Lawrence Berkeley Laboratory
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Engineering & Technical Services Division

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LAWRENCE RADIATION LABORATORY - UNIVERSITY OF CALIFORNIA

ENGINEERING NOTE

CODE	SERIAL	PAGE
P40200	M5436 A	1 OF 11

AUTHOR	DEPARTMENT	LOCATION	DATE
C. PETERS	MECH. ENGR.	LBL	11-20-79

PROGRAM — PROJECT — JOB
PEP-4

MAGNET CORE

TITLE
POLE TIP CARRIAGE

THE PURPOSE OF THIS NOTE IS TO DETERMINE STRESSES AND SAFETY FACTORS WITHIN MEMBERS AND JOINTS OF THE POLE TIP CARRIAGE UNDER STATIC LOADS AND UNDER EARTHQUAKE LOAD CONDITIONS.

REV "A" 2-23-81 P. Pingel

ADDED OVERTURNING CALCULATIONS AND TIE DOWN REQUIREMENTS FOR A .7g EARTHQUAKE.

WITHOUT TIE DOWN THE ACCELERATION "g" AT WHICH ONE SET OF ROLLERS WOULD START TO LIFT IS:

	DIRECTION OF FORCE (SEE PAGE 2)		
	FRONT	BACK	SIDE
CARRIAGE & POLE TIP ASSY	.28g	.50g	.46g
CARRIAGE & POLE TIP ASSY + CONCRETE BLOCK	.32g	.50g	.49g

IF THE CARRIAGE IS RESTRAINED AGAINST A .7g EARTHQUAKE THE BOLTS HOLDING PART 19C9914 "CARRIAGE ROLLER OUTER SLEEVE" TO THE FRAME HAVE TO BE SAE Gr 5. THE BOLTS SHOULD BE CHECKED AND REPLACED IF REQUIRED.

LAWRENCE RADIATION LABORATORY - UNIVERSITY OF CALIFORNIA

ENGINEERING NOTE

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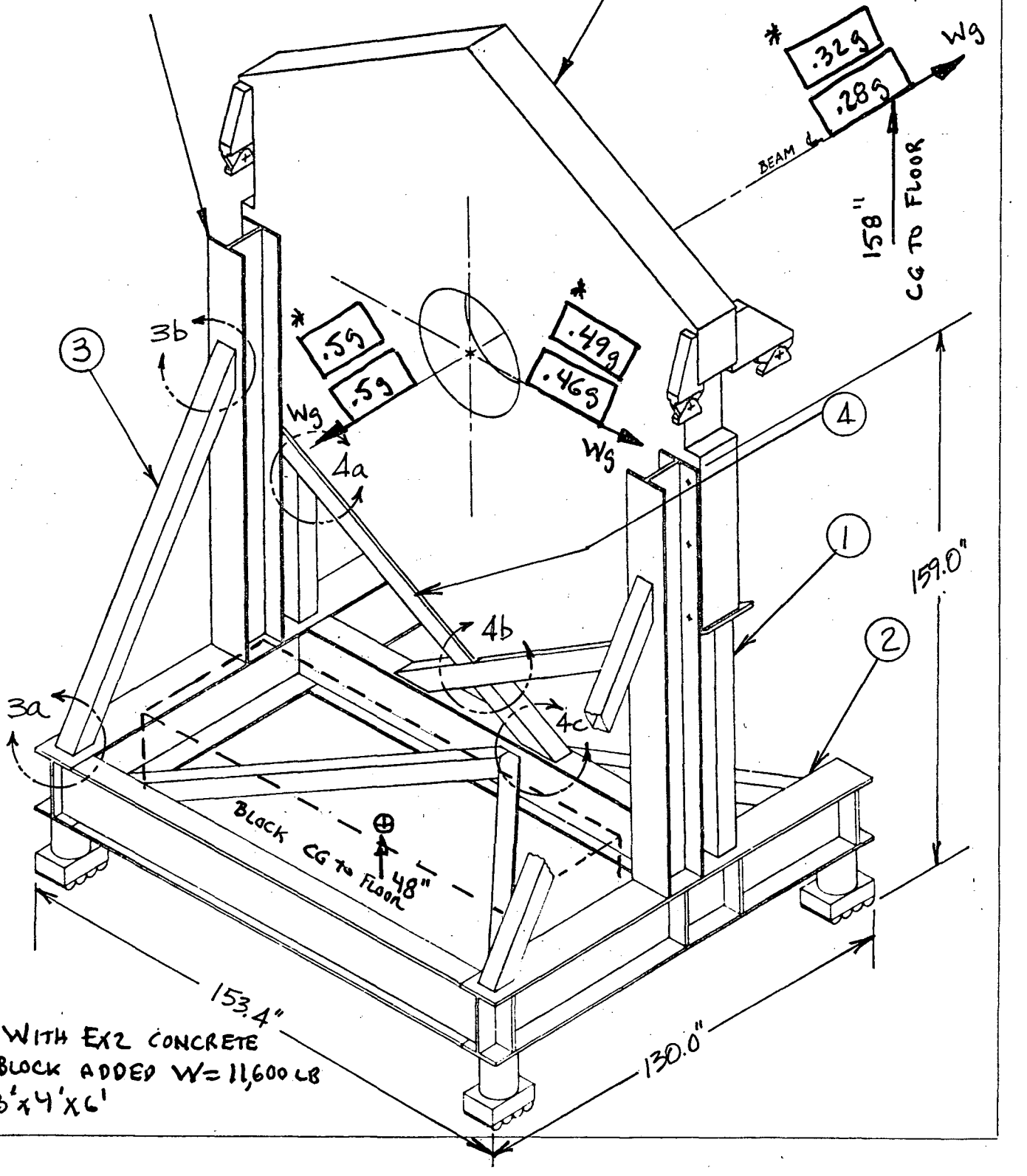
TITLE

POLE TIP CARRIAGE

POLE TIP ASSY.

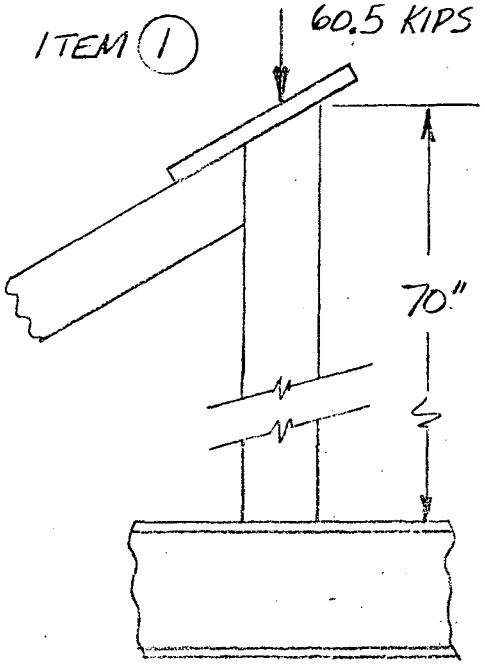
TYP. ASSY. WT. = 121,000 LBS.



* WITH EX2 CONCRETE BLOCK ADDED W = 11,600 LB 3'x4'x6'

I) STATIC LOAD CONDITIONS

ITEM (1)

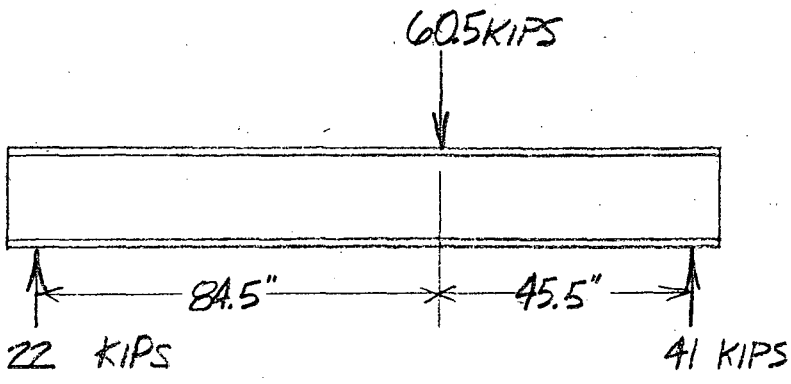


6"x6"x ½" STL. TUBE

MAX. ALLOWABLE AXIAL LOAD
(KL = 6' & 46 KSI YIELD) =
269 KIPS
(RYERSON DATA)

FACTOR OF SAFETY = $\frac{269}{60.5}$
= **4.29**

ITEM (2) WF 16" X 105 LBS.



$\sigma_b = MC/I$ $I = 1850$ IN^4
$M = 1789$ IN-KIP
$C = 9$ IN

$\sigma_b =$ **8,705 PSI**

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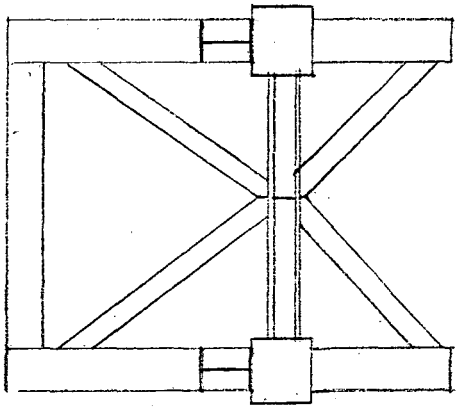
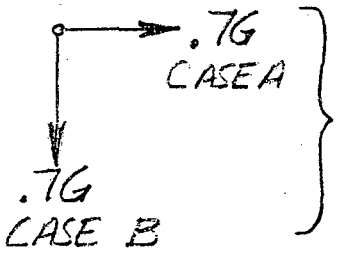
II. EARTHQUAKE LOAD CONDITIONS

TWO CASES:



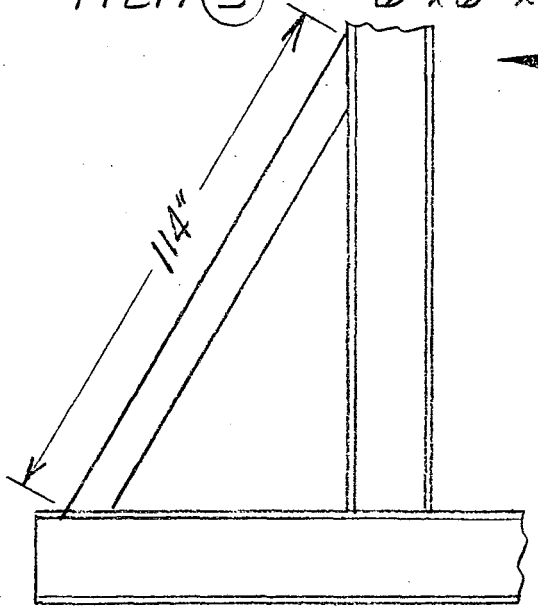
CARRIAGE PLAN VIEW



GROUND ACCELERATION RELATIVE TO CARRIAGE.

CASE A:

ITEM (3) 6"x6"x½" STL. TUBE.



← FORCE DUE TO ACCEL. = 58 KIPS

COMPRESSIVE FORCE IN TUBE = $\frac{58 \text{ KIPS}}{\cos 60^\circ}$

= 116 KIPS

MAX. ALLOWABLE AXIAL LOAD
(KL = 10' & 46 KSI YIELD) = 248 KIPS
(RYERSON DATA)

FACTOR OF SAFETY = $\frac{248}{116} = \boxed{2.14}$

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PROGRAM — PROJECT — JOB

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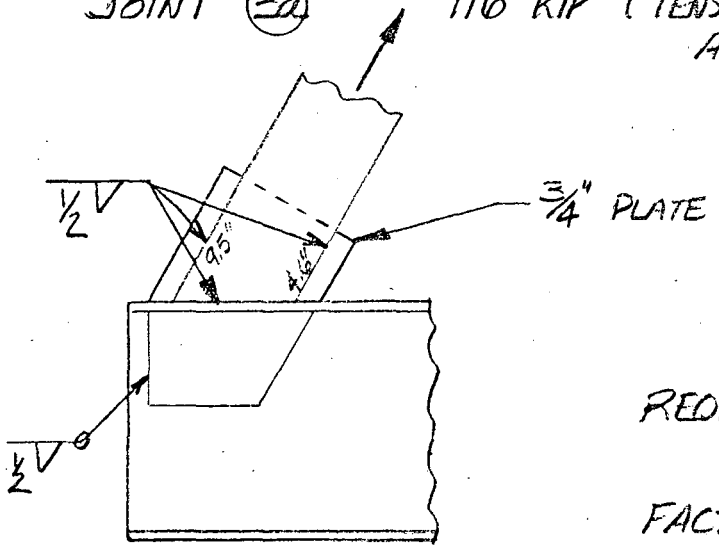
JOINT (3A) 116 KIP (TENSION WHEN ACCEL. IS OPPOSITE AS SHOWN PG. 4)



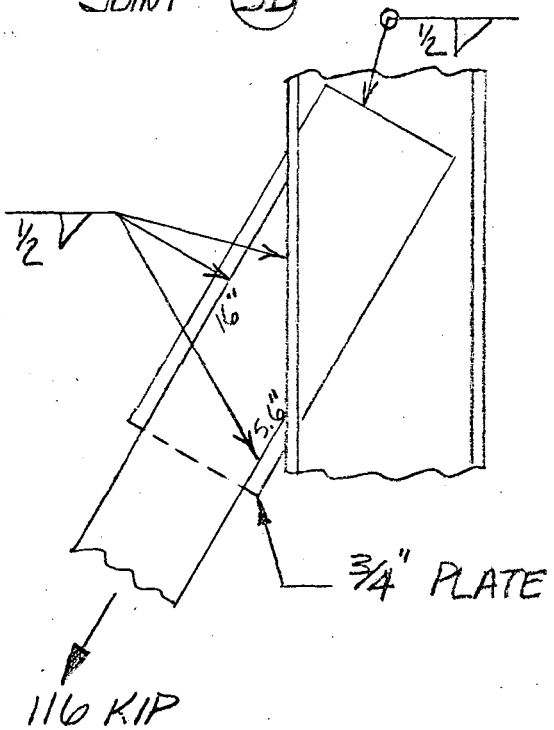
LENGTH OF WELD = $2(9.2+4.6+6+4) = 47.6$ IN

FORCE ON WELD = $\frac{116 \text{ KIP}}{47.6 \text{ IN}} = 2.43 \frac{\text{KIP}}{\text{IN}}$

REQR. WELD SIZE = $\frac{2.43}{9.6} = .25$ IN ← (AWS)

FACTOR OF SAFETY = $\frac{.50}{.25} = \boxed{2.00}$

JOINT (3B)



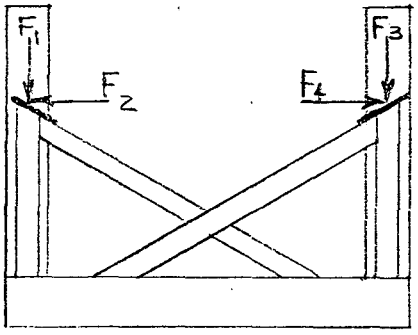
LENGTH OF WELD = $2(16+5.6+4+6) = 63.2$ IN

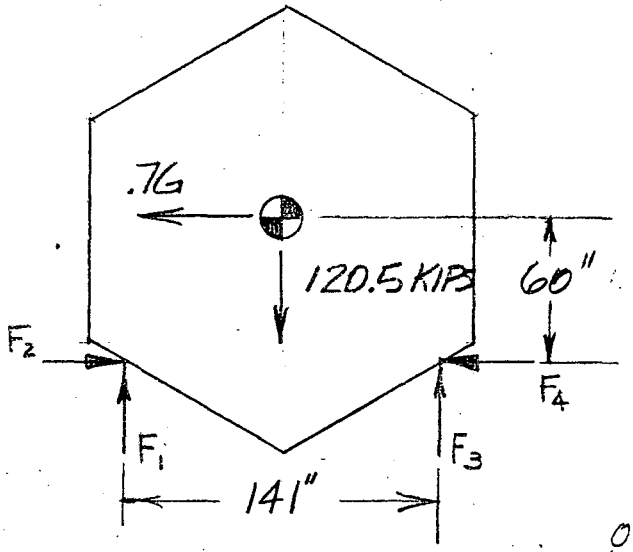
FORCE ON WELD = $\frac{116 \text{ KIP}}{63.2 \text{ IN}} = 1.83$ KIP/IN

REQR. WELD SIZE = $\frac{1.83}{9.6} = .19$

FACTOR OF SAFETY = $\frac{.50}{.19} = \boxed{2.62}$

CASE B:





$\Sigma F_x = Ma_x$

$F_4 - F_2 = (.7)(120.5)$

$= 84.3$ KIP

ASSUME $F_2 \sim 0$

$F_4 = 84.3$ KIPS

$\Sigma F_Y = Ma_Y$

$F_1 + F_3 = 120.5$ KIPS

$\Sigma M = \bar{I}\alpha^{0} + M\bar{a}d$

$\Sigma M_{3,4}$:

$(F_1)(141) - \frac{(120.5)(141)}{2} =$

$-(120.5)(.7)(60)$

$F_1 = 96.1$ KIPS

$F_2 = 24.9$ KIPS

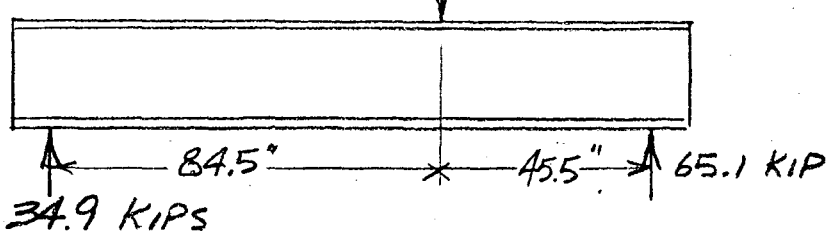
ITEM (1) 6"X 6"X 1/2" STL. TUBE

MAX. ALLOWABLE AXIAL LOAD (KL = 6' & 46 KSI YIELD) = 269 KIPS (RYERSON DATA)

FACTOR OF SAFETY $= \frac{269}{F_1} =$ **2.79**

ITEM (2) WF 18"X 105 LBS

$F_1 =$ 96.1 KIPS



$\sigma_b = MC/I$ $M = 2842$ KIP IN

$C = 9$ IN

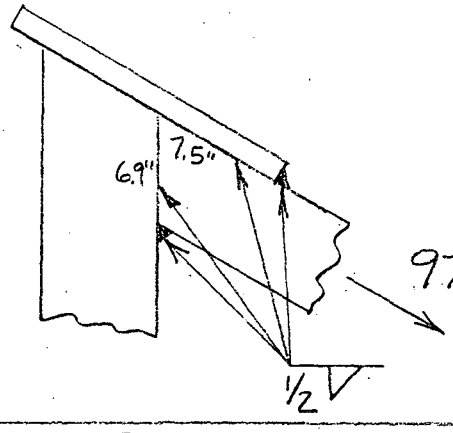
$I = 1850$ IN^4

$\sigma_b =$ **13,827 PSI**

ITEM (4) 6"X6"X ½" STL. TUBE

TENSION FORCE IN TUBE (WHEN ACCEL. OPPOSITE SHOWN) = $\frac{84.3}{\cos 30°}$ = 97.3 KIP

JOINT (4a)



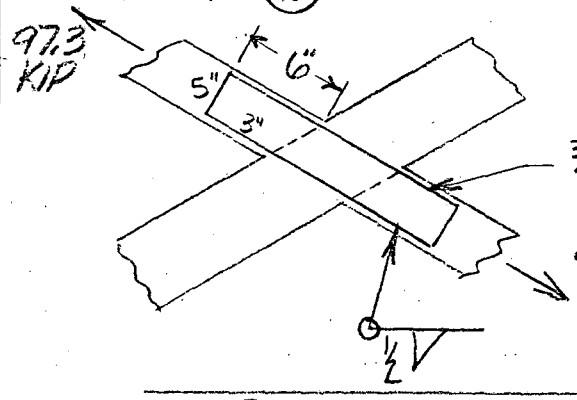
WELD LENGTH = (7.5+6.9+6)2 = 41 IN.

FORCE ON WELD = $\frac{97.3 \text{ KIPS}}{41 \text{ IN}}$ = 2.37 $\frac{\text{KIP}}{\text{IN}}$

REQR. WELD SIZE = $\frac{2.37}{9.6}$ = .25 IN

FACTOR OF SAFETY = $\frac{.50}{.25}$ = **2.0**

JOINT (4b)



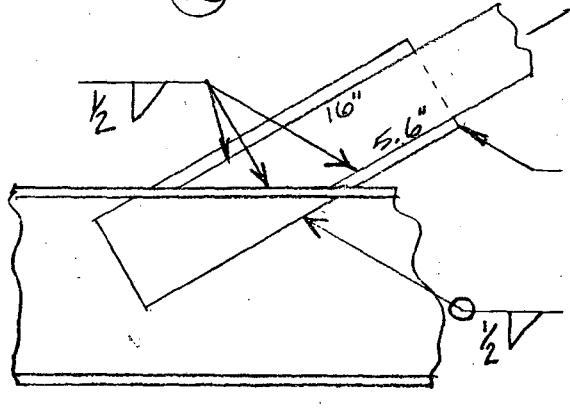
WELD LENGTH = (6+6+5+3)2 = 40 IN.

FORCE ON WELD = $\frac{97.3 \text{ KIP}}{40 \text{ IN}}$ = 2.43 $\frac{\text{KIP}}{\text{IN}}$

REQR. WELD SIZE = $\frac{2.43}{9.6}$ = .25 IN

FACTOR OF SAFETY = $\frac{.50}{.25}$ = **2.0**

JOINT (4c)



WELD LENGTH = 2(16+5.6+6+4) = 63.2 IN.

FORCE ON WELD = $\frac{97.3 \text{ KIP}}{63.2 \text{ IN}}$ = 1.54 KIP/IN

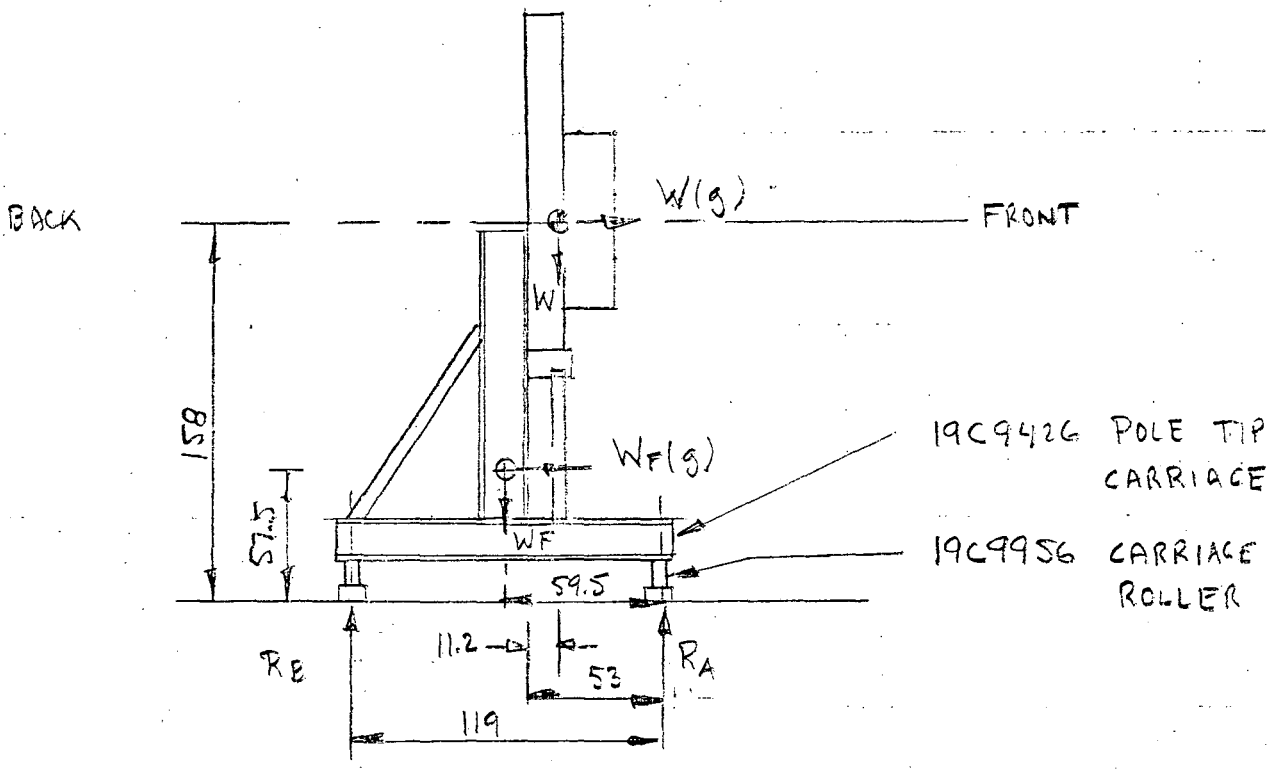
REQR. WELD SIZE = $\frac{1.54}{9.6}$ = .16 IN

FACTOR OF SAFETY = $\frac{.50}{.16}$ = **3.12**

FOR POLE TIP ASSY CG LOCATION SEE ENG NOTE M5267

POLE TIP ASSY WT = W = 130 K

CARRIAGE WT = W_F = 8.0 K



CHECK OVERTURNING IN WORST DIRECTION

(+) ΣM AROUND R_A

CHECK g FOR OVERTURNING $R_B = 0$

$$130^K(g)\,158 + 8^K(g)\,57.5 - 8^K(59.5) - 130^K(53-11.2) = 0$$

$$g = \frac{8(59.5) + 130(41.8)}{130(158) + 8(57.5)} = \frac{5910}{21000} = \boxed{.28}$$

HOLD DOWN FORCE REQUIRED AT R_B FOR g = .7

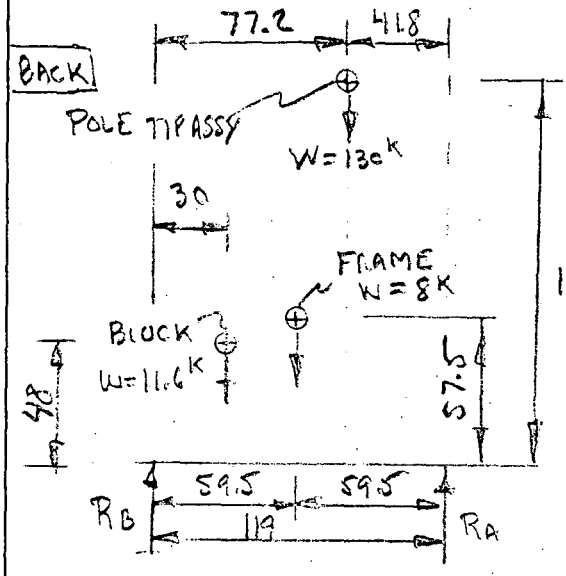
$$R_B(119) + 21000(.7) - 5910 = 0$$

$$R_B = -\frac{21000(.7) + 5910}{119} = \frac{8790\ K\text{-}IN}{119\ IN} = 74K$$

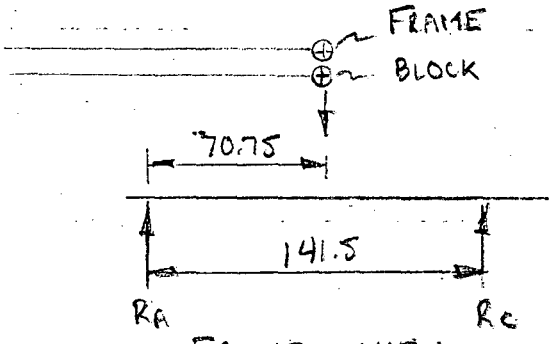
HOLD DOWN FORCE EACH CORNER $= \frac{74}{2} = \boxed{37\ K} = P$

SHEAR AT EACH CORNER $S = \left(\frac{130+8}{4}\right).7 = \frac{96.6K}{4} = \boxed{24.1K}$

CHECK OVERTURNING IN OTHER DIRECTIONS



SIDE VIEW



FRONT VIEW

NO CONCRETE BLOCK

← FORCE TOWARDS BACK

+↻ ΣM AROUND RB

$$g = \frac{130(77.2) + 8(59.5)}{130(158) + 8(57.5)} = \frac{10516}{21000}$$

$$= \boxed{.50}$$

FORCE SIDE WAYS →

$$g = \frac{(130+8)(70.75)}{21000}$$

$$= \boxed{.46}$$

WITH EXT BLOCK W=11.6K 3'x4'x6'

→ F TOWARDS FRONT

↺+ ΣM AROUND RA

$$g = \frac{130(41.8) + 8(59.5) + 11.6(89)}{130(158) + 8(57.5) + 11.6(48)} = \frac{6940}{21560}$$

$$= \boxed{.32}$$

→ FORCE SIDE WAYS

$$g = \frac{(130+8+11.6)70.75}{21560}$$

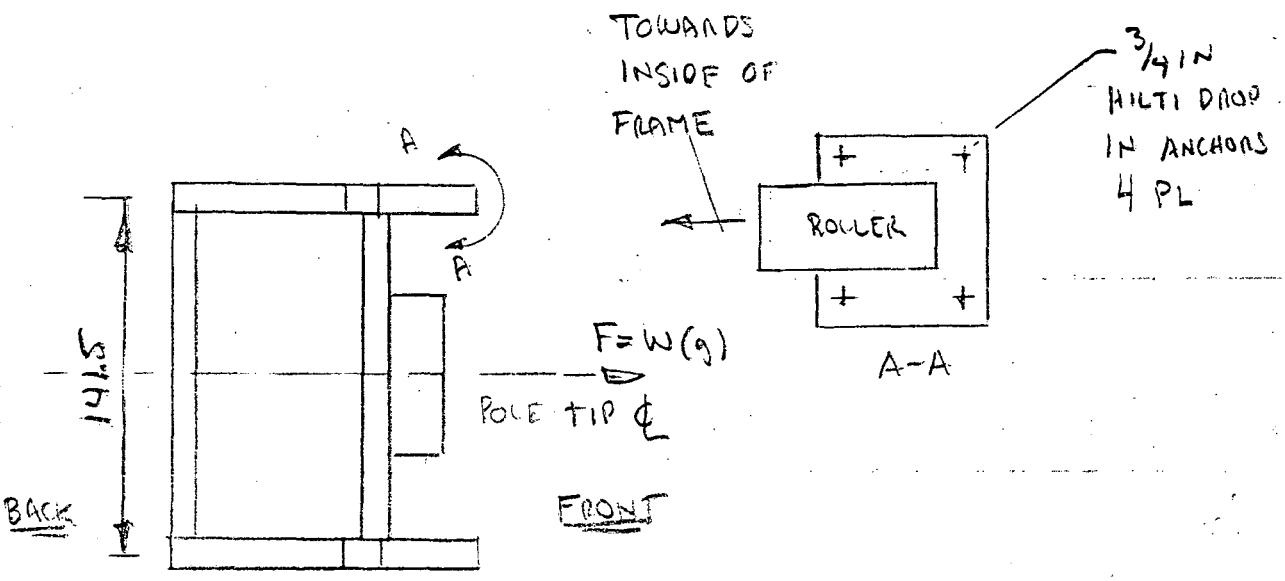
$$= \boxed{.49}$$

← FORCE TOWARDS BACK

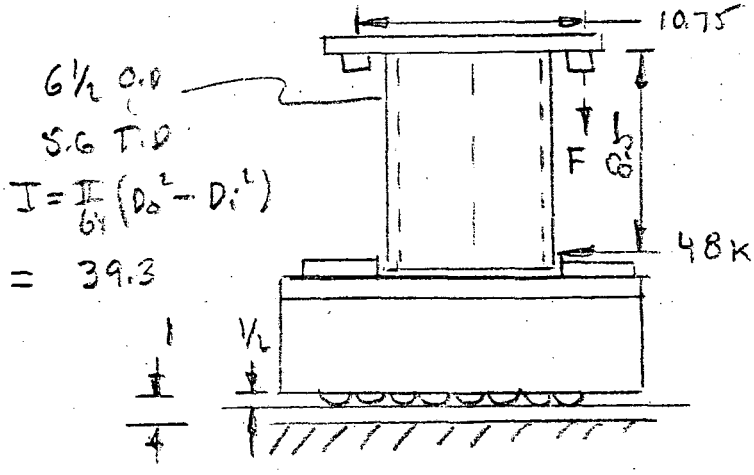
+↻ ΣM AROUND RB

$$g = \frac{130(77.2) + 8(59.5) + 11.6(30)}{21560} = \frac{10860}{21560}$$

$$= \boxed{.50}$$



WORST CASE FORCE ALONG POLE TIP ℄
FRONT STOPS TAKE ALL SHEAR FORCE
BACK STOPS TAKE UPLIFT



$I = \frac{\pi}{64}(D_o^4 - D_i^4)$

$= 39.3$

$M = 48^K \times 8.5 = 408\ K\text{-}IN$

$S = \frac{I}{C} = \frac{39.3}{3.25} = 12.09$

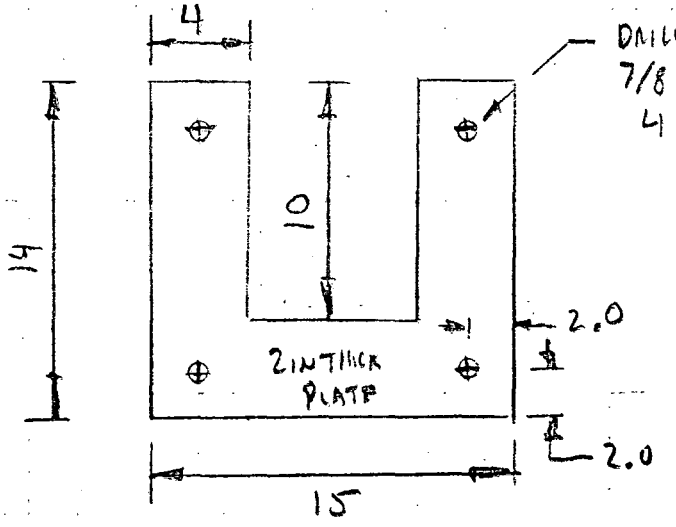
$\sigma = \frac{M}{S} = \frac{408\ K\text{-}IN}{12.1\ IN^3} = 33.7\ KSI$

F.S = 2.2

$\sigma_{YIELD} = 75\ KSI$

D.O.M TUBE

CHECK ATTACHMENT BOLTS



$F = \frac{48^K (8.5)}{10.75} = 38K$

$\sigma = \frac{38K}{.606} = 62\ KSI$

F.S = 1.25

Gr 5 1-8 UNC BOLT

PROOF LOAD = 78 KSI

HILTI DROP IN ANCHORS HDI 3/4 3/4 IN BOLT

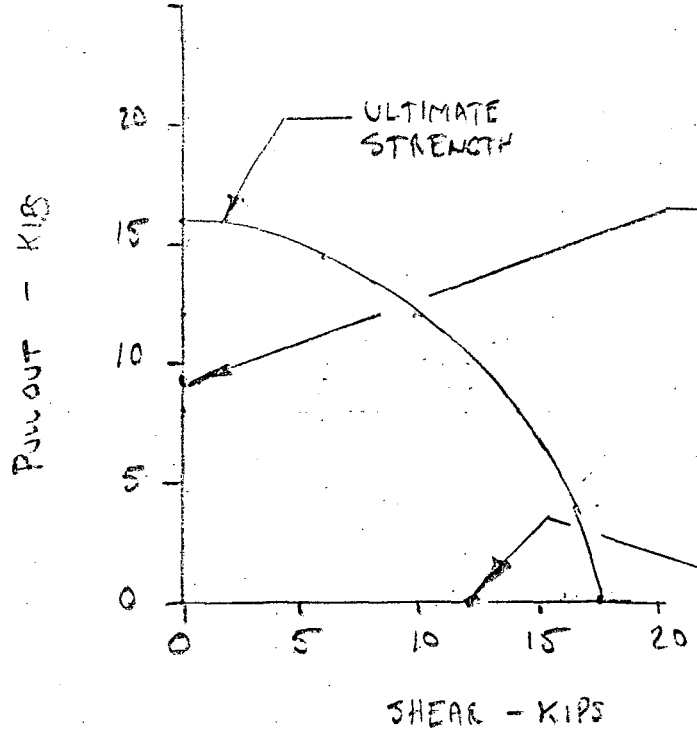
ULTIMATE CAPACITY IN 4000 PSI CONCRETE

PULLOUT = P_u = 16.03 K
SHEAR = S_u = 17.6 K
} HILTI CATALOG H-3408 4/77

FROM McMACKIN P.J "HEADED STEEL ANCHORS UNDER COMBINED LOADING" AISC ENG JOURNAL 2ND QUARTER 1973

FOR COMBINED LOADING

$$\left(\frac{P}{P_u}\right)^{5/3} + \left(\frac{S}{S_u}\right)^{5/3} \leq 1$$



UPLIFT ON REAR STOP

$\frac{37^K}{4 \text{ BOLTS}} = 9.25^K$

SHEAR ON FRONT STOPS

$\frac{96.6^K}{2} = 48.3^K$ / STOP

$\frac{48.3^K}{4 \text{ BOLTS}} = 12^K$

TECHNICAL INFORMATION DEPARTMENT
LAWRENCE BERKELEY LABORATORY
UNIVERSITY OF CALIFORNIA
BERKELEY, CALIFORNIA 94720