. [m]	0.11	0.08	0.16	0.08	0.07	0.11	0.20	0.18

## S5. Analytical Methods Supplements

### S5.1. DEM Uncertainty Supplements

The survey error, *SE*, represents the ability to accurately detect the ground surface. In this DEM uncertainty method, *SE* is estimated for three different land cover classes: bare earth, water, and vegetated ground.

For the bare earth land cover, LiDAR points from the 2008 and 2014 data sets were compared to each other on road surfaces throughout the study area. Points that were within 5 cm of each other in the horizontal direction were compared for their vertical differences. Assuming that the road surface hasn't changed over that time period, this analysis provides a representation of the relative accuracy of the LiDAR to repeatedly identify a bare ground surface. 1033 comparisons were made yielding a standard deviation of 0.039 m (Table S13).

**Table S13.** Comparisons between points less than 5 cm apart in the horizontal direction for the 2008 and 2014 LiDAR data sets on road surfaces throughout the lower Yuba River.

N	Mean Error [m]	Median [m]	Standard Deviation [m]
1033	0.000	0.001	0.039

For wetted areas, there are not surfaces that can be identified as static over the study period. However, the vertical agreement between the 2014 LiDAR and 2014 MBS data sets provide a good estimate of the relative accuracy of detecting the ground surface within the water. These comparisons are presented in Table S12 and yielded a standard deviation of 0.074m.

Lastly, for vegetated areas, a review of literature for vertical errors in LiDAR for forested settings was conducted (Table S14). Due to the LiDAR being flown during leaf-on conditions in August and September, the studies that represent those conditions (Reutebuch et al., 2003; Gould et al., 2013; Edson and Wing, 2015) present the best comparisons. Reviewing the list of relevant studies, 0.30 m was chosen for vegetated ground areas, which is within the range of reported RMSEs.

**Table S14.** Vertical errors in ground detection for LiDAR in forested settings.

Study	Vegetation Type	RMSE [m]	Slope Degrees	Leaf on/off
Reutebuch et al. (2003)	conifer forest	0.32	wide range	On
Hodgson and Bresnahan (2004)	Brush/low trees	0.233	4.2	Off
Hodgson and Bresnahan (2004)	Deciduous	0.259	2.5	Off
Gould et al. (2013)	Isolated <i>Ceanothus</i>	0.207	<14	On
Gould et al. (2013)	Continuous <i>Ceanothus</i>	0.433	<14	On
Tinkham et al. (2013)	Shrub	0.18	7.9	Off
Tinkham et al. (2013)	<i>Ceanothus</i>	0.30	8.9	Off
Tinkham et al. (2013)	Deciduous	0.20	6.5	Off
Edson and Wing (2015)	2:1, Conifer to deciduous	0.38	17	On
Edson and Wing (2015)	2.5:1, Conifer to Deciduous	0.37	17	On

In conclusion, *SE* for bare ground, water, and vegetated ground was 0.039 m, 0.074 m, and 0.30 m, respectively. These values compare well with the LiDAR absolute and relative accuracy statistics provided by QS for terrestrial and bathymetric points (Table S10 & Table S11). No parallel test is available for assessing the value chosen for the vegetated ground *SE*.

S5.2. Sediment Budget Supplements

Fig. S9 shows an example of the station boxes that were used to create longitudinal profiles of volumetric change and percent area of scour and fill.



Fig. S9. Station boxes used for creating longitudinal profiles of erosion and deposition volumes and percent area. The boxes are 1.524 m wide (5-ft), orthogonal to the valley centerline, and clipped to the width of the river valley.

S6. Results Supplements

See the attached overview maps of the 2006/2008 – 2014 topographic change results.



## Supplementary References

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